

April 19 2013

DESIGN DATA				
Traffic	Average Daily			Max.Hr.
Current 2012 EB	Pass: 3700	Trucks: 2695	Total: 6395	640
Forecast 2032 EB	Pass: 6440	Trucks: 5365	Total: 11805	1185
Current 2012 WB	Pass: 3530	Trucks: 2785	Total: 6315	635
Forecast 2032 WB	Pass: 6145	Trucks: 5545	Total: 11690	1170
Clear Zone Distance: See table.		Design Speed: 35 MPH, 50 MPH		
Minimum Sight Dist. for Stopping: 267', 425'		Bridges: N/A		
Limited Access Control		Design ESALs: 24,468,922 accumulated flexible ESALs		
Pavement Design Life 30 (years)				

CLEAR ZONE DISTANCE TABLE			
DESIGN SPEED	DESIGN ADT	FORESLOPE	
		FLAT	1V:6H
35 MPH or less	Over 6,000	14	16
50 MPH	Over 6,000		22

JOB# 61
NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION

SOIA-7-002(119)053

Williams County
Sec 8, 9, 10, 15, 16, 17 T 156 N, R 97 W
PCC Reconstruction, Turn Lanes, Turn Lane Extensions, Traffic Signal, HAWK System,
Guardrail, Sidewalk, Sidewalk Canopy, Bridge Approach Slabs Joint Seal, and Incidentals

STATE	PROJECT NO.	PCN	SECTION NO.	SHEET NO.
ND	SOIA-7-002(119)053	19702	1	1

GOVERNING SPECIFICATIONS:

Standard Specifications adopted by the North Dakota
Department of Transportation October 2008; Standard Drawings
currently in effect; and other Contract Provisions submitted herein.

PROJECT NUMBER \ DESCRIPTION	NET MILES	GROSS MILES
SOIA-7-002(119)053	1.4589	1.7167
*0.0688 Mi deducted for bridges and bridge approach slabs		
0.189 Mi deducted for two exceptions		

*Average length was used since each
structure has a different length.

NOTE: All stationing is based off of PR2-WB
chain, unless otherwise noted.

Begin Project
RP 52.7334
Sta 2781+40
Sec. 8 and 17,
Twp. 156 N, Rge 97 W

End Project
RP 54.4759
Sta 2872+04.39
Sec 10, Twp. 156 N, Rge. 97 W

DESIGNERS

Alayna L. Gottsman /s/

Darell Arne /s/

Douglas A. Schumaker /s/

DISTRICT REVIEW

Joel M. Wilt /s/

WILLISTON DISTRICT

APPROVED DATE 3/01/13

Robert Fode /s/

OFFICE OF PROJECT DEVELOPMENT
ND DEPARTMENT OF TRANSPORTATION

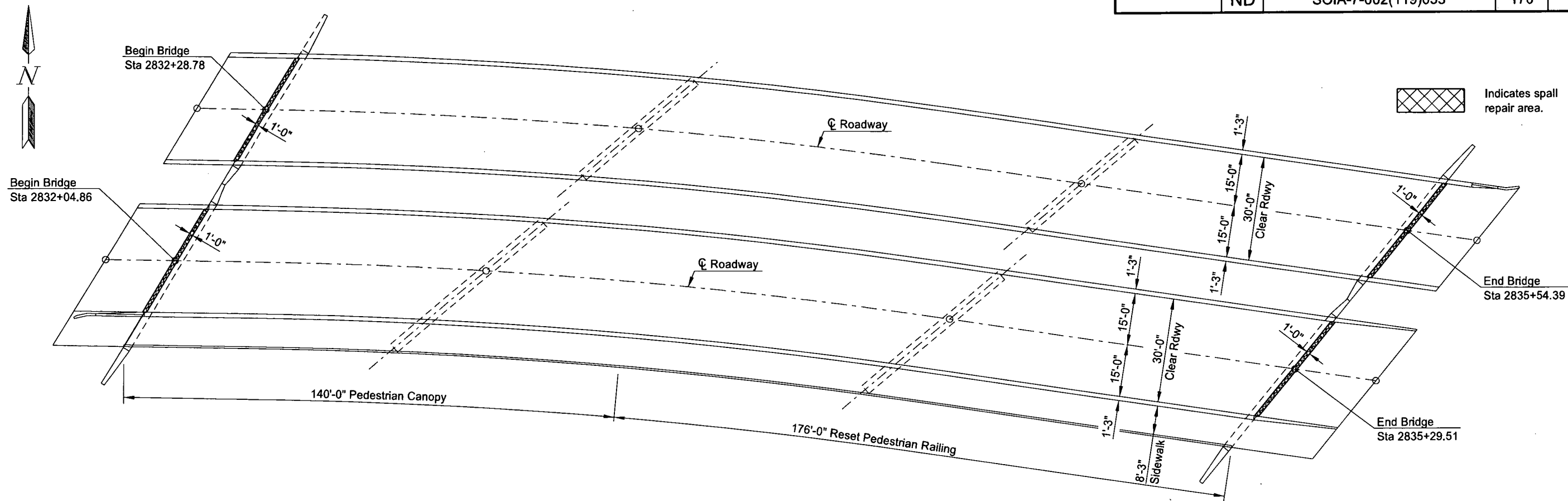
I hereby certify that the attached plans were
prepared by me or under my direct supervision
and that I am a duly registered professional
engineer under the laws of the state of ND.

APPROVED DATE 2/28/13

James Douglas Rath /s/
NDDOT DESIGN DIVISION

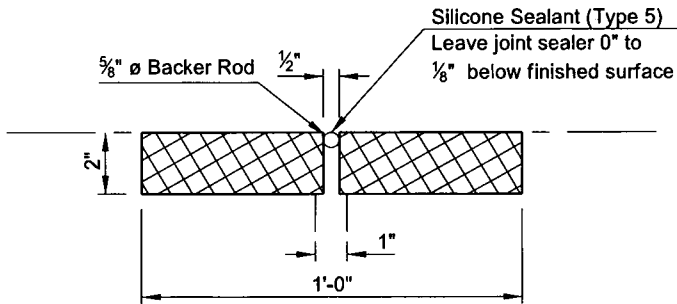
This document was originally
issued and sealed by
James Douglas Rath,
Registration Number
PE- 4288,
on 02/28/13 and the original
document is stored at the
North Dakota Department
of Transportation

STATE	PROJECT NUMBER	SECTION NO.	SHEET NO.
ND	SOIA-7-002(119)053	170	1



NOTES:

- 100 SCOPE OF WORK: Work at this site consists of repairing concrete spall areas on the bridge deck and approach slab surfaces, replacing the damaged pedestrian canopy & modifying the rest of the existing canopy.
- 930 SILICONE SEALANT: After placing the new concrete, the joint shall be cleaned of all foreign material and sandblasted before the backer rod is installed. A low modulus (Type 5) silicone sealant shall be used. All materials, labor and equipment required to remove and replace the silicone sealant shall be included in the bid item "Silicone Sealant."
- 930 DECK SPALL REPAIR: The bridge deck and approach slabs have spall areas as shown a both ends of the bridges. The deck spall repair shall be constructed as a Bridge Deck Overlay meeting Section 650 of the NDDOT Standard Specifications with the exception that a mobile mixer will not be required. The minimum depth of removal shall be 2". The perimeter of the repair area shall be saw cut to a depth of 1". The saw cutting and all material, labor and equipment required to remove the concrete and repair the deck spall areas shall be included in the bid item "Deck Spall Repair."



DECK SPALL REPAIR

BRIDGE BID ITEMS

SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
624	0121	RESET PEDESTRIAN RAILING	LF	176
624	0126	PEDESTRIAN CANOPY	LF	140
930	8644	SILICONE SEALANT	LF	140
930	9610	DECK SPALL REPAIR	SF	140

This document was originally issued and sealed by Tim L Schwagler, Registration Number PE 3151, on 02/14/13 and the original document is stored at the North Dakota Department of Transportation

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
BNSF RR OVERHEAD
RAY

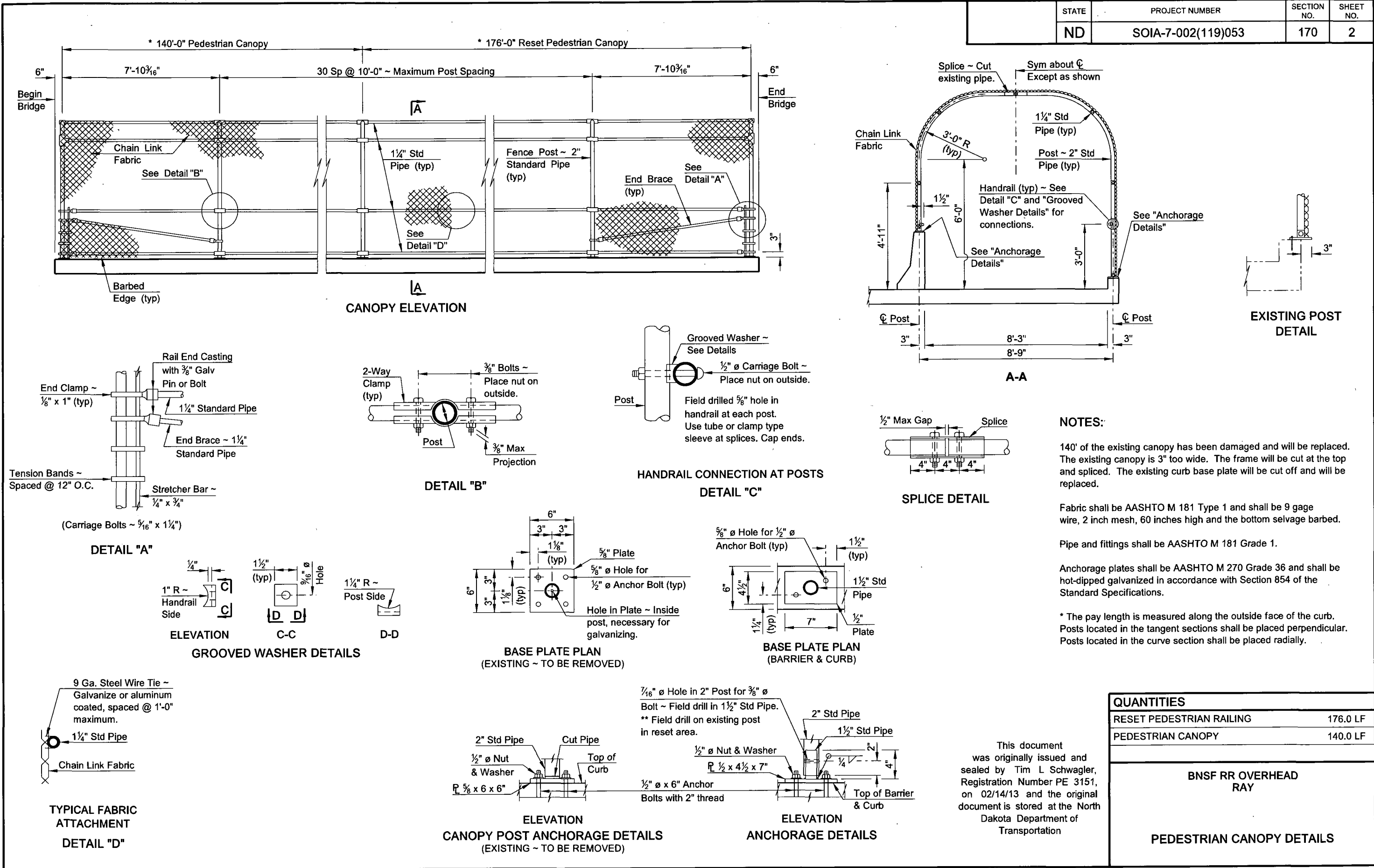
BRIDGE LAYOUT

PROJECT: SOIA-7-002(119)053

WILLIAMS COUNTY

2-19-13
DATE

Terrence R. Udland
BRIDGE ENGINEER



BNSF RR Overhead Pedestrian Canopy

PAGE INDEX

COVER PAGE

BRIDGE CANOPY LAYOUT - PAGE 1

CANOPY LAYOUT & DETAILS - PAGE 2-3

PARTS LIST & NOTES - PAGE 4

ND PROJECT # SOIA-7-002(119)053

BRIDGE # 2-053.731R

CITY OF RAY

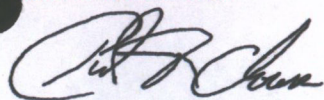
WILLIAMS COUNTY, ND

DAKOTA FENCE CO. - SUBCONTRACTOR

ACME CONCRETE PAVING INC. - PRIME CONTRACTOR

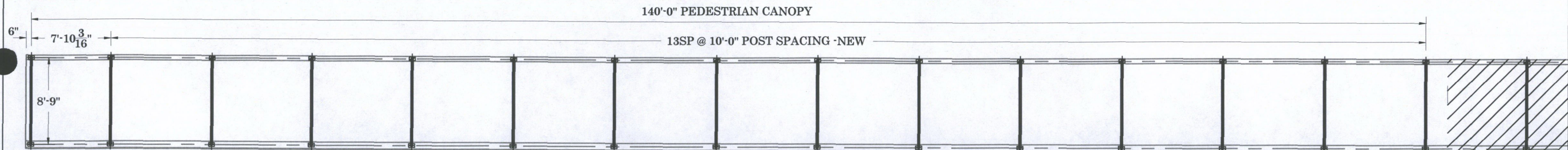
NORTH DAKOTA DEPARTMENT OF TRANSPORTATION. - ENGINEER

Shop Drawings Have
Been Reviewed and
Checked As Submitted
and Are in Conformance
with Contract Documents
Dakota Fence Co.

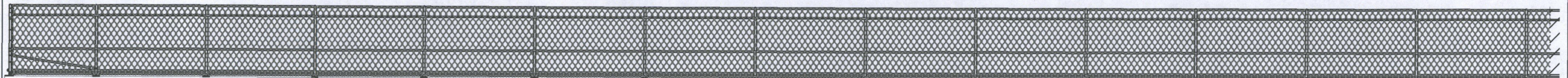


NO EXCEPTIONS NOTED	
Dean L Stolz	Digitally signed by Dean L Stolz DN: cn=Dean L Stolz, o=ND DOT, ou=Bridge Division, email=dstolz@nd.gov, c=US Date: 2013.07.03 08:13:24 -05'00'
REVIEW DOES NOT RELIEVE CONTRACTOR FROM RESPONSIBILITY FOR ERRORS, OMISSIONS, OR DEVIATIONS FROM CONTRACT REQUIREMENTS	
NDDOT	

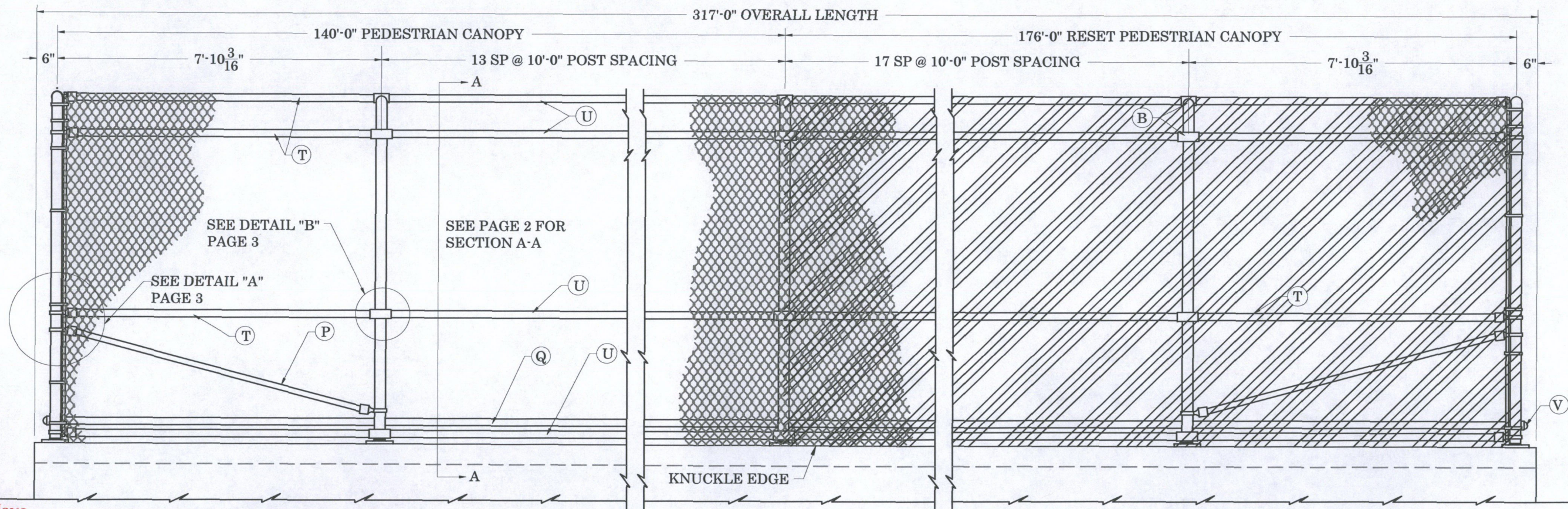
5/30/2013



PEDESTRIAN CANOPY LAYOUT



PEDESTRIAN CANOPY



PEDESTRIAN CANOPY ELEVATION

Shop Drawings Have
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and Are in Conformance
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Dakota Fence Co.

No Exceptions Noted

BY: D Stolz

DATE: July 3, 2013

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NDDOT

PROJECT# 13A4211	DRAWING# 13.057	SCALE: VARIES	REVISED: 5/30/13 NJS
DATE: 6/17/13	DRAWN BY: NJS		
ALL VERIFIES WILL BE ASSUMED CORRECT AS SHOWN UNLESS NOTED OTHERWISE BY APPROVER			
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ND PROJECT # SOIA-7-002(119)053 BRIDGE # 2-053.731R
BNSF RR OVERHEAD PEDESTRIAN CANOPY
WILLIAMS COUNTY, ND

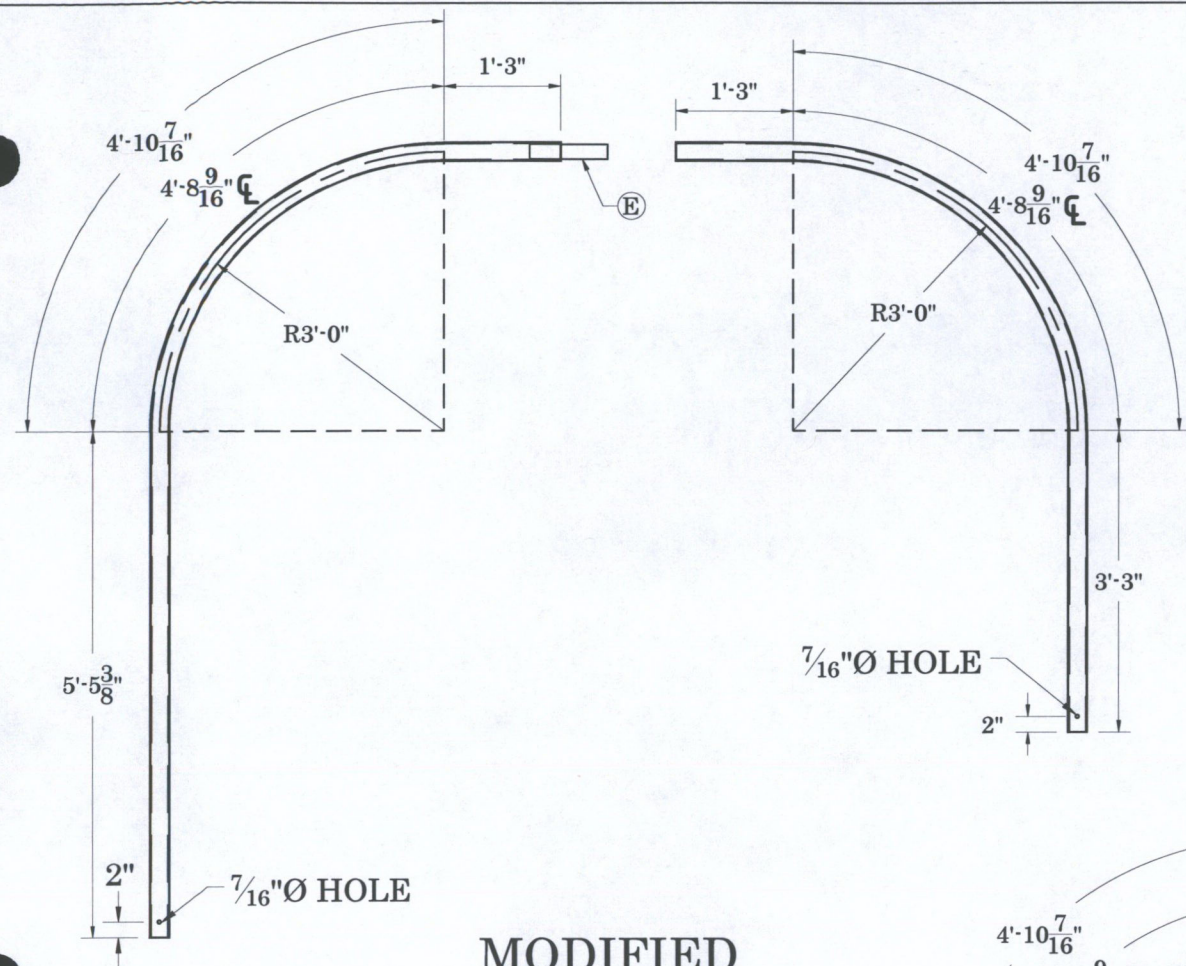
ACME CONCRETE PAVING INC.>>PRIME CONTRACTOR
DAKOTA FENCE CO.>> SUB-CONTRACTOR
NDDOT>>ENGINEER

DUE TO SHOP PROCESS, SPLICES MAY OCCUR OTHER THAN SHOWN

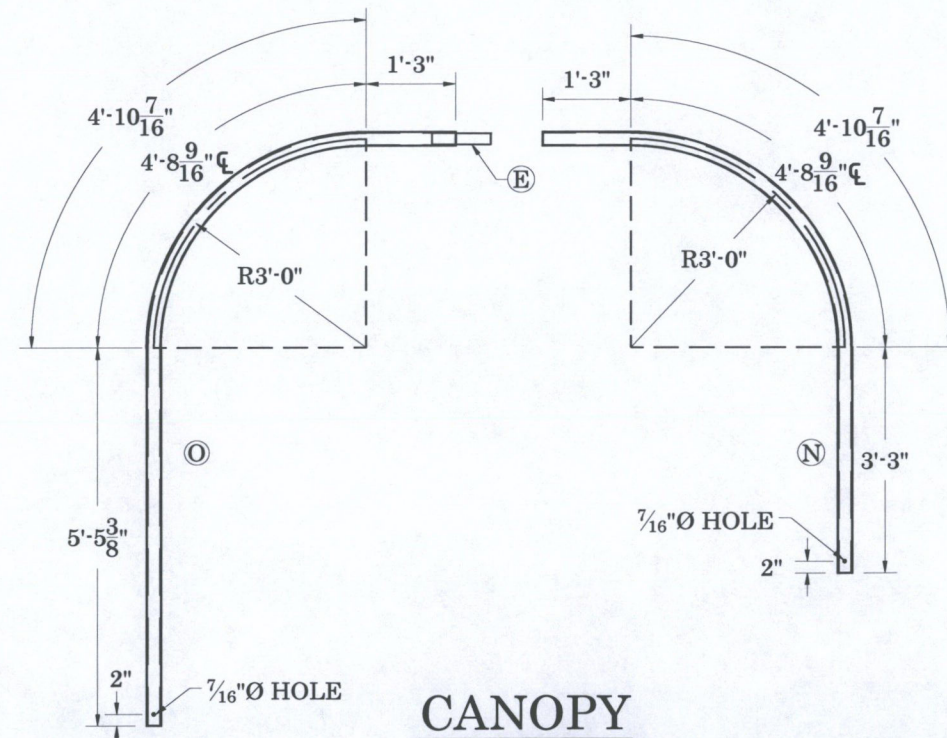
PAGE:
1 of 4

Dakota Fence.
FARGO, BISMARCK,
MINOT, WILLISTON

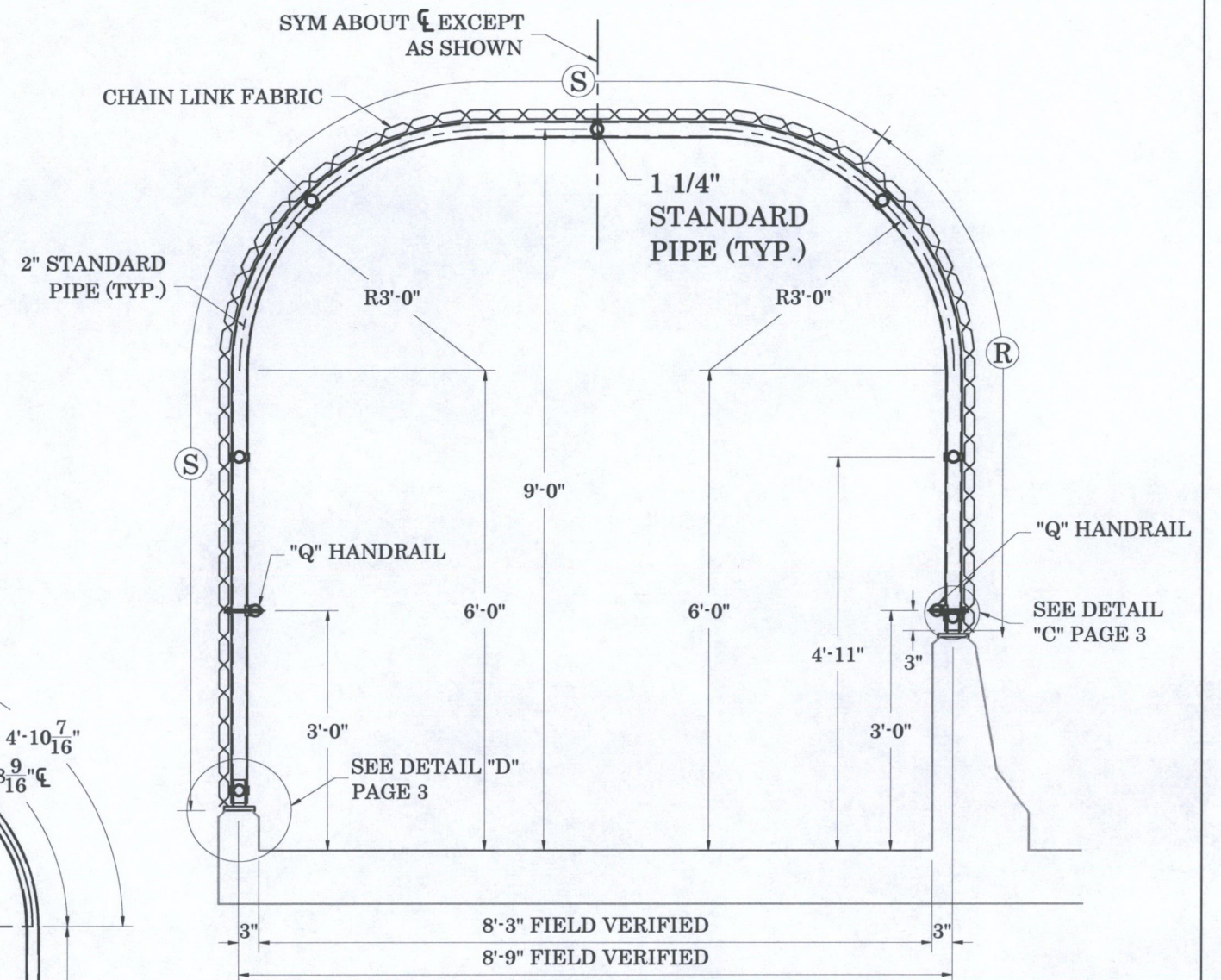
1110 25th Ave N.
BOX 1408
Fargo, ND 58107
Phone: (701) 237-6181
Fax: (701) 293-7811
1720 12th St. S.
BOX 62
Bismarck, ND 58502
Phone: (701) 258-9095
Fax: (701) 223-3140
1915 20th Ave. SE.
BOX 3066
Minot, ND 58702
Phone: (701) 852-6263
Fax: (701) 852-0931
6407 138th Ave. NW.
Williston, ND 58802
Phone: (701) 826-6264



**MODIFIED
CANOPY PIPE
DETAIL
(RESET)**



**CANOPY
PIPE DETAIL
(NEW)**



SECTION - A-A

No Exceptions Noted

BY: D Stolz

DATE: July 3, 2013

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NDDOT

Shop Drawings Have
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Dakota Fence Co.

[Signature]

PROJECT# 13A4211	DRAWING# 13.057	SCALE: VARIES	REVISED: 5/30/13 NJS
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ND PROJECT # SOIA-7-002(119)053 BRIDGE # 2-053.731R
BNSF RR OVERHEAD PEDESTRIAN CANOPY
WILLIAMS COUNTY, ND
ACME CONCRETE PAVING INC.>>PRIME CONTRACTOR
DAKOTA FENCE CO.>> SUB-CONTRACTOR
NDDOT>>ENGINEER

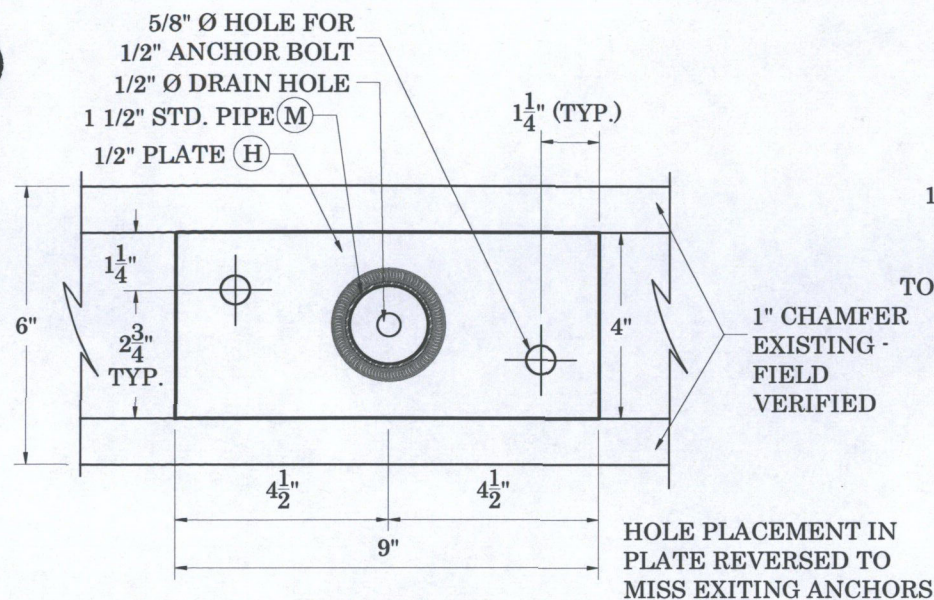
Dakota Fence.
FARGO, BISMARCK,
MINOT, WILLISTON

1110 25th Ave N.
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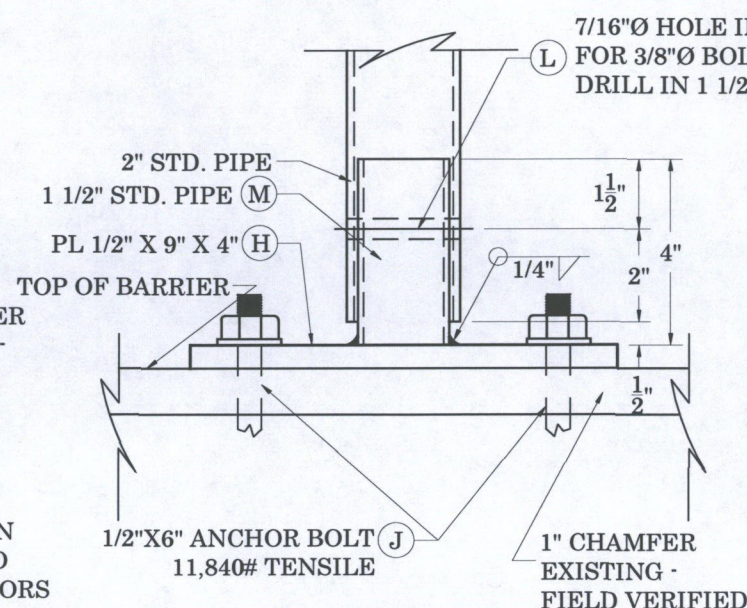
6407 138th Ave. NW.
Williston, ND 58802
Phone: (701) 826-6264



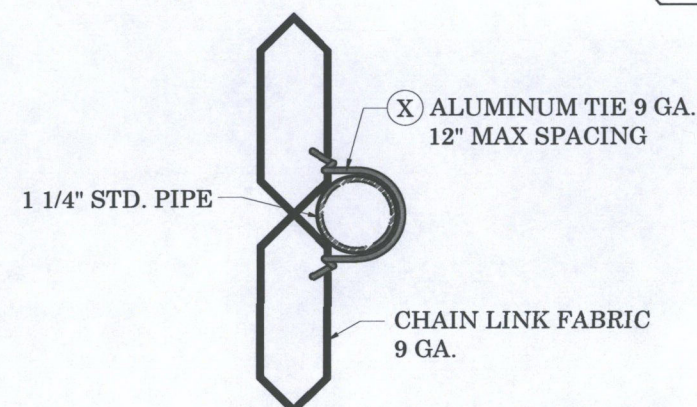
PLAN

CURB / BARRIER POST ANCHOR DETAILS

DETAIL PROPOSED AS PER DAKOTA FENCE

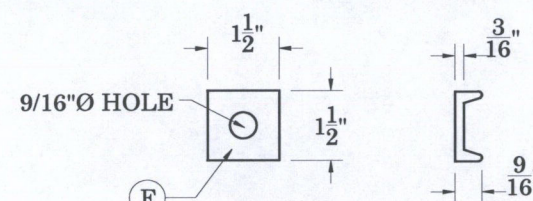


ELEVATION

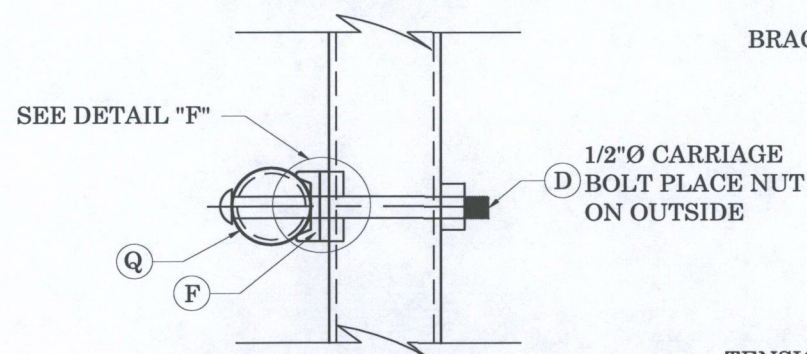


TYP. FABRIC ATTACHMENT

DETAIL "E"



DETAIL - F

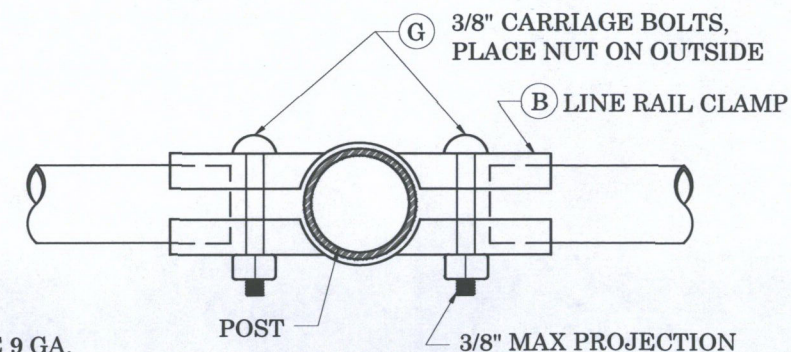


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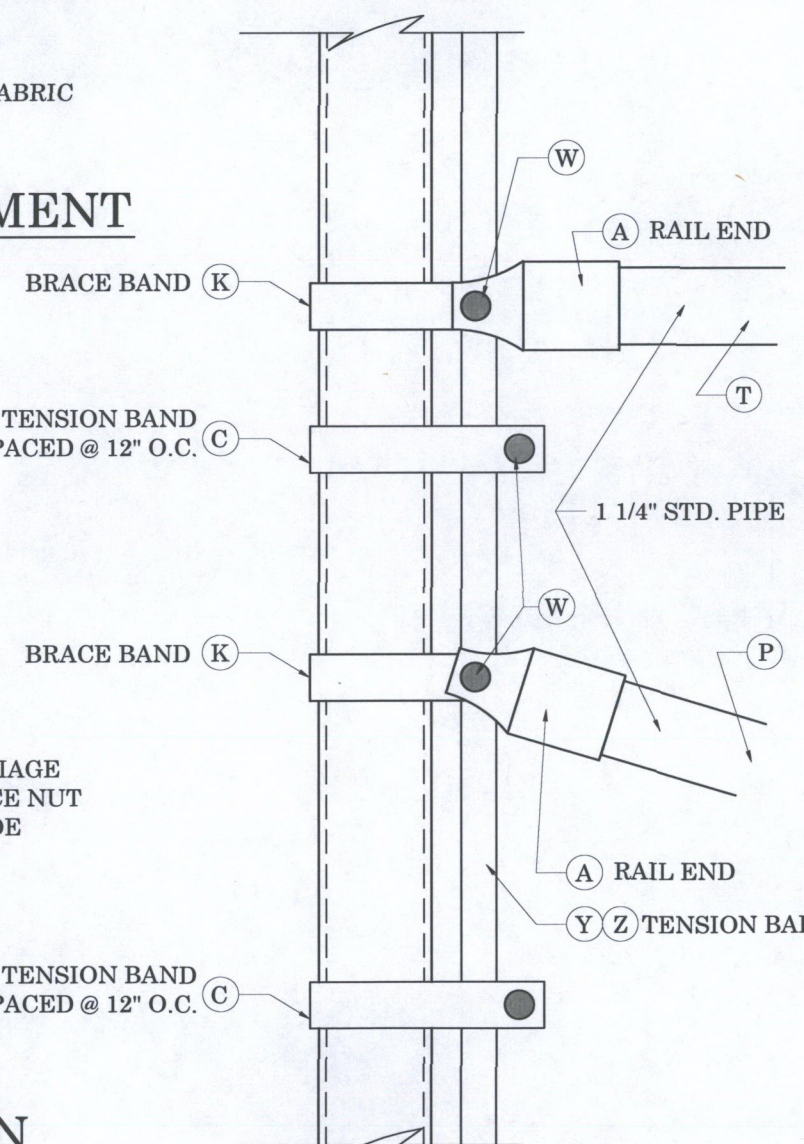
PROVIDE 5/8"Ø HOLES IN HANDRAIL AT EACH POST. USE TUBE AT SPLICES. CAP ENDS

HANDRAIL CONNECTION

AT POST DETAIL "C"

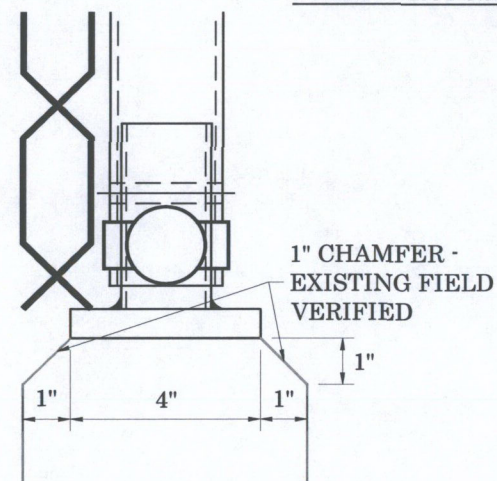


DETAIL - B



DETAIL - A

TYP. WALL TOP FINISH DETAIL "D"



Shop Drawings Have
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Dakota Fence Co.

[Signature]

No Exceptions Noted			
BY: D Stolz			
DATE: July 3, 2013			
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NDDOT			

PROJECT# 13A4211	DRAWING# 13.057	SCALE: VARIES	REVISED:
DATE: 5/30/13	DRAWN BY: NJS		
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ND PROJECT # SOIA-7-002(119)053 BRIDGE # 2-053.731R
BNSF RR OVERHEAD PEDESTRIAN CANOPY
WILLIAMS COUNTY, ND
ACME CONCRETE PAVING INC.>>PRIME CONTRACTOR
DAKOTA FENCE CO.>> SUB-CONTRACTOR
NDDOT>>ENGINEER

Dakota Fence.	
FARGO, BISMARCK, MINOT, WILLISTON	
1110 25th Ave. N. BOX 1408 Fargo, ND 58107	Phone: (701) 237-6181 Fax: (701) 293-7811
1720 12th St. S. BOX 62 Bismarck, ND 58502	Phone: (701) 258-9095 Fax: (701) 223-3140
1915 20th Ave. SE. BOX 3066 Minot, ND 58702	Phone: (701) 852-6263 Fax: (701) 852-0931
6407 138th Ave. NW. Williston, ND 58802	Phone: (701) 826-6264

PARTS LIST				
PART	QUANT.	LENGTH	DESCRIPTION	MATERIAL
A	11	-	1 5/8" RAIL END	FENCE FITTING GALVANIZED
B	98	-	2 3/8" X 1 5/8" LINE RAIL CLAMP	FENCE FITTING GALVANIZED
C	22	2 1/2"	TENSION BANDS	FENCE FITTING GALVANIZED
D	30	5 1/2"	1/2"Ø CARRIAGE BOLTS	GR-2 GALVANIZED
E	15	10"	1 1/2" X PIPE SLEEVE	STD. PIPE GALVANIZED
F	60	-	C-CHANNEL SUPPORTS	A-36 GALVANIZED
G	196	2 1/4"	3/8"Ø X CARRIAGE BOLTS/NUTS	GR-2 GALVANIZED
H	48	9"	4" X 1/2" BASE PLATES	A-36 GALVANIZED
J	96	6"	1/2"Ø ANCHOR BOLTS/NUTS/FLAT WASHERS	GR-2 GALVANIZED
K	11	2 1/2"	BRACE BANDS	FENCE FITTING GALVANIZED
L	48	3"	3/8"Ø BOLT W/ NUT	GR-2 GALVANIZED
M	48	4"	1 1/2" PIPE INSERT	STD. PIPE GALVANIZED
N	15	9'-2 9/16"	2 3/8" O.D. CANOPY PIPE	STD. PIPE GALVANIZED
O	15	11'-4 15/16"	2 3/8" O.D. CANOPY PIPE	STD. PIPE GALVANIZED
P	2	7'-3" ±	1 5/8" O.D. ANGLE PIPE BRACE	STD. PIPE GALVANIZED
Q	2	140'-4"	1 5/8" O.D. HANDRAIL	STD. PIPE GALVANIZED
R	140'-0"	6'-0"	9 GAUGE CHAIN LINK KNUCKLE/KNUCKLE	FABRIC GALVANIZED
S	280'-0"	8'-0"	9 GAUGE CHAIN LINK KNUCKLE/KNUCKLE	FABRIC GALVANIZED
T	7	7'-4" ±	1 5/8" O.D. HORIZONTAL BRACE PIPES	STD. PIPE GALVANIZED
U	98	9'-7 5/8" ±	1 5/8" O.D. HORIZONTAL BRACE PIPES	STD. PIPE GALVANIZED
V	2	-	1 5/8" DOME CAPS	FENCE FITTING GALVANIZED
W	33	1 1/4"	5/16" CARRIAGE BOLTS/NUTS	GR-2 GALVANIZED
X	1340	-	9 GAUGE ALUMINUM FENCE TIES	FENCE FITTING GALVANIZED
Y	1	6'-0"	TENSION BARS	FENCE FITTING GALVANIZED
Z	2	8'-0"	TENSION BARS	FENCE FITTING GALVANIZED

NOTES: CANOPY

- FABRIC SHALL BE AASHTO M181 TYPE 1, AND SHALL BE 9 GA. WIRE, 2" MESH.
- POST AND FITTINGS SHALL BE AASHTO M 181 GRADE 1.
- ANCHORAGE PLATES SHALL BE AASHTO M 270 GR. 36 & SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH SECTION 854 OF THE STANDARD SPECIFICATIONS.

Shop Drawings Have
Been Reviewed and
Checked As Submitted
and Are in Conformance
with Contract Documents
Dakota Fence Co.

PROJECT# 13A4211	DRAWING# 13.057	SCALE: VARIES	REVISED:
DATE: 5/30/13	DRAWN BY: NJS		
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ND PROJECT # SOIA-7-002(119)053 BRIDGE # 2-053.731R
BNSF RR OVERHEAD PEDESTRIAN CANOPY
WILLIAMS COUNTY, ND

ACME CONCRETE PAVING INC.>>PRIME CONTRACTOR
DAKOTA FENCE CO.>> SUB-CONTRACTOR
NDDOT>>ENGINEER

DUE TO SHOP PROCESS, SPLICES MAY OCCUR OTHER THAN SHOWN		PAGE: 4 of 4
Dakota Fence.		
1110 25th Ave N. BOX 1408 Fargo, ND 58107	Phone:(701) 237-6181 Fax: (701) 293-7811	
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No Exceptions Noted

BY: D Stolz

DATE: July 3, 2013

REVIEW DOES NOT RELIEVE CONTRACTOR FROM
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DEVIATIONS FROM CONTRACT REQUIREMENTS

NDDOT

DESIGN DATA				
Traffic	Average Daily			Est. Max. Hr.
Current 1993	Pass: 1700	Trucks 225	Total 1925	192
Forecast 2013	Pass: 1700	Trucks 275	Total 2100	210
Minimum Sight Dist. for:		Design Speed 40 MPH.		
Stopping 275 FT.		Bridges		
Safe Passing				
Passing for Marking				

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION

WILLIAMS COUNTY
FEDERAL AID PROJECT NO.
RSN-NH-7-002(041)052
Grading, Surfacing, Storm drains, Structural
Signing, Marking, Guardrail & Incidentals

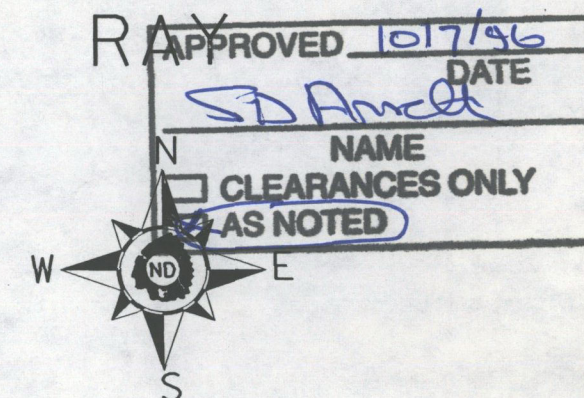
FHWA REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	RSN-NH-7-002(041)052	1

GOVERNING SPECIFICATIONS:

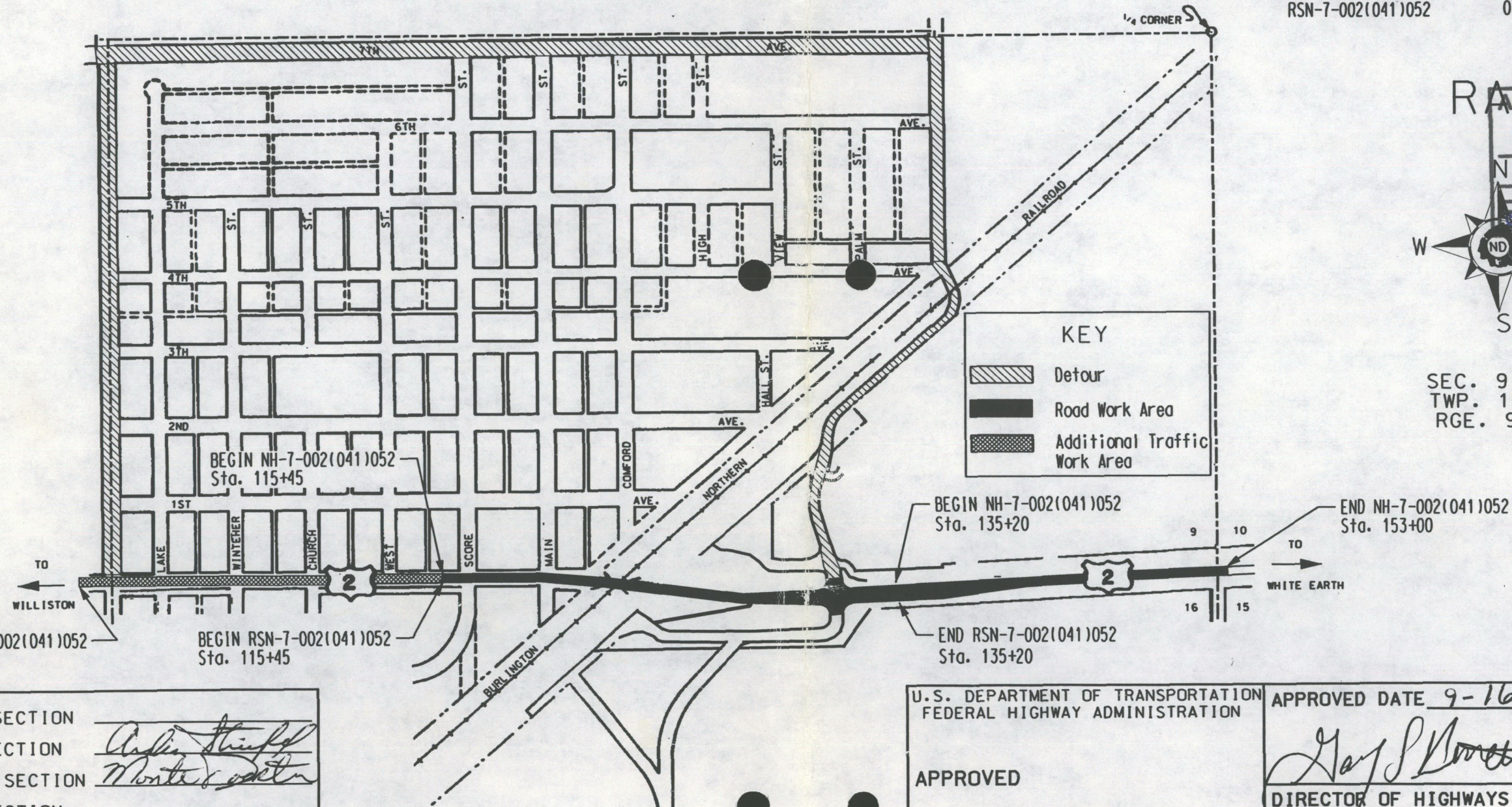
Standard Specifications adopted by the North Dakota Department of Transportation September 1992: Standard Drawings currently in effect; and other Contract Provisions submitted herein.

LENGTH OF PROJECT

Project	Miles-Gross	Miles-Net
NH-7-002(041)052	0.6794	0.6794
RSN-7-002(041)052	0.3741	0.3741



SEC. 9 & 16
TWP. 156 N.
RGE. 97 W.



BEGIN NH-7-002(041)052
Sta. 97+38

BEGIN RSN-7-002(041)052
Sta. 115+45

BEGIN NH-7-002(041)052
Sta. 135+20

END RSN-7-002(041)052
Sta. 135+20

END NH-7-002(041)052
Sta. 153+00

PAVING SECTION
URBAN SECTION
TRAFFIC SECTION
RURAL SECTION
RECOMMEND APPROVAL 9-16-96
DESIGN ENGINEER

U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

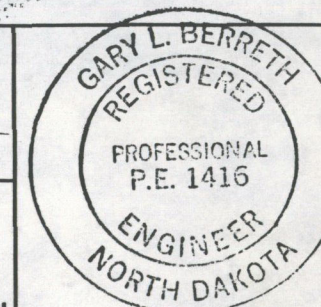
APPROVED

DIVISION ADMINISTRATOR DATE

APPROVED DATE 9-16-96

DIRECTOR OF HIGHWAYS
AND ENGINEERING

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION



FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	RSN-7-002(041)052	125

BRIDGE NOTES

- 100 SCOPE OF WORK: This project consists of building a new 3-span steel girder bridge 320' long with clear roadway width of 30' and an 8'-9" sidewalk. Part of the new structure is on a tangent alignment and part is on a curve.
- 100 GENERAL: The cost of furnishing and placing preformed expansion joint filler, concrete inserts, tie wire, bar spacers, bar supports, and other miscellaneous items shall be included in the price bid for Class AE-3 and AAE-3 concrete.
- 105 The existing structural steel is painted with lead-based paint. Certain contractor operations could expose employees to hazardous levels of lead. The contractor shall plan accordingly and shall inform employees of the hazards of lead-based paint.
- 202 REMOVAL OF STRUCTURE: The existing structure is an underpass 107' long with a clear roadway of 30 feet. The substructures are made of concrete, and the superstructure is made of steel. The contractor may have to remove portions of the existing wingwall footings and existing retaining walls to allow for pile driving. The contractor shall remove a minimum amount of material from the wingwall footings. The contractor shall place compacted fill in the existing underpass up to an elevation 6' below the bottom of the steel beams. The contractor shall entirely fill the pedestrian tunnels and the voids under the sidewalks of the existing structure with either earth or flowable fill. Burlington Northern (BN) Railroad will remove the superstructure and portions of the substructure. BN Railroad will place the removed material on the railroad right of way at a location agreed to by BN Railroad and the engineer. All materials removed by the contractor and BN Railroad shall become the property of the contractor and shall be disposed of properly off the right of way.
- 203 EMBANKMENT: The embankment at the abutments shall be in place for a minimum of 30 days before piling are driven.
- The contractor will be required to bore through the fill at the abutments before driving piling. All pilot holes not completely filled by the piles shall be backfilled with sand or fine gravel before the substructure is placed.
- In lieu of the above waiting period, the contractor may begin pile driving operations as soon as the embankment is in place, but all pilot holes shall be extended to a depth of 15 feet into the original ground.
- 210 EXCAVATION: The excavation at the abutments, as shown, and the excavation required to build the piers shall be included in the lump sum bid item, "Class 1 Excavation, Site 2."

- 210 SELECT BACKFILL: Select backfill shall meet the requirements of Section 816.03, Class 3. The backfill shall be placed in layers of not more than 6 inches, moistened or dried as required, and thoroughly compacted with mechanical tamping equipment.
- 550 BRIDGE APPROACH SLABS: Mechanical finishing of the approach slabs shall be required. A mechanical or hand-held transverse metal tine finish shall be applied. Tining shall start 6" from the beginning and end of the approach slabs. A surface tolerance of 3/16" in 10 feet is also required.
- 602 Exterior beams shall be braced to prevent rotation. The strength and spacing of the bracing shall be dependent on the forces induced by the weight of the concrete, forms, equipment, and workers.
- 602 END BEAMS: The end beam concrete shall be placed before the deck concrete. The concrete shall cure for at least 72 hours before deck placement.
- 602 SURFACE FINISH "D": Surface Finish "D" shall be required for the inside, outside, and top surfaces of the barrier, and on the outside edge of the deck.
- 602 DECK CONCRETE: Beams and girders have slight variations in the anticipated camber. To build the deck to the designated thickness will require slight adjustments in deck elevation and/or riser dimensions. These adjustments result in minor concrete quantity discrepancies. The contractor shall consider this quantity discrepancy when he bids the unit price for Class AAE-3 Concrete. The Department will pay plan quantity of Class AAE-3 Concrete.
- 602 Deflection of the deck shoring shall be computed using the total dead load plus the weight of the finishing machine. The forming shall be adjusted properly to accommodate the deflection and thereby maintain the total slab thickness specified in the plans.
- 602 PENETRATING WATER REPELLENT TREATMENT: Penetrating water repellent shall be applied to the driving surface of the concrete deck.
- 602 BARRIERS: Barriers shall be constructed according to the provisions of Section 602.03 B.4 except that there shall be no expansion or deflection joints. Make 3/4" V-grooves in all faces of the barriers at each pier and at equal spaces between substructures at approximately 10-foot spacing.

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BRIDGE NOTES

- 602 DECK TINING: Tining shall begin 6 inches from the beginning and the end of the deck and 6 inches from each deck joint.
- 602 SIDEWALK BROOMING: The sidewalk on the bridge shall not be tined. The sidewalk shall be transversely broomed to slightly roughen the surface.
- 612 REINFORCING STEEL: Dimensions for bent bars are given out to out and to tangent intersections unless otherwise noted.
- 612 The bar fabricator shall add a prefix to all bar designations to differentiate between the different structures on this project.
- 612 All reinforcing steel shall be Grade 60.
- 616 STRUCTURAL STEEL: Structural steel shall be AASHTO M 270, Grade 50T2, except the diaphragms and swedge bolts may be grade 36T2. The requirement for Charpy V-Notch is waived for the bearings, diaphragms, and diaphragm connection material.
- 616 Shear connectors on splice plates shall be moved to clear bolt holes.
- 616 Field connections shall be made with 7/8 inch diameter, AASHTO M 164 high-strength bolts unless otherwise shown.
- 616 Temporary or permanent attachments or devices that are not shown on the plans as part of the structure shall not be welded to the structural steel members during the fabrication and construction process.
- 616 The cost of swedge bolts shall be included in the total cost of structural steel.
- 622 PILING: Piling shall be driven with a steam, air, or diesel hammer with a rated energy and ram weight not less than 55,154 foot-pound-tons, as computed by the formula $W(E-13,444)+.821E$, where W is the weight of the ram in tons and E is the rated hammer energy. In no case shall the ram weight be less than 2700 pounds.
- 622 PILING: Cast steel pile tips (ASTM 27 steel) shall be required on all pile driven.
- 630 PAINT AND PAINTING: The structural steel shall be painted according to the specifications. The finish coats shall be blue, color number 25240, and shall meet Federal Standard No. 595B colors.

- 708 SLOPE PROTECTION: The concrete slope protection will be limited to the cast-in-place type shown on Standard D-708-1.

SHOP DRAWINGS: CAD-generated shop drawings may be submitted on 11-inch by 17-inch detail sheets. The contractor shall submit the following shop drawings to the Construction office for approval:

1. Structural Steel.
2. Preformed Elastomeric Neoprene Expansion Joint.

DESIGN STRENGTH: F'C 3,000 PSI Cl. AE-3 Concrete
F'C 4,000 PSI Cl. AAE-3 Concrete
FY 36,000 PSI Structural Steel M270
Grade 36T2
FY 50,000 PSI Structural Steel M270
Grade 50T2
FY 60,000 PSI Grade 60 Reinforcing Steel
Load Factor Design

- 930 PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT: The neoprene joint shall have a profile that allows thermal cycles and dynamic loading up to a total range of movement of two inches while providing watertight protection. The neoprene profile shall be bonded to the concrete surfaces with an epoxy adhesive that is chemically resistant to oxidation and corrosion, is impervious to weather extremes and is compatible with the nosing concrete. During installation, the neoprene profile shall be pressurized with compressed air to maximize the adhesion against the gap walls. After curing, the air pressure shall be permanently released. This shall be a Jeene Structural Sealing Joint System (2 W) or an approved equal.

- 930 NOSING CONCRETE: The nosing concrete material shall be an elastomeric concrete or a polymeric concrete that will provide a durable edge that can withstand live-load traffic without chipping or spalling. The nosing concrete material shall be Ceva Crete 95, manufactured by E-Poxy Industries Inc.; Elastomeric Concrete, as manufactured by the D.S. Brown Company; Silspec 900, as manufactured by Construction Materials Inc.; Elastomeric Concrete as manufactured by Harris Speciality Chemicals Inc., or an approved equal. The nosing shall be mixed and installed according to the manufacturer's recommendations. The cost for the equipment, materials, and labor to install the nosing concrete on each side of the joint shall be included in the lineal foot bid item "Nosing Concrete."

- 930 ROADWAY CANOPY: The contractor shall construct a canopy above the railroad under the structure to protect traffic from falling material. The canopy is an added safeguard and does not relieve the contractor of any responsibility

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BRIDGE NOTES

for the safety of the public.

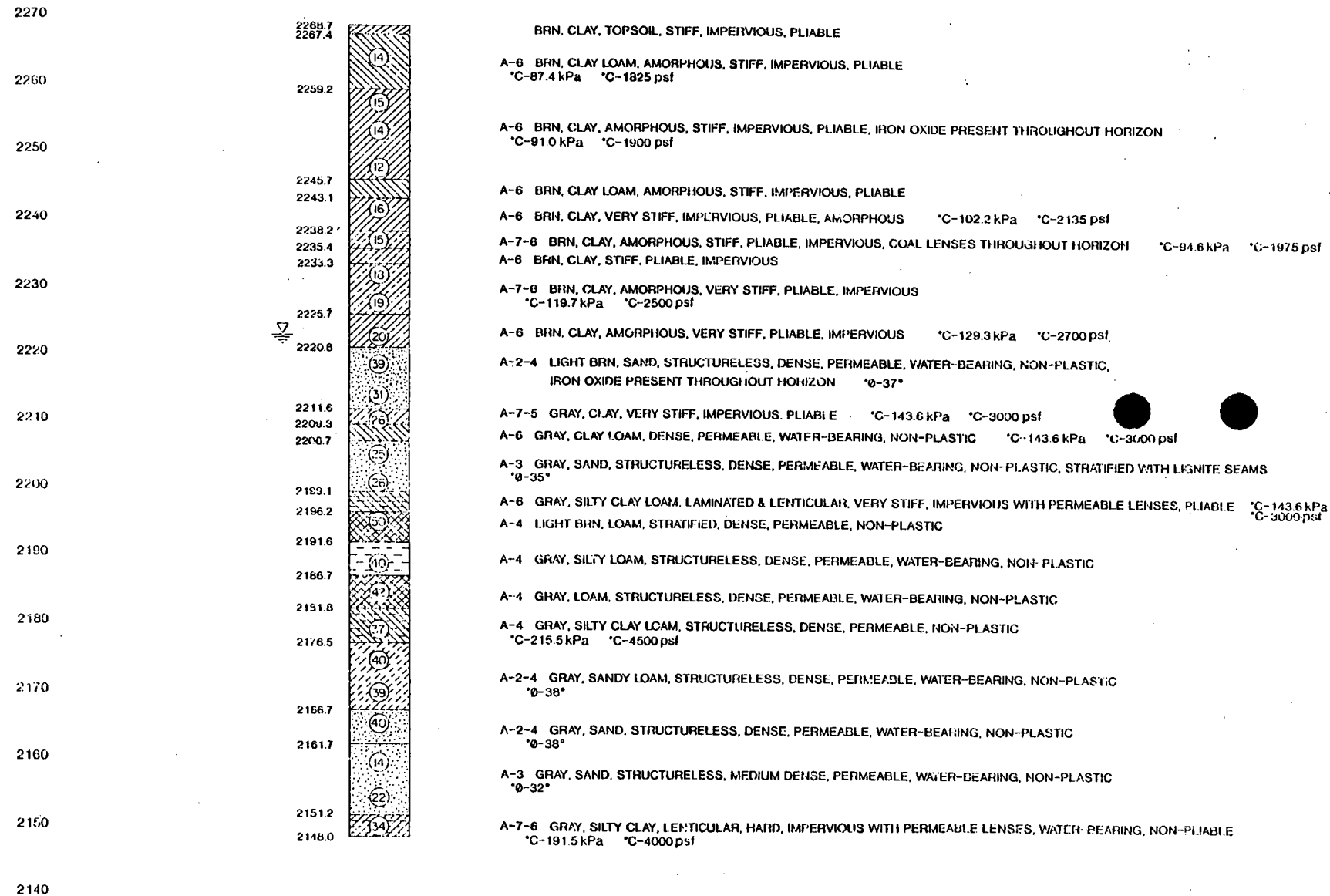
The canopy may be supported from the ground or suspended from the girders. The erection of the canopy and the girder shall be completed in a minimum amount of time and with the least inconvenience to the public and the railroad.

The canopy shall be of a design and material selected by the contractor and approved by the engineer. The minimum vertical clearance shall be 22' above the railroad tracks. The canopy shall project a minimum distance of 5'-0" beyond the outside edge of curb of the proposed structure.

The canopy shall project a minimum distance of 5'-0" beyond the edge of the driving lanes beneath the structure and 5'-0" beyond the edge of the railroad tracks.

After completion of the structure, the canopy shall be removed and shall remain the property of the contractor.

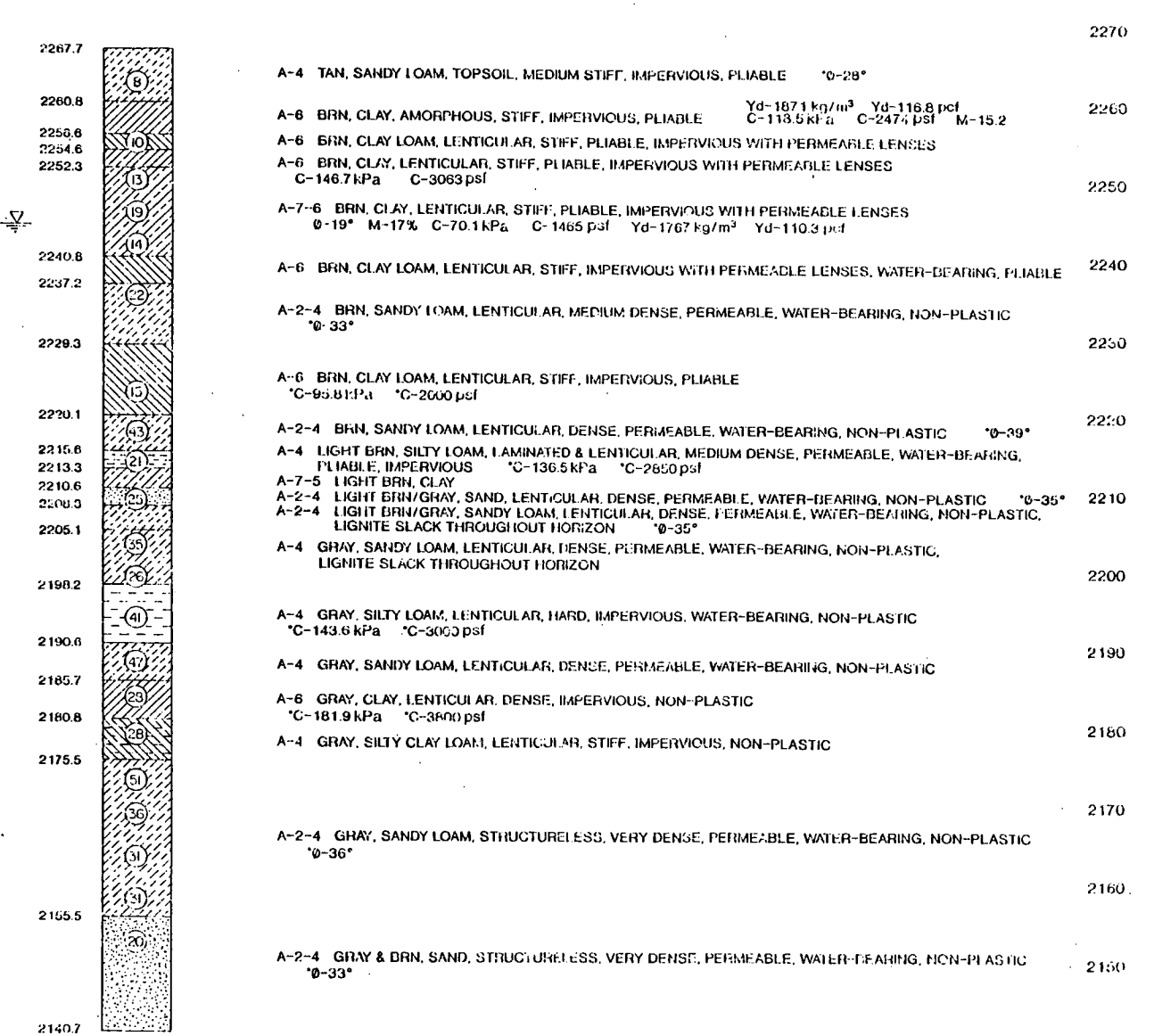
The roadway canopy shall be paid for at the contract lump sum unit price for "Roadway Canopy." The roadway canopy shall be measured as a lump sum item and shall include construction, maintenance, and removal.



BORING NO. 1

STA. 124+51.4 - RT. 66.6' FROM EXISTING CENTERLINE

NOTES: Encircled numbers indicate the number of blows delivered by a 140 lb. hammer from a height of 30" to drive a core tube 1'-0".
The boring log data shown is for design purposes only. The state assumes no responsibility if soil conditions encountered during construction differ from those shown.

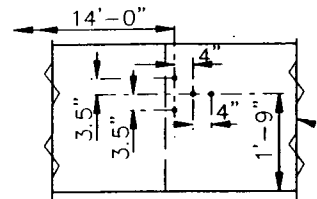


BORING NO. 2

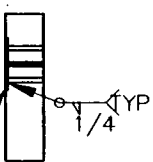
STA. 126+81.1 - LT. 51.2' FROM EXISTING CENTERLINE

SYMBOLS: 0- Angle of Internal Friction (Degrees)
C- Cohesion (lbs./Sq. Ft.) (kPa)
M- Moisture (Percent)
Yd- Dry Weight (Lbs./Cu.Ft.) (kg/Cu.m)
* - (These cohesive values & friction angles are estimated from blow count.)

WILLIAMS COUNTY
RAY OVERPASS ON HWY 2
BORING LOG



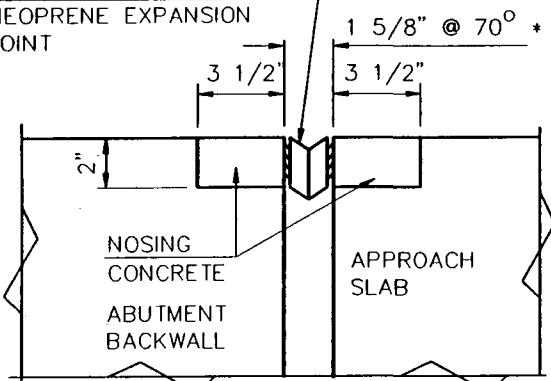
1" Ø STANDARD PIPE
WELDED TO PL 1/4" X
11 1/2" X 1'-2 1/2" GAL-
VANIZED AFTER FABRICATION



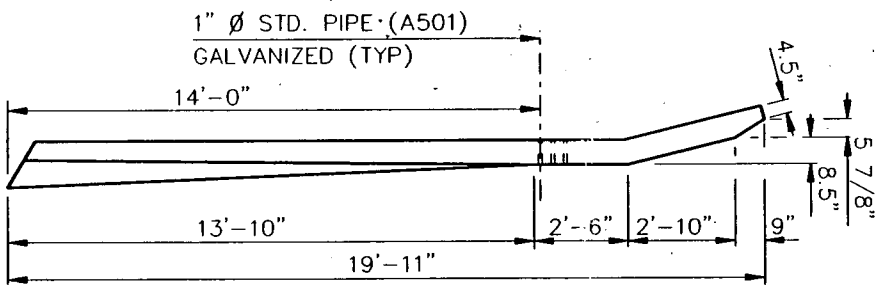
CONNECTION PLATE ASSEMBLY

PREFORMED ELASTOMERIC
NEOPRENE EXPANSION
JOINT

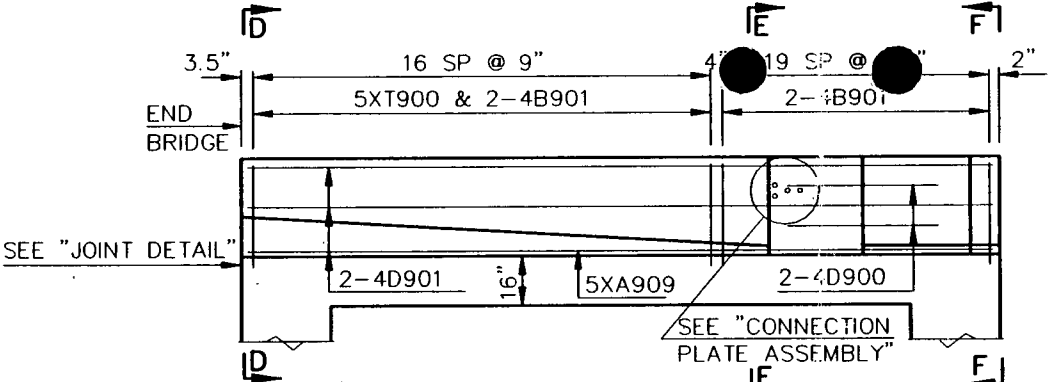
* 1/8" CHANGE PER 10°



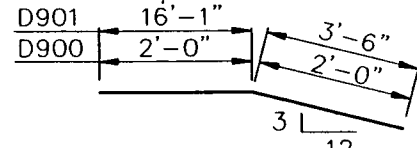
JOINT DETAIL



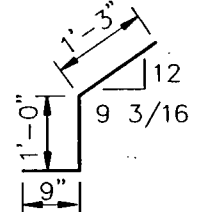
PLAN



ELEVATION



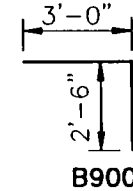
D900 & D901



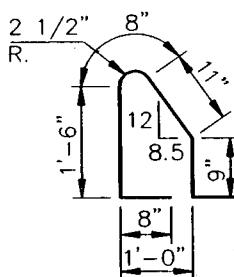
XT900
FIELD BEND
TO FIT



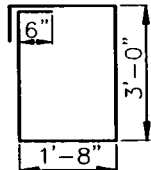
B901



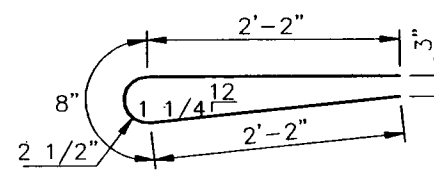
B900



K900

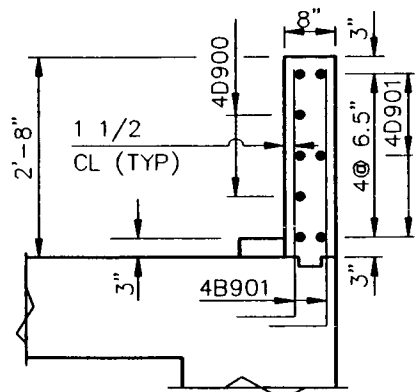


N900

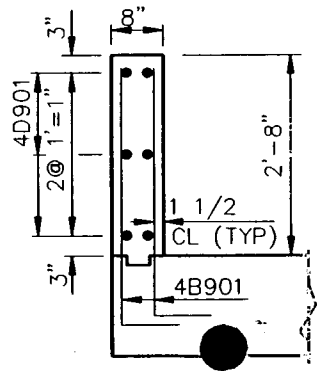


L900

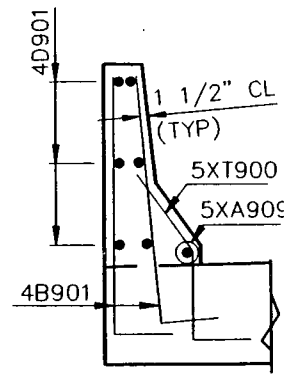
BENT BAR DETAILS
DIMENSIONS SHOWN ARE OUT TO OUT



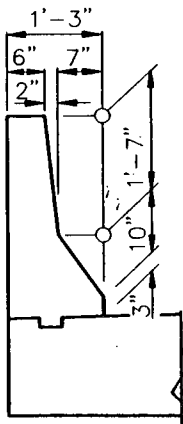
F-F



E-E



D-D



D-D

WIDTH = 30'-0" CL. RDWY.			
SKEW ANGLE = -30°			
BAR LIST - ONE SLAB			
SIZE	MARK	NO.	LENGTH
7	A900	12	37'-2"
5	A901	40	37'-2"
8	A903	59	19'-7"
5	A904	36	19'-7"
5	A905	8	6'-0"
9	A906	6	19'-7"
5	A907	5	19'-7"
5	XA908	1	19'-7"
5	XA909	1	13'-10"
5	B900	144	5'-6"
4	B901	74	4'-2"
4	D900	2	4'-0"
4	D901	6	19'-7"
5	XK900	22	5'-2"
5	L900	22	5'-0"
4	N900	56	10'-4"
5	XT900	17	3'-0"

ESTIMATED MATERIAL QUANTITIES

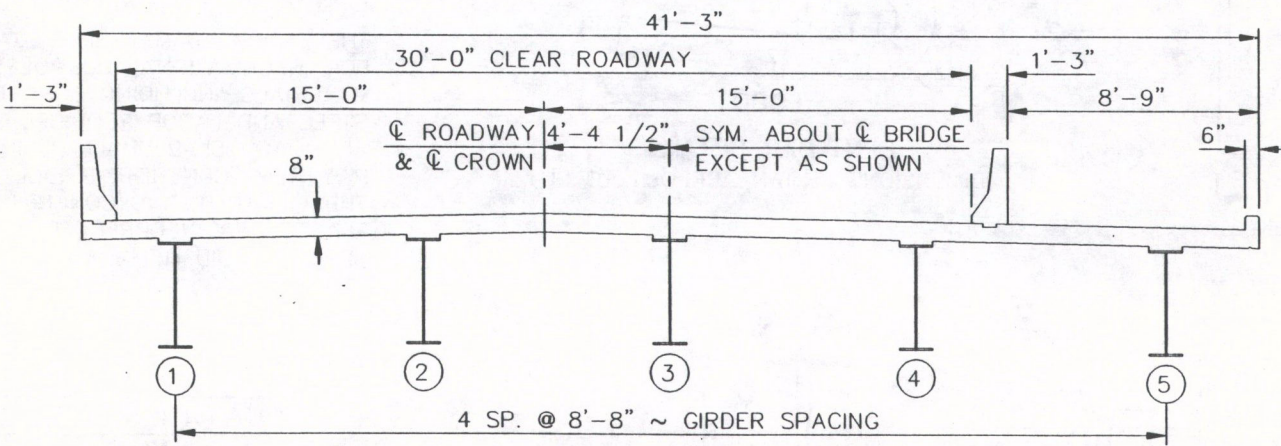
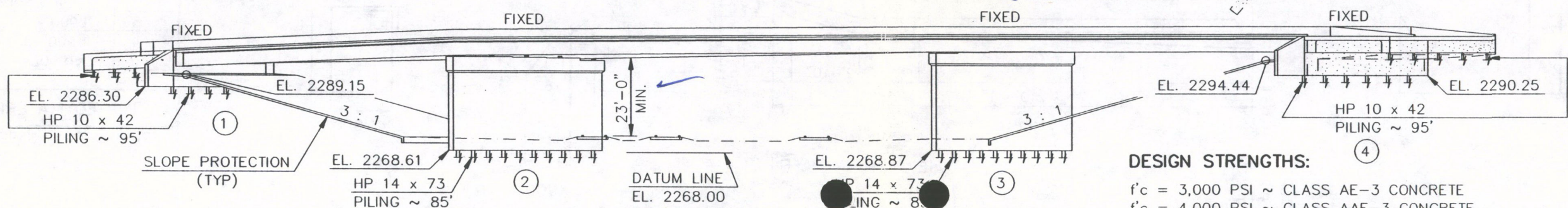
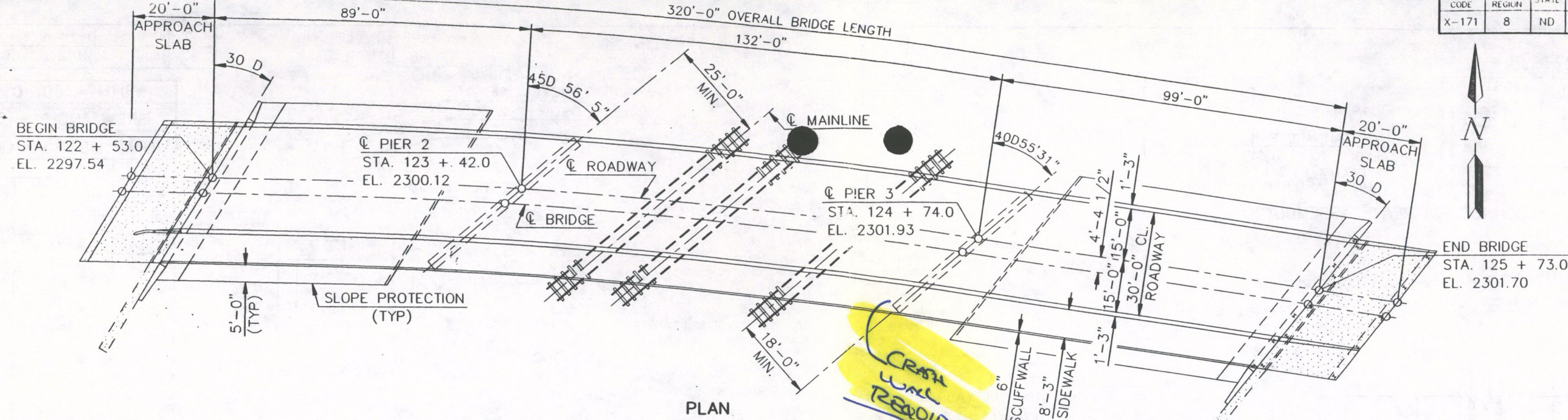
REINFORCING STEEL LBS.	CONCRETE C.Y.
8,658	47.3

X = EPOXY COATED BAR

NOTES:
THE ESTIMATED MATERIAL QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. ALL MATERIALS, INCLUDING CONCRETE, REINFORCING STEEL AND LABOR REQUIRED TO BUILD THE APPROACH SLAB SHALL BE INCLUDED IN THE PAY ITEM "CONCRETE BRIDGE APPROACH SLAB". THE PILING, NOSING CONCRETE AND PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT ARE SEPARATE BID ITEMS.

QUANTITIES	
APPROACH SLAB	72.1 SY
NOSING CONCRETE	37.5 LF
PREF. ELAST. NEO. EXP. JT.	35.6 LF

BURLINGTON NORTHERN OVERHEAD
RAY
APPROACH SLAB
END BRIDGE



DESIGN STRENGTHS:

$f'_c = 3,000$ PSI ~ CLASS AE-3 CONCRETE
 $f'_c = 4,000$ PSI ~ CLASS AAE-3 CONCRETE
 $f_y = 36,000$ PSI ~ AASHTO M-270 GR. 36 STRUCTURAL STEEL
 $f_y = 50,000$ PSI ~ AASHTO M-270 GR. 50 STRUCTURAL STEEL
 $f_y = 60,000$ PSI ~ REINFORCING STEEL

LOAD FACTOR DESIGN

SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
107	0100	RAILWAY PROTECTION INSURANCE	L. SUM	.5
202	0105	REMOVAL OF STRUCTURE	L. SUM	.5
210	0103	CLASS 1 EXCAVATION - SITE 2	L. SUM	1
210	0198	SELECT BACKFILL	TON	548
210	0203	FOUNDATION PREPARATION - SITE 2	L. SUM	1
550	0215	CONCRETE BRIDGE APPROACH SLAB	SQ. YD.	182.6
602	0130	CLASS AAE-3 CONCRETE	CU. YD.	414.8
602	1130	CLASS AE-3 CONCRETE	CU. YD.	289.1
602	1250	PENETRATING WATER REPELLENT TR.	SQ. YD.	1066.7
612	0115	REINFORCING STEEL GRADE 60	LBS.	29,549
612	0116	REINFORCING STEEL GRADE 60 EPOXY	LBS.	101,525
616	5889	STRUCTURAL STEEL SITE 2 (APPROXIMATELY 333,018 LBS.)	L. SUM	1
622	0012	STEEL H-PILE TIPS 10x42	EA.	32
622	0016	STEEL H-PILE TIPS 14x73	EA.	20
622	0020	STEEL PILING HP 10x42	L. FT.	3040
622	0060	STEEL PILING HP 14x73	L. FT.	1700
624	0126	PEDESTRIAN CANOPY	L. FT.	316.7
708	1100	SLOPE PROTECTION, CONC.	SQ. YD.	1009
930	3000	BRIDGE BENCH MARKS	SET	.5
930	3000	ROADWAY CANOPY - SITE 2	L. SUM	1
930	3000	NOSING CONCRETE	L. FT.	95.2
930	8650	PREFORMED ELASTOMERIC NEOPRENE EXP. JT.	L. FT.	93.4

BENCH MARKS

NO.	DESCRIPTION	LOCATION	ELEVATION
BH7	RIVET ON CONCRETE	STA. 123 + 83 - .66' LT.	2270.92
I	TOP OF FIRE HYDRANT	STA. 125 + 53 - 239' RT.	2268.40



STANDARD DRAWINGS

D-900-1, D-708-1, D-622-1

F.W.S. 15 PSF

HS 25 DESIGN LOADING

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION

BURLINGTON NORTHERN OVERHEAD
RAY

BRIDGE LAYOUT

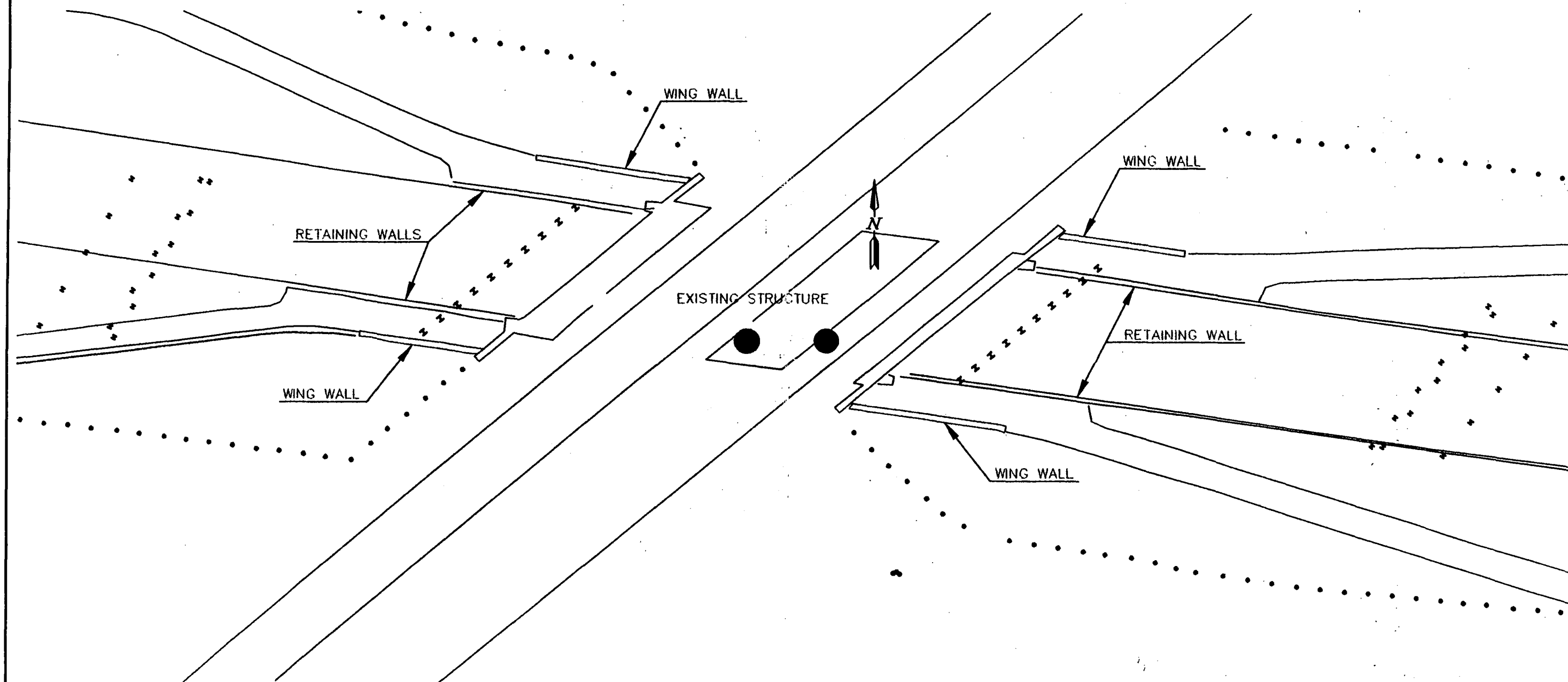
PROJECT: RSN-7-002(041)052
STATION 124 + 13.0

WILLIAMS COUNTY

19SEP96
DATE

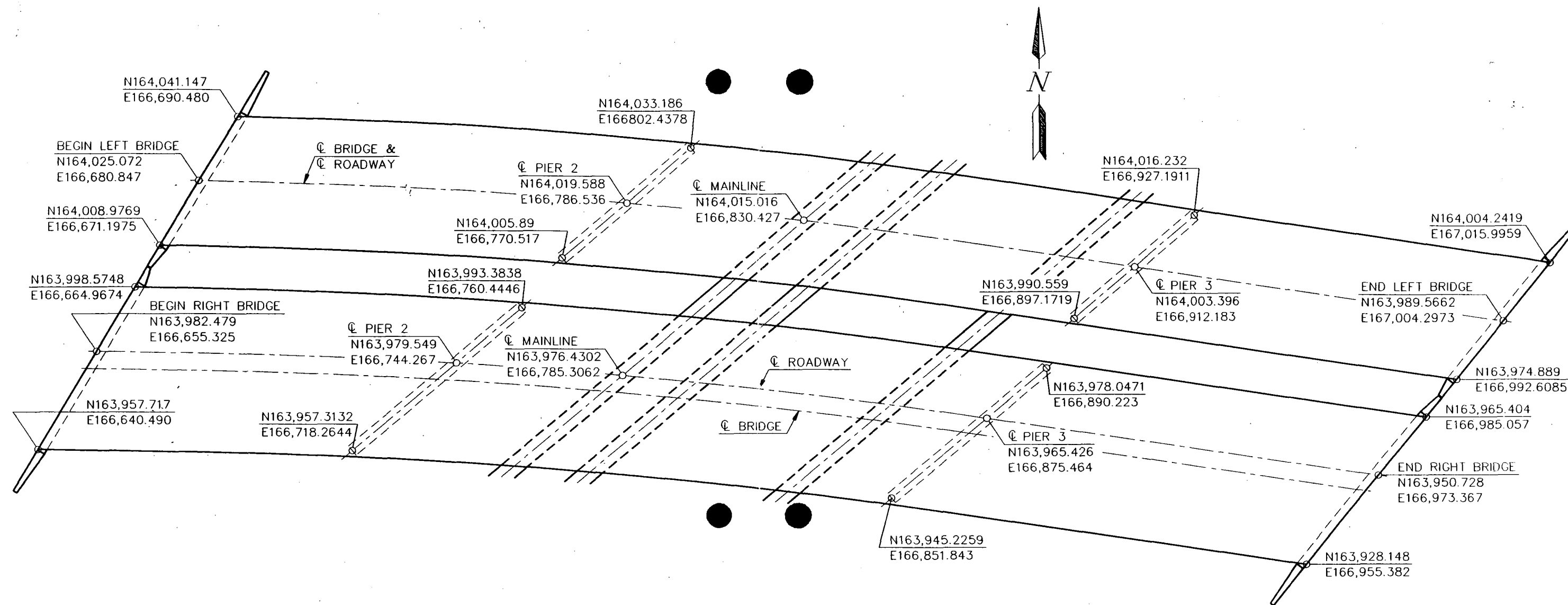
Larry Schwartz
FOR - BRIDGE ENGINEER

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	129



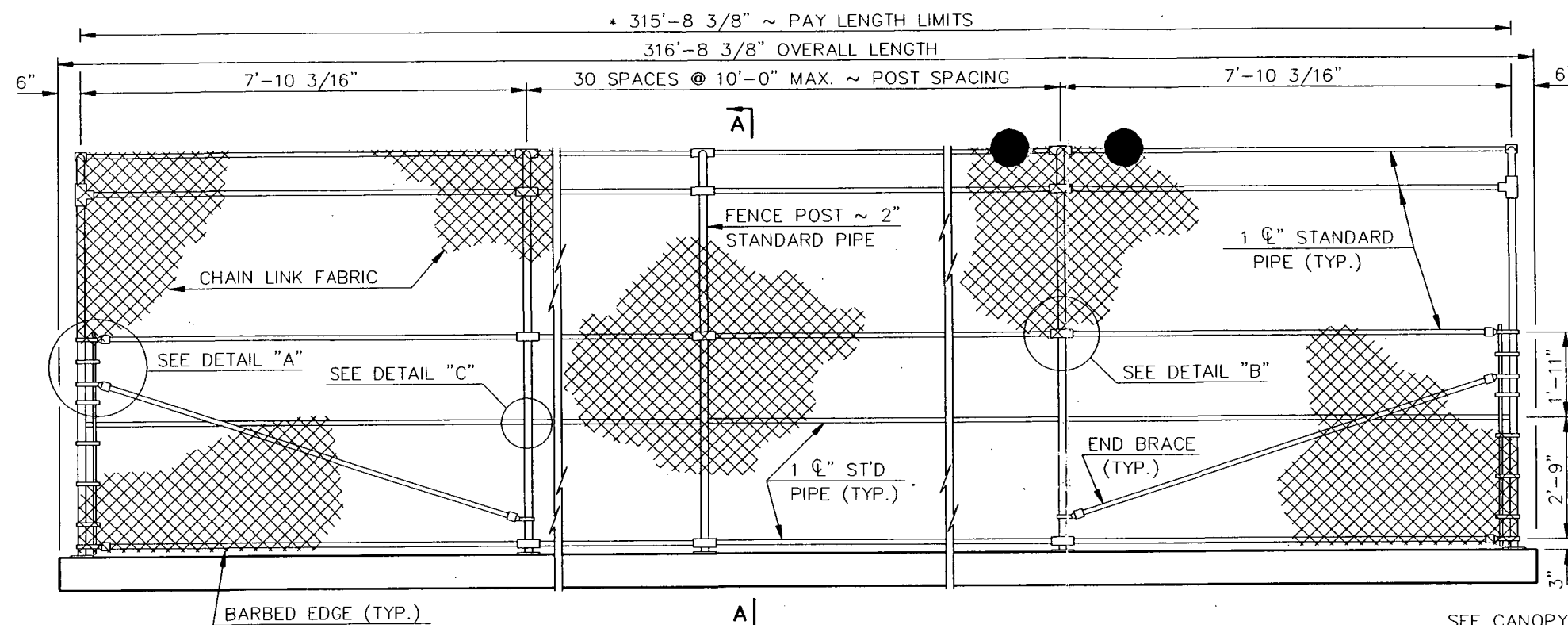
BURLINGTON NORTHERN OVERHEAD
RAY

EXISTING STRUCTURE AND
NEW PILE LOCATIONS

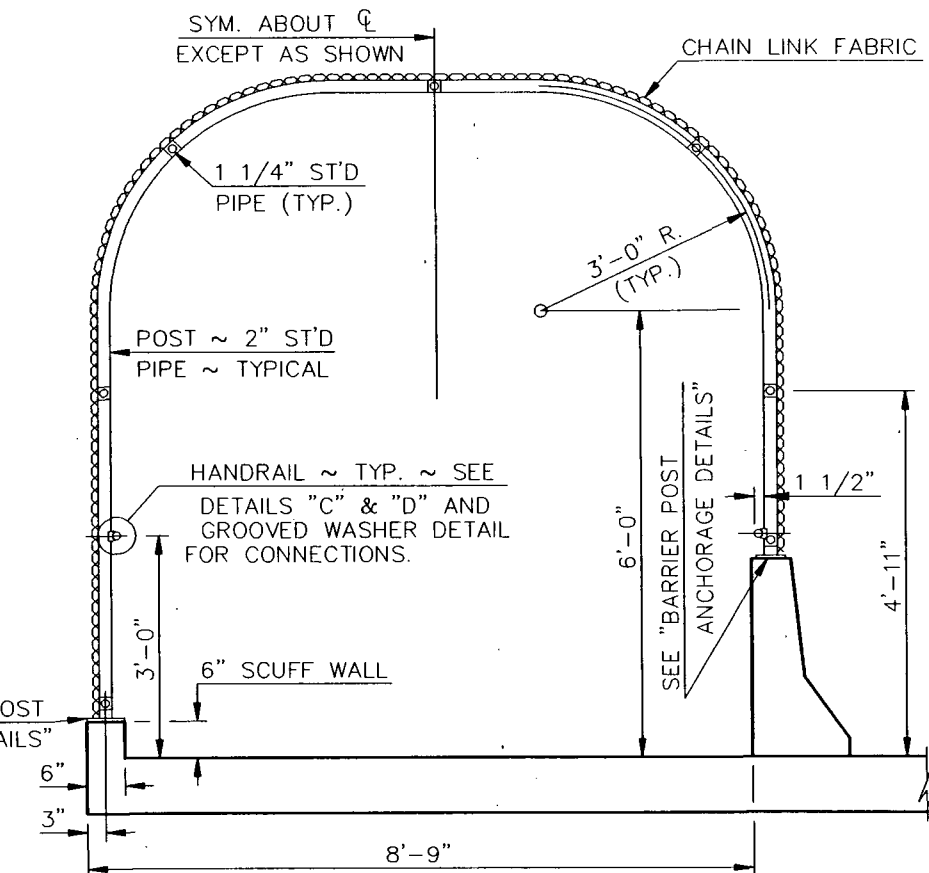


BURLINGTON NORTHERN OVERHEAD
RAY

GENERAL PROJECT LAYOUT



CANOPY ELEVATION



A-A

NOTE:
FABRIC SHALL BE AASHTO M181, TYPE 1 AND SHALL BE 9 GAGE WIRE, 2 INCH MESH, 60 INCHES HIGH, AND THE BOTTOM SELVAGE BARBED.

POSTS AND FITTINGS SHALL BE AASHTO M181, CLASS 1.

ANCHORAGE PLATES SHALL BE AASHTO M183 AND SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH AASHTO M232.

TO ANCHOR BASE PLATES, THE CONTRACTOR HAS THE OPTION TO EITHER DRILL IN 1/2" Ø SWEDGE BOLTS OR PLACE THEM IN THE CONCRETE AT THE TIME OF POURING. IF THE CONTRACTOR ELECTS TO DRILL THE BOLT HOLES, EXTREME CAUTION SHALL BE EXERCISED NOT TO DAMAGE THE CONCRETE BARRIERS AND SHALL PLACE THE BASE PLATES LONGITUDINALLY ALONG THE BARRIERS AS SHOWN ON THE "BASE PLATE PLAN" VIEW.

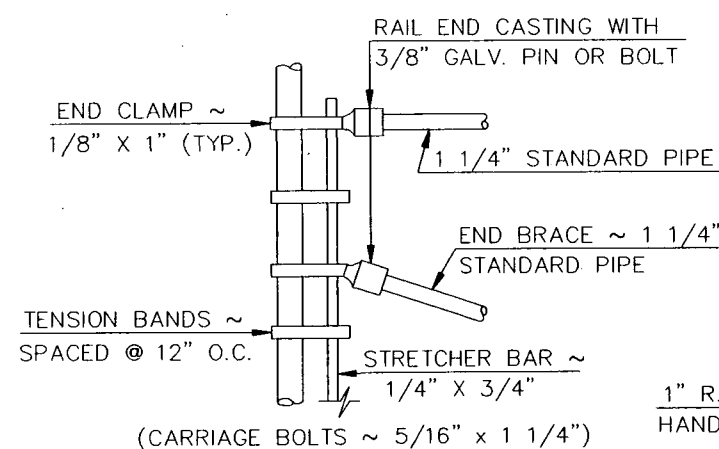
* THE PAY LENGTH IS MEASURED ALONG THE OUTSIDE FACE OF THE SCUFF WALL. POSTS LOCATED IN THE TANGENT SECTIONS SHALL BE PLACED PERPENDICULAR. POSTS LOCATED IN THE CURVE SECTION SHALL BE PLACED RADially.

QUANTITIES

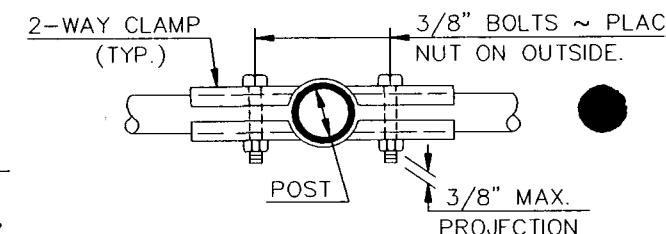
PEDESTRIAN CANOPY	316.7 LF
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BURLINGTON NORTHERN OVERHEAD RAY
PEDESTRIAN CANOPY DETAILS

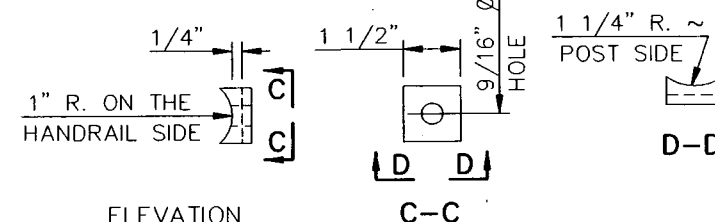
2-053.731R-33



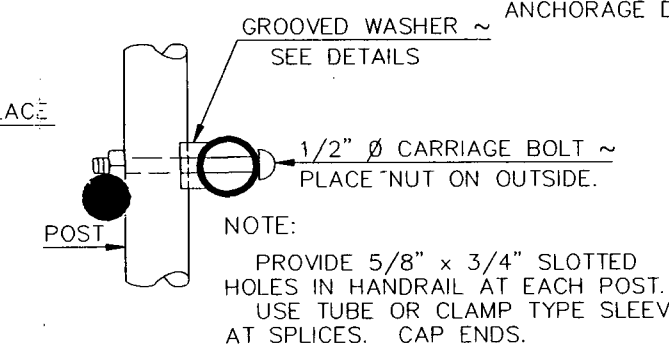
DETAIL "A"



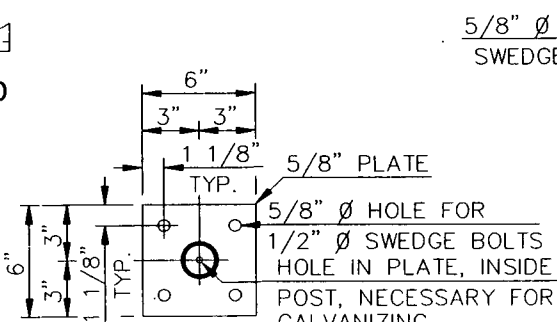
DETAIL "B"



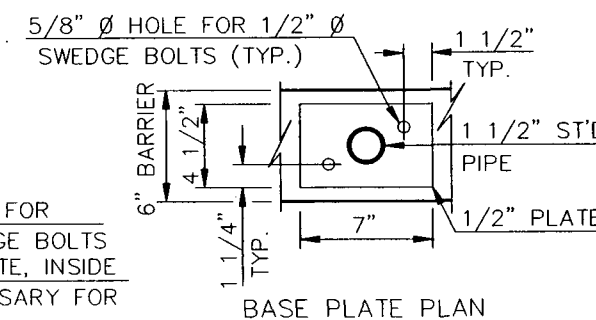
GROOVED WASHER DETAILS



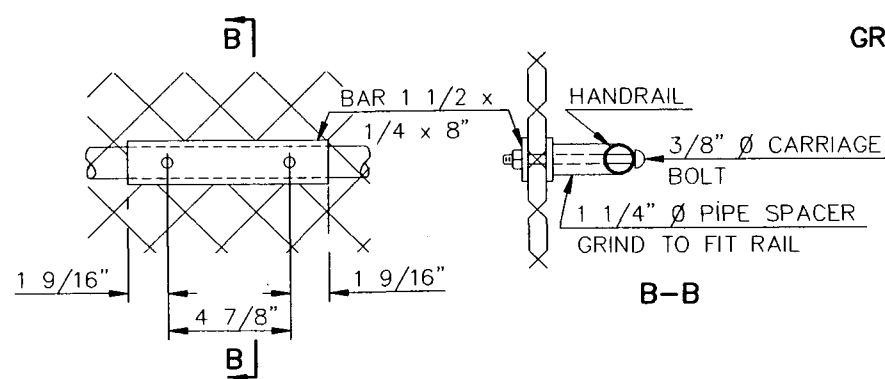
HANDRAIL CONNECTION AT POSTS
DETAIL "C"



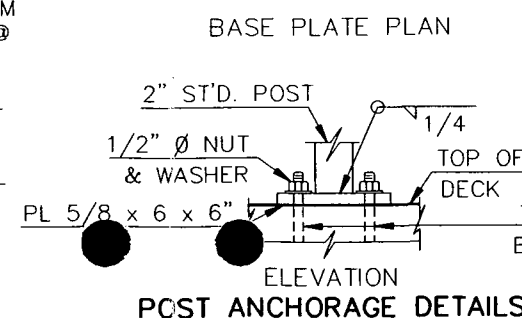
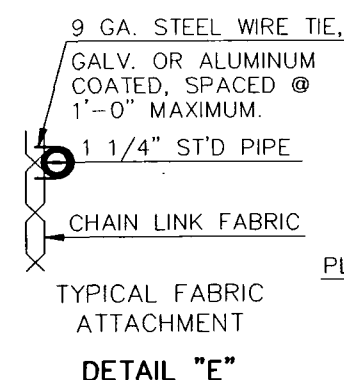
BASE PLATE PLAN



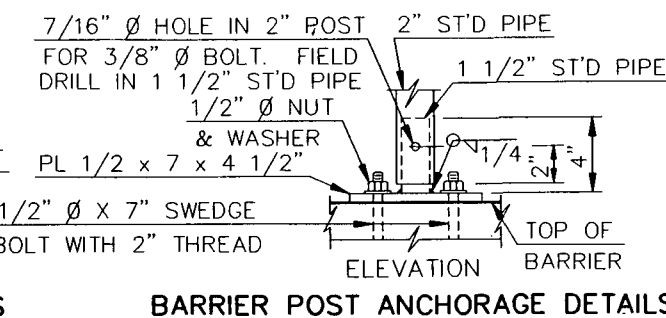
BASE PLATE PLAN



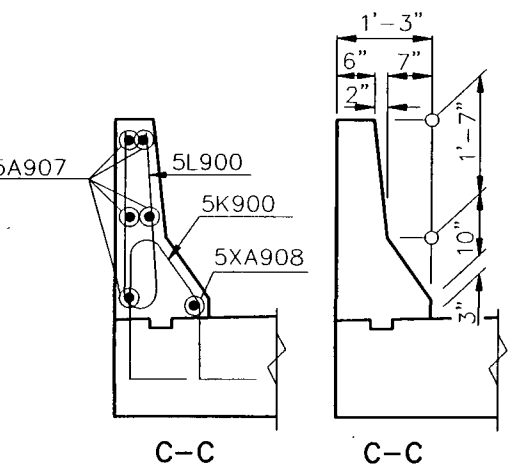
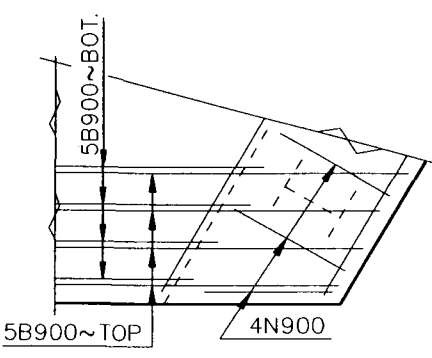
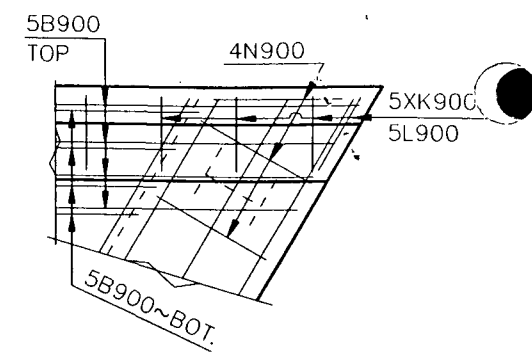
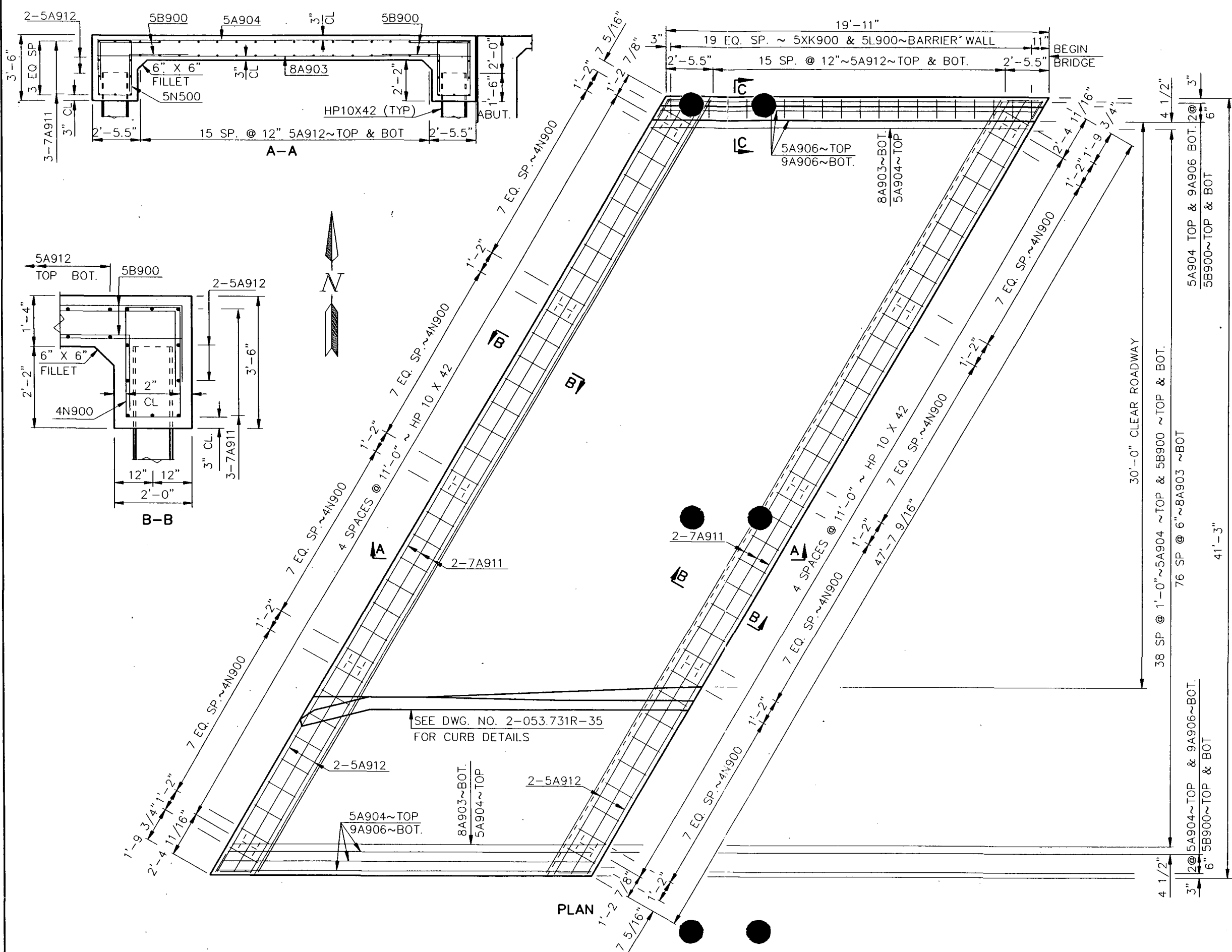
HANDRAIL CONNECTION DETAILS CENTERED BETWEEN POSTS
DETAIL "D"



POST ANCHORAGE DETAILS



BARRIER POST ANCHORAGE DETAILS



QUANTITIES
SEE DWG. NO. 2-053.731R-35
BURLINGTON NORTHERN OVERHEAD RAY
APPROACH SLAB BEGIN BRIDGE

DESIGN DATA				
Traffic	Average Daily			Est. Max. Hr.
Current 1993	Pass: 1700	Trucks 225	Total 1925	192
Forecast 2013	Pass:	Trucks 275	Total 2100	210
Minimum Sight Dist. for:		Design Speed		40 MPH.
Stopping 275 FT.		Bridges		
Safe Passing				
Passing for Marking				

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION

WILLIAMS COUNTY
FEDERAL AID PROJECT NO.
RSN-NH-7-002(041)052
Grading, Surfacing, Storm drains, Structural
Signing, Marking, Guardrail & Incidentals

JOB# 10

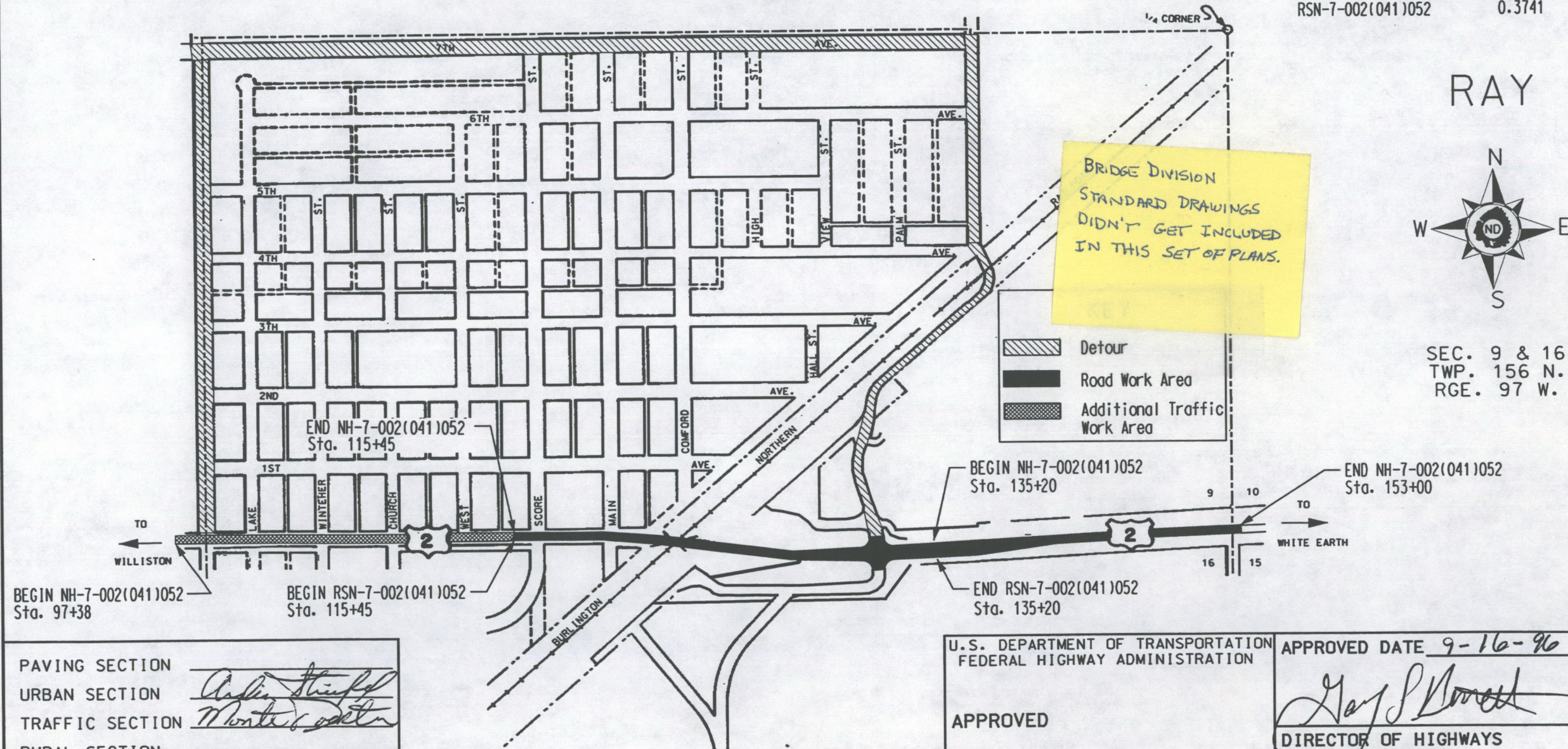
FHWA REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	RSN-NH-7-002(041)052	1

GOVERNING SPECIFICATIONS:

Standard Specifications adopted by the North Dakota Department of Transportation September 1992; Standard Drawings currently in effect; and other Contract Provisions submitted herein.

LENGTH OF PROJECT

Project	Miles-Gross	Miles-Net
NH-7-002(041)052	0.6794	0.6794
RSN-7-002(041)052	0.3741	0.3741



PAVING SECTION
URBAN SECTION
TRAFFIC SECTION
RURAL SECTION
RECOMMEND APPROVAL
DESIGN ENGINEER

U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

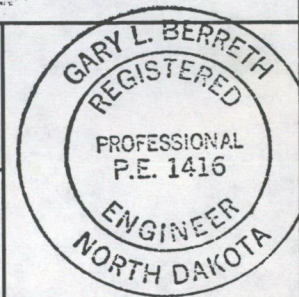
APPROVED

DIVISION ADMINISTRATOR DATE

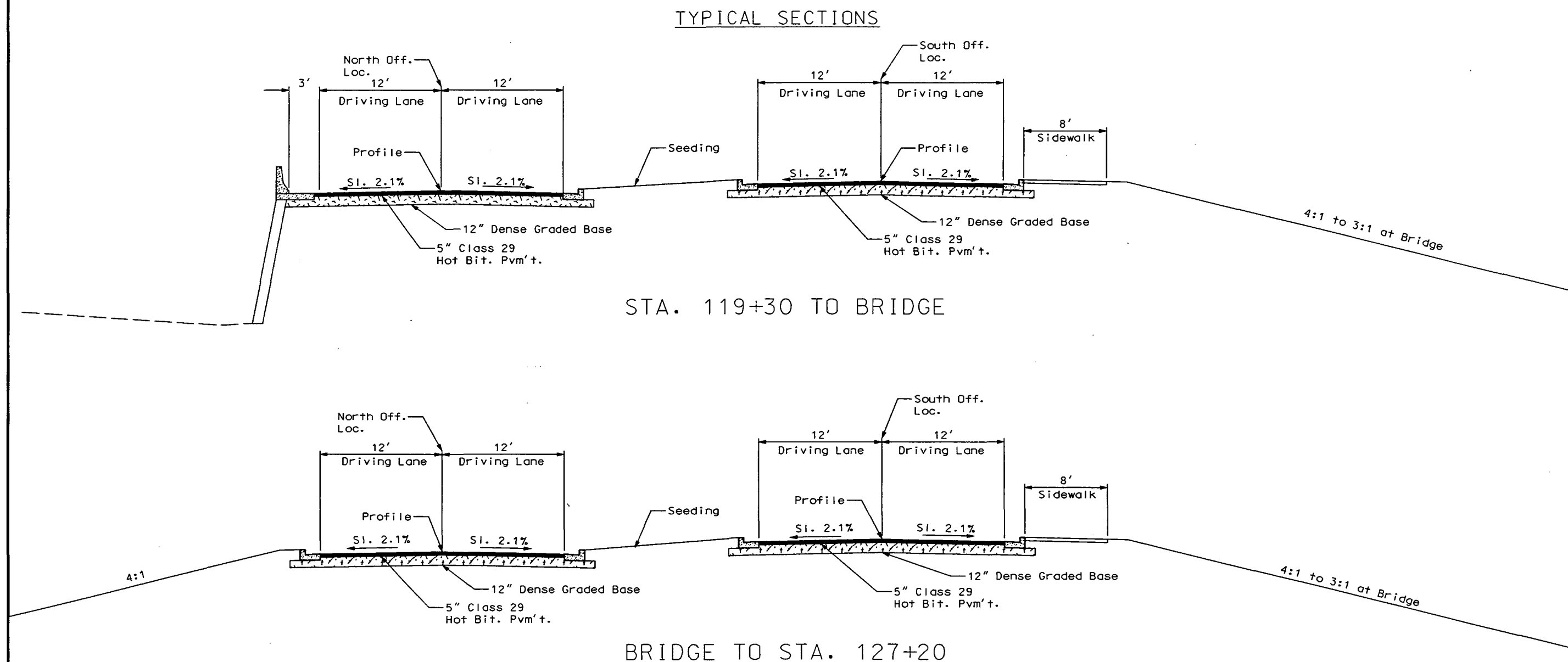
APPROVED DATE 9-16-96

DIRECTOR OF HIGHWAYS
AND ENGINEERING

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION



FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	RSN-7-002(041)052	21

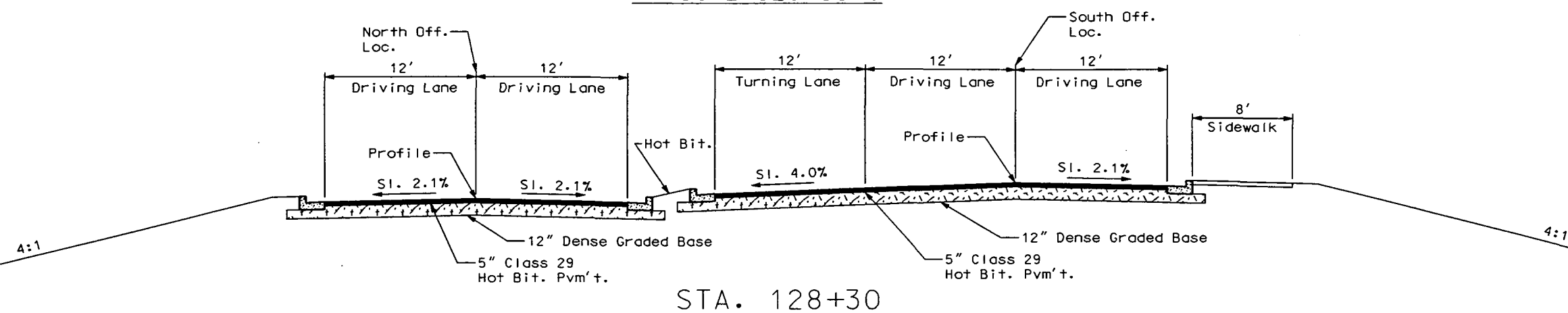


TYPICAL SECTIONS
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 STA. 119+30 TO 127+20
 TYPICAL2.DGN 1=10

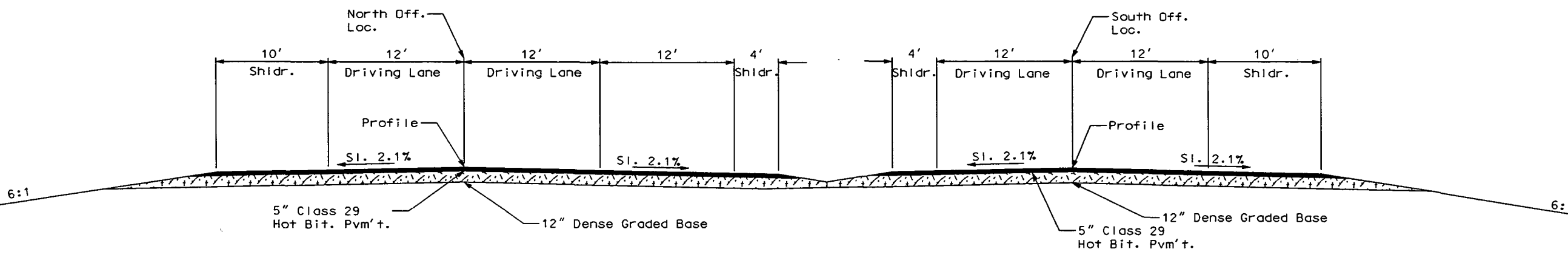
FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	RSN-7-002(041)052	22

NH-7-002(041)052

TYPICAL SECTIONS



STA. 128+30

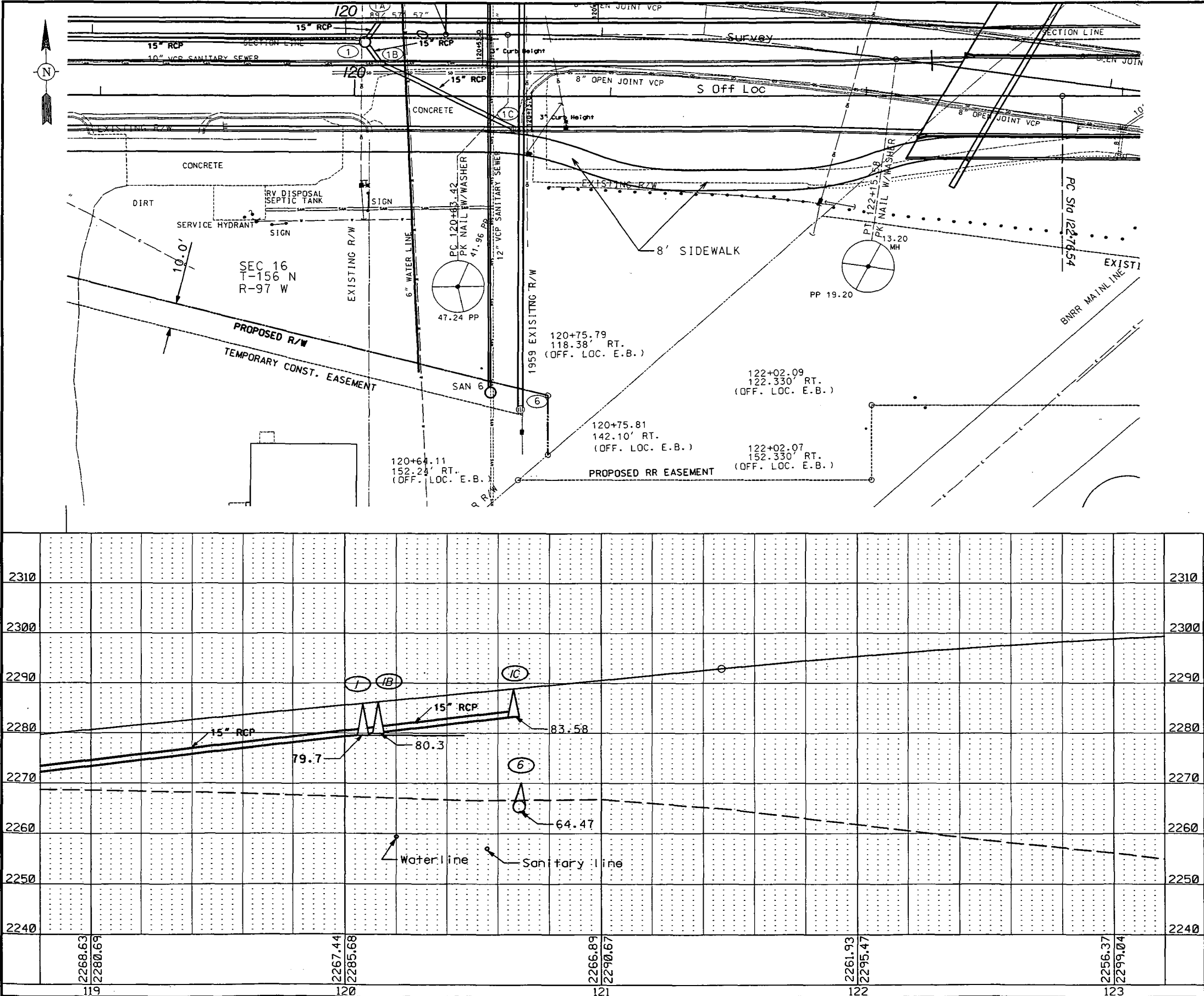


STA. 135+00

TYPICAL SECTIONS

US 2 - RAY, ND
STA. 128+30 TO 135+00
TYPICAL2.DGN 1=10

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
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REMOVE & SALVAGE CONCRETE
Sta 119+00 to 120+06 Rt 270 SY
Sta 120+70 to 121+00 Rt 43 SY
Sta 119+00 to 121+00-Rt 532 SY

REMOVE & SALVAGE BITUMINOUS SURFACING
Sta 119+00 to 120+06-Rt 190 SY
Sta 119+00 to 119+15-34'Rt 36 SY

REMOVAL OF CURB & GUTTER
Sta 119+50 to 120+06-Rt 65 LF

REMOVAL OF MANHOLES
Sta 120+57-11'Rt 1 EA

PIPE CONC REINF 15IN CL III-STORM DRAIN
1B to 1C 56 LF
1B to 1 8 LF

PIPE CONC REINF 18IN CL III-STORM DRAIN
6 to 5 188 LF

INLET-TYPE 1
1B 1 EA
1C 1 EA

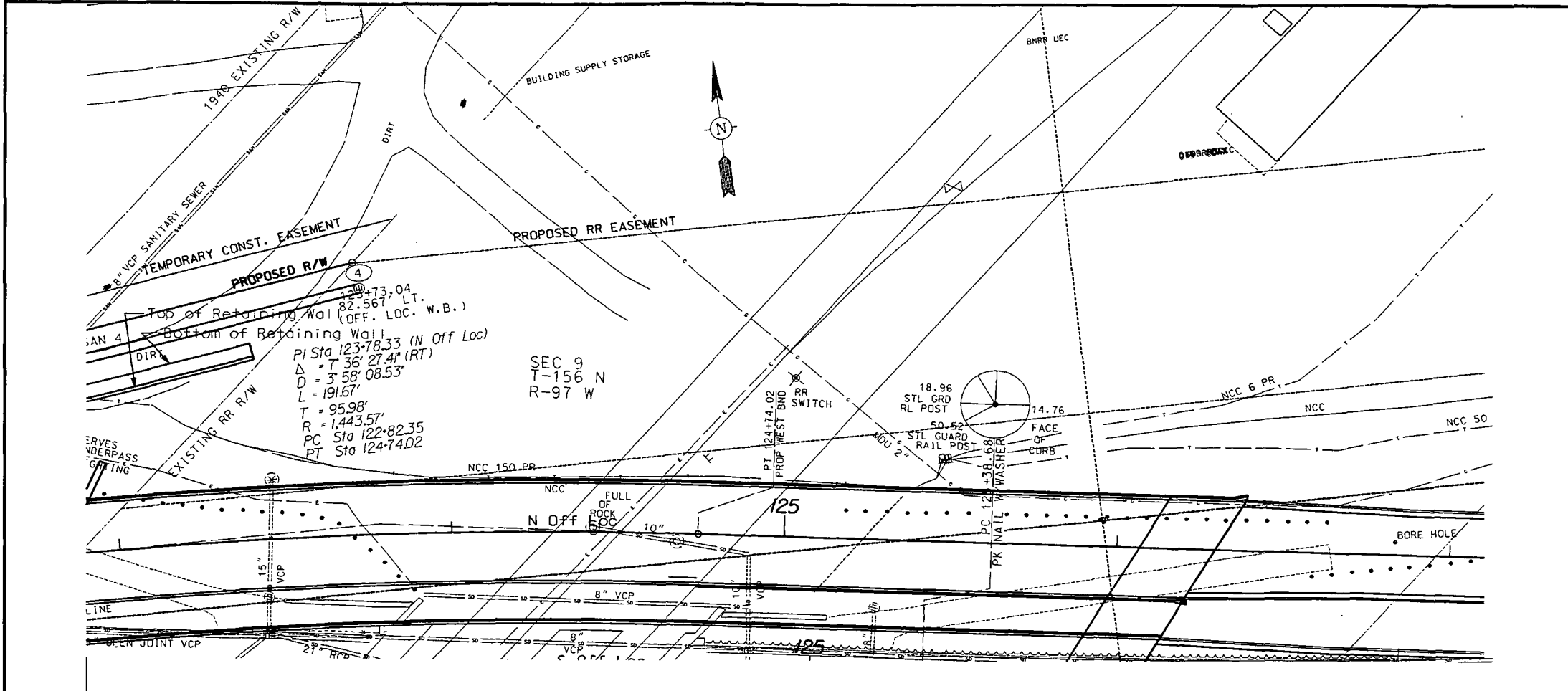
INLET CATCH BASIN 6IN BEEHIVE
6 1 EA

CURB & GUTTER MOUNTABLE TYPE 1
121+88 to 122+28.71 LT 41 LF

CURB & GUTTER TYPE 1
119+00 to 121+88 LT 288 LF
119+00 to BRIDGE RT 322 LF

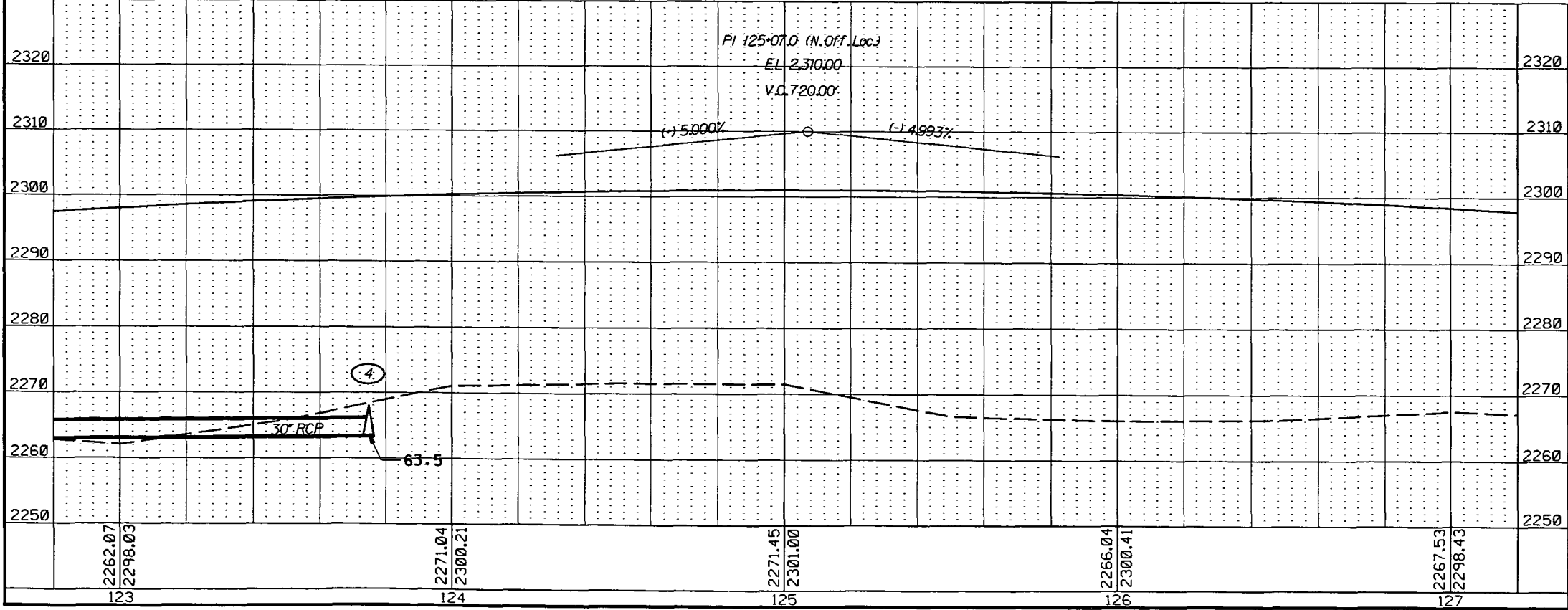
SIDEWALK CONCRETE
119+00 to BRIDGE 289 LF

BENCH MARKS			NGVD-29
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-	-	-	-
PROJECT LOCATION			
STA. 119+00 TO 123+00			
FILE:	PP04.DGN		



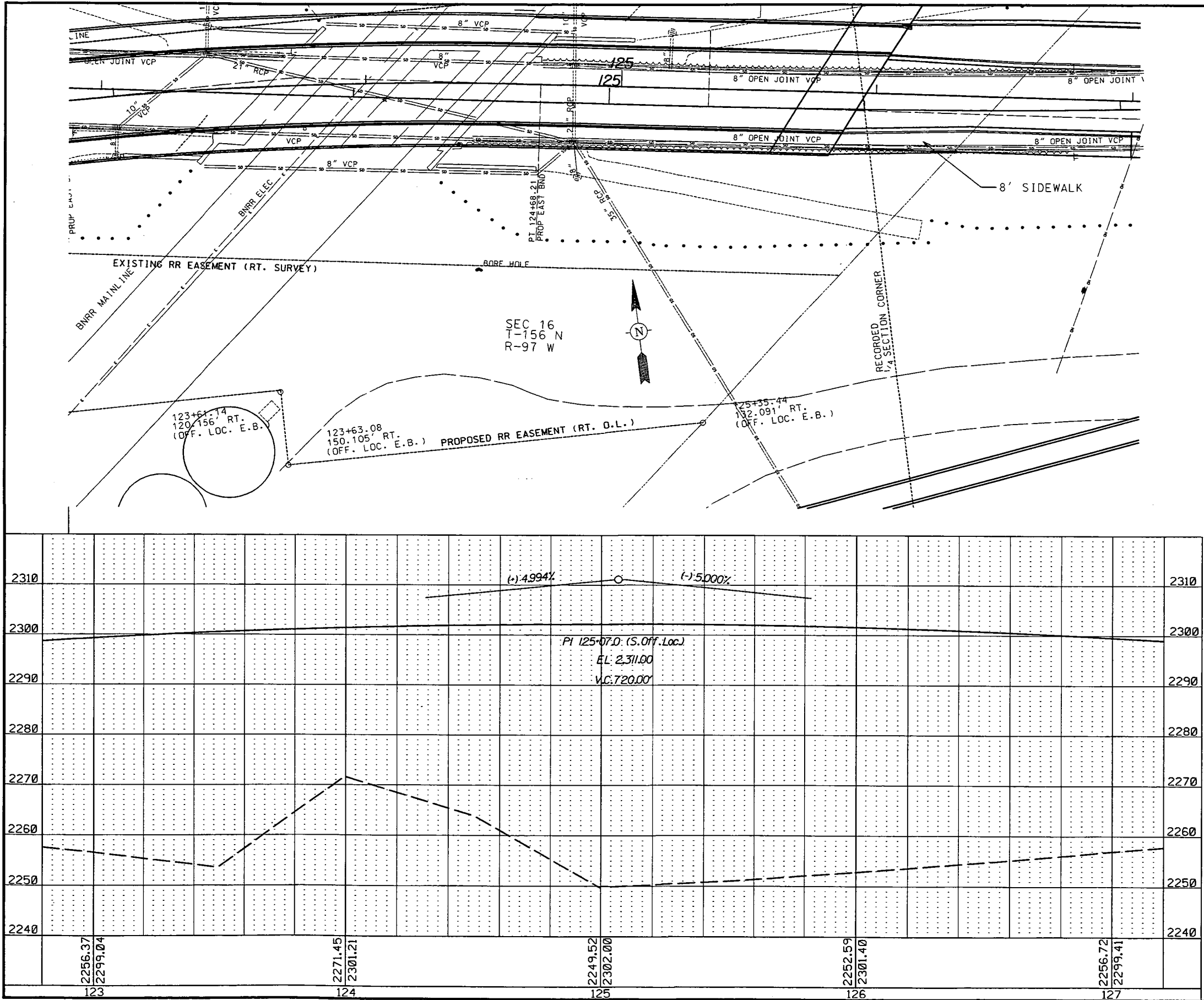
FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	ND	RSN-7-002(041)052	44

- PIPE CONC REINF 30IN CL III-STORM DRAIN
4 to 4A 150 LF
- SPECIAL CATCHBASIN 6IN BEEHIVE 48IN
4 1 EA
- CURB & GUTTER MOUNTABLE TYPE .1
126+33.79 to 126+71 LT 37.21 LF
- CURB & GUTTER TYPE 1
Bridge to 127+00 LT 71 LF
126+71 to 127+00 RT 29 LF



BENCH MARKS			
			NGVD-29
NO.	DESCRIPTION	LOCATION	ELEV.
B7H	RIVET ON CONC.	123+83-.66'RT.	2270.92
USC&G E 466	BRASS DISC	124+12-17'RT.	2270.46
I	TOP OF FH	125+53-239'RT.	2268.40
-	-	-	-
PROJECT LOCATION			
STA. 123+00 TO 127+00			
FILE: PP05.DGN		SCALE IN FEET	

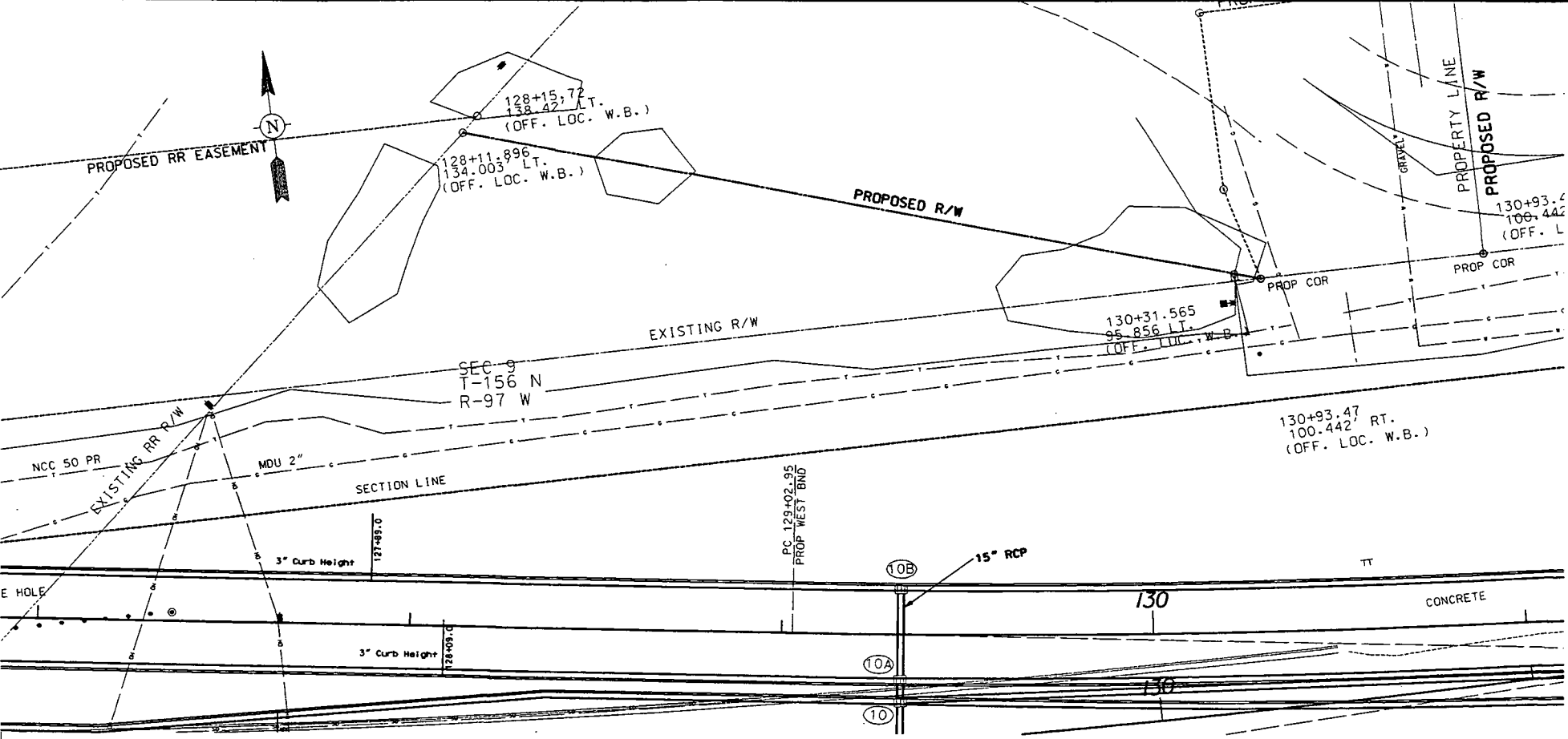
FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	ND	RSN-7-002(041)052	45



CURB & GUTTER TYPE 1	
Bridge to 127+00 LT	95 LF
Bridge to 127+00 RT	95 LF
SIDEWALK CONCRETE	
Bridge to 127+00 RT	102 SY

BENCH MARKS			
NO.	DESCRIPTION	LOCATION	ELEV.
-	-	-	-
-	-	-	-
PROJECT LOCATION			
STA. 123+00 TO 127+00			
FILE: PP06.DGN		SCALE IN FEET	

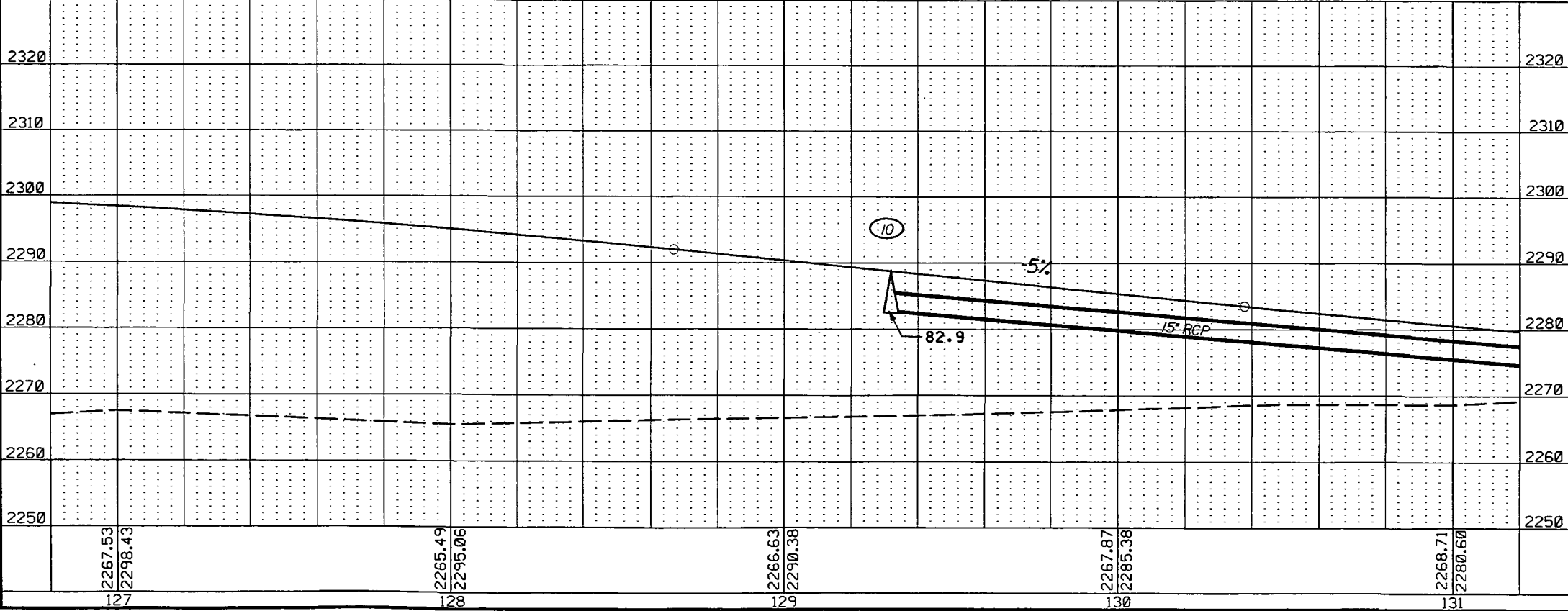
FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	ND	RSN-7-002(041)052	46



PIPE CONC REINF 15IN CL III-STORM DRAIN
 10A to 10 6 LF
 10B to 10A 22 LF

INLET-TYPE 1
 10A 1 EA
 10B 1 EA

CURB & GUTTER TYPE 1
 127+00 to 131+00 LT 400 LF
 127+00 to 131+00 RT 400 LF



BENCH MARKS				NGVD-29
NO.	DESCRIPTION	LOCATION	ELEV.	
BM 1	U-IRON BY PP	127+50-188'RT.	2265.03	
-	-	-	-	-
PROJECT LOCATION				
STA. 127+00 TO 131+00				
FILE:	PP07.DGN			

Total Station Layout Sheet

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	RSN-NH-7-002(041)052	87

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Curve Data

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"306 308 307" @ curv3
"309 311 310" @ curv4
"312 314 313" @ curv5
"315 316 317" @ curv6
"318 320 319" @ curv20
"321 323 322" @ curv21
"324 326 325" @ curv30
"327 329 328" @ curv31
"330 332 331" @ curv32
"333 335 334" @ curv33
"336 338 337" @ curv37
"339 341 340" @ curv38

Survey Alignment

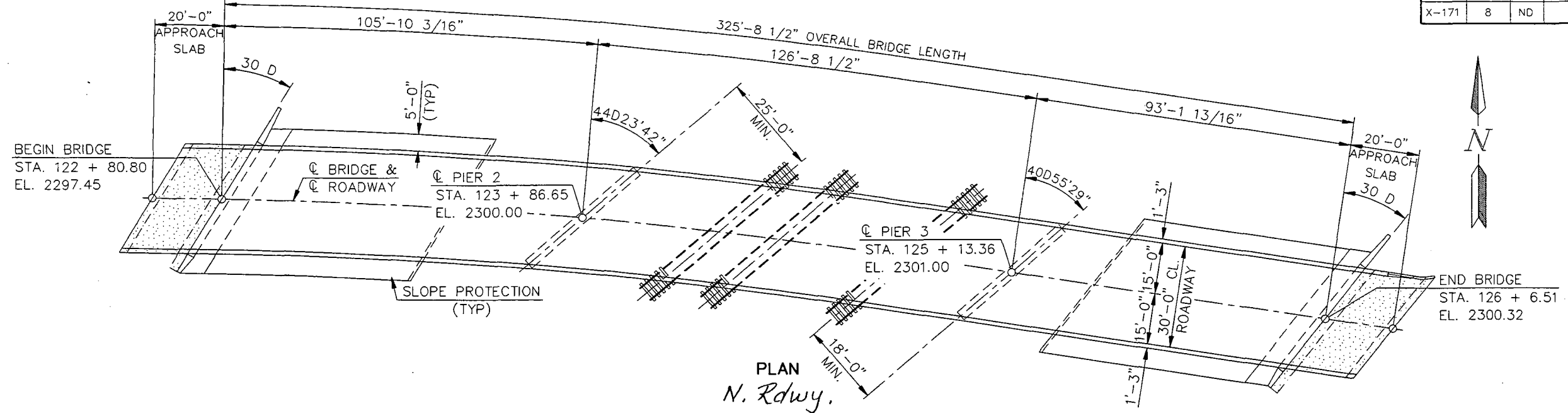
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"303 305 304"
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8
9

North Office Location

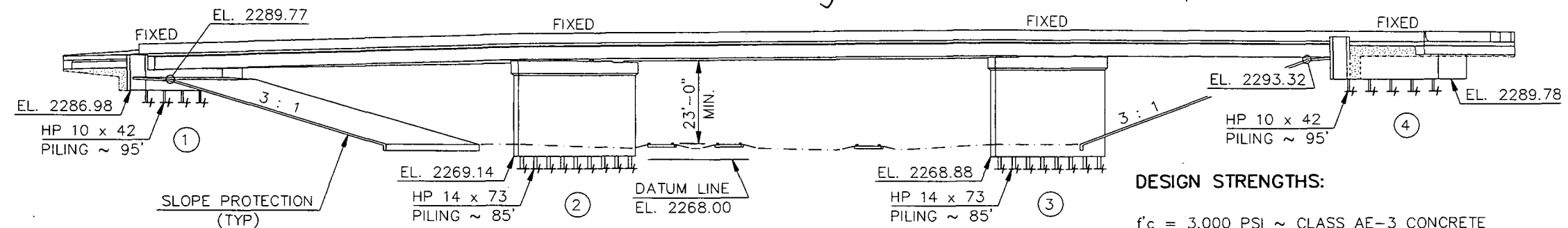
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24
25

South Office Location

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"339 341 340"



PLAN
N. Rdwy.

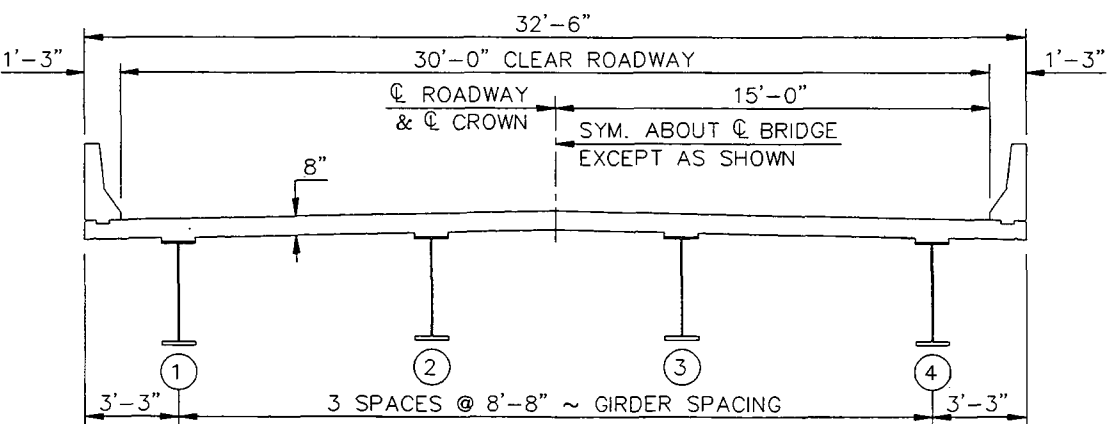


ELEVATION

DESIGN STRENGTHS:

f'c = 3,000 PSI ~ CLASS AE-3 CONCRETE
f'c = 4,000 PSI ~ CLASS AAE-3 CONCRETE
fy = 36,000 PSI ~ AASHTO M-270 GR. 36 STRUCTURAL STEEL
fy = 50,000 PSI ~ AASHTO M-270 GR. 50 STRUCTURAL STEEL
fy = 60,000 PSI ~ REINFORCING STEEL

LOAD FACTOR DESIGN



TYPICAL SLAB SECTION

SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
107	0100	RAILWAY PROTECTION INSURANCE	L. SUM	.5
202	0105	REMOVAL OF STRUCTURE	L. SUM	.5
210	0102	CLASS 1 EXCAVATION - SITE 1	L. SUM	1
210	0198	SELECT BACKFILL	TON	389.8
210	0202	FOUNDATION PREPARATION - SITE 1	L. SUM	1
550	0215	CONCRETE BRIDGE APPROACH SLAB	SQ. YD.	144.0
602	0130	CLASS AAE-3 CONCRETE	CU. YD.	340.2
602	1130	CLASS AE-3 CONCRETE	CU. YD.	218.7
602	1250	PENETRATING WATER REPELLENT TR.	SQ. YD.	1085.7
612	0115	REINFORCING STEEL GRADE 60	LBS.	25,029
612	0116	REINFORCING STEEL GRADE 60 EPOXY	LBS.	81,927
616	5888	STRUCTURAL STEEL SITE 1 (APPROXIMATELY 311,691 LBS.)	L. SUM	1
622	0012	STEEL H-PILE TIPS 10x42	EA.	26
622	0016	STEEL H-PILE TIPS 14 x 73	EA.	17
622	0020	STEEL PILING HP 10x42	L. FT.	2470
622	0060	STEEL PILING HP 14x73	L. FT.	1445
708	1100	SLOPE PROTECTION, CONC.	SQ. YD.	738
930	3000	BRIDGE BENCH MARKS	SET	.5
930	7013	ROADWAY CANOPY - SITE 1	L. SUM	1
930	8640	NOSING CONCRETE	L. FT.	75.0
930	8650	PREFORMED ELASTOMERIC NEOPRENE EXP. JT.	L. FT.	71.2

BENCH MARKS

NO.	DESCRIPTION	LOCATION	ELEVATION
B7H	RIVET ON CONCRETE	STA. 123 +83 - .66' RT.	2270.92
I	TOP OF FIRE HYDRANT	STA. 125 +53 - 239' RT.	2268.40



STANDARD DRAWINGS
D-900-1, D-708-1, D-622-1

F.W.S. 15 PSF

HS 25 DESIGN LOADING

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION

BURLINGTON NORTHERN OVERHEAD
RAY

BRIDGE LAYOUT
PROJECT: RSN-7-002(041)052
STATION 124 + 43.6
WILLIAMS COUNTY

19 SEP 96
DATE
For - BRIDGE ENGINEER

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
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BRIDGE NOTES

- 100 SCOPE OF WORK: This project consists of building a new 3-span steel girder bridge 326' long with clear roadway width of 30'. Part of the new structure is on a tangent alignment and part is on a curve.
- 100 GENERAL: The cost of furnishing and placing preformed expansion joint filler, concrete inserts, tie wire, bar spacers, bar supports, and other miscellaneous items shall be included in the price bid for Class AE-3 and AAE-3 concrete.
- 105 The existing structural steel is painted with lead-based paint. Certain contractor operations could expose employees to hazardous levels of lead. The contractor shall plan accordingly and shall inform employees of the hazards of lead-based paint.
- 202 REMOVAL OF STRUCTURE: The existing structure is an underpass 107' long with a clear roadway of 30 feet. The substructures are made of concrete, and the superstructure is made of steel. The contractor may have to remove portions of the existing wingwall footings and existing retaining walls to allow for pile driving. The contractor shall remove a minimum amount of material from the wingwall footings. The contractor shall place compacted fill in the existing underpass up to an elevation 6' below the bottom of the steel beams. The contractor shall entirely fill the pedestrian tunnels and the voids under the sidewalks of the existing structure with either earth or flowable fill. Burlington Northern (BN) Railroad will remove the superstructure and portions of the substructure. BN Railroad will place the removed material on the railroad right of way at a location agreed to by BN Railroad and the engineer. All materials removed by the contractor and BN Railroad shall become the property of the contractor and shall be disposed of properly off the right of way. All labor and materials required to perform the work described above shall be included in the bid item "Removal of Structure."
- 203 EMBANKMENT: The embankment at the abutments shall be in place for a minimum of 30 days before piling are driven.
- The contractor will be required to bore through the fill at the abutments before driving piling. All pilot holes not completely filled by the piles shall be backfilled with sand or fine gravel before the substructure is placed.
- In lieu of the above waiting period, the contractor may begin pile driving operations as soon as the embankment is in place, but all pilot holes shall be extended to a depth of 15 feet into the original ground.

- 210 EXCAVATION: The excavation at the abutments, as shown, and the excavation required to build the piers shall be included in the lump sum bid item, "Class 1 Excavation, Site 1."
- 210 SELECT BACKFILL: Select backfill shall meet the requirements of Section 816.03, Class 3. The backfill shall be placed in layers of not more than 6 inches, moistened or dried as required, and thoroughly compacted with mechanical tamping equipment.
- 550 BRIDGE APPROACH SLABS: Mechanical finishing of the approach slabs shall be required. A mechanical or hand-held transverse metal tine finish shall be applied. Tining shall start 6" from the beginning and end of the approach slabs. A surface tolerance of 3/16" in 10 feet is also required.
- 602 Exterior beams shall be braced to prevent rotation. The strength and spacing of the bracing shall be dependent on the forces induced by the weight of the concrete, forms, equipment, and workers.
- 602 END BEAMS: The end beam concrete shall be placed before the deck concrete. The concrete shall cure for at least 72 hours before deck placement.
- 602 SURFACE FINISH "D": Surface Finish "D" shall be required for the inside, outside, and top surfaces of the barrier, and on the outside edge of the deck.
- 602 DECK CONCRETE: Beams and girders have slight variations in the anticipated camber. To build the deck to the designated thickness will require slight adjustments in deck elevation and/or riser dimensions. These adjustments result in minor concrete quantity discrepancies. The contractor shall consider this quantity discrepancy when he bids the unit price for Class AAE-3 Concrete. The Department will pay plan quantity of Class AAE-3 Concrete.
- 602 Deflection of the deck shoring shall be computed using the total dead load plus the weight of the finishing machine. The forming shall be adjusted properly to accommodate the deflection and thereby maintain the total slab thickness specified in the plans.
- 602 PENETRATING WATER REPELLENT TREATMENT: Penetrating water repellent shall be applied to the driving surface of the concrete deck.
- 602 BARRIERS: Barriers shall be constructed according to the provisions of Section 602.03 B.4 except that there shall be no expansion or deflection joints. Make 3/4" V-grooves in

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BRIDGE NOTES

all faces of the barriers at each pier and at equal spaces between substructures at approximately 10-foot spacing.

- 602 DECK TINING: Tining shall begin 6 inches from the beginning and the end of the deck and 6 inches from each deck joint.
- 612 REINFORCING STEEL: Dimensions for bent bars are given out to out and to tangent intersections unless otherwise noted.
- 612 The bar fabricator shall add a prefix to all bar designations to differentiate between the different structures on this project.
- 612 All reinforcing steel shall be Grade 60.
- 616 STRUCTURAL STEEL: Structural steel shall be AASHTO M 270, Grade 50T2, except the diaphragms and swedge bolts may be grade 36T2. The requirement for Charpy V-Notch is waived for the bearings, diaphragms, and diaphragm connection material.
- 616 Shear connectors on splice plates shall be moved to clear bolt holes.
- 616 Field connections shall be made with 7/8 inch diameter, AASHTO M 164 high-strength bolts unless otherwise shown.
- 616 Temporary or permanent attachments or devices that are not shown on the plans as part of the structure shall not be welded to the structural steel members during the fabrication and construction process.
- 616 The cost of swedge bolts shall be included in the total cost of structural steel.
- 622 PILING: Piling shall be driven with a steam, air, or diesel hammer with a rated energy and ram weight not less than 55,154 foot-pound-tons, as computed by the formula $W(E-13,444)+.821E$, where W is the weight of the ram in tons and E is the rated hammer energy. In no case shall the ram weight be less than 2700 pounds.
- 622 PILING: Toothed cast steel pile tips (ASTM A148 steel) shall be required on all pile driven.
- 630 PAINT AND PAINTING: The structural steel shall be painted according to the specifications. The finish coats shall be blue, color number 25240, and shall meet Federal Standard No. 595B colors.

- 708 SLOPE PROTECTION: The concrete slope protection will be limited to the cast-in-place type shown on Standard D-708-1.

SHOP DRAWINGS: CAD-generated shop drawings may be submitted on 11-inch by 17-inch detail sheets. The contractor shall submit the following shop drawings to the Construction office for approval:

1. Structural Steel.
2. Preformed Elastomeric Neoprene Expansion Joint.

DESIGN STRENGTH: F'C 3,000 PSI Cl. AE-3 Concrete
F'C 4,000 PSI Cl. AAE-3 Concrete
FY 36,000 PSI Structural Steel M270
Grade 36T2
FY 50,000 PSI Structural Steel M270
Grade 50T2
FY 60,000 PSI Grade 60 Reinforcing Steel
Load Factor Design

- 930 PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT: The neoprene joint shall have a profile that allows thermal cycles and dynamic loading up to a total range of movement of two inches while providing watertight protection. The neoprene profile shall be bonded to the concrete surfaces with an epoxy adhesive that is chemically resistant to oxidation and corrosion, is impervious to weather extremes and is compatible with the nosing concrete. During installation, the neoprene profile shall be pressurized with compressed air to maximize the adhesion against the gap walls. After curing, the air pressure shall be permanently released. This shall be a Jeene Structural Sealing Joint System (2 W) or an approved equal.
- 930 NOSING CONCRETE: The nosing concrete material shall be an elastomeric concrete or a polymeric concrete that will provide a durable edge that can withstand live-load traffic without chipping or spalling. The nosing concrete material shall be Ceva Crete 95, manufactured by E-Poxy Industries Inc.; Elastomeric Concrete, as manufactured by the D.S. Brown Company; Silspec 900, as manufactured by Construction Materials Inc.; Elastomeric Concrete as manufactured by Harris Speciality Chemicals Inc., or an approved equal. The nosing shall be mixed and installed according to the manufacturer's recommendations. The cost for the equipment, materials, and labor to install the nosing concrete on each side of the joint shall be included in the lineal foot bid item "Nosing Concrete."
- 930 ROADWAY CANOPY: The contractor shall construct a canopy above the railroad under the structure to protect traffic from falling material. The canopy is an added safeguard and does not relieve the contractor of any responsibility

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BRIDGE NOTES

for the safety of the public.

The canopy may be supported from the ground or suspended from the girders. The erection of the canopy and the girder shall be completed in a minimum amount of time and with the least inconvenience to the public and the railroad.

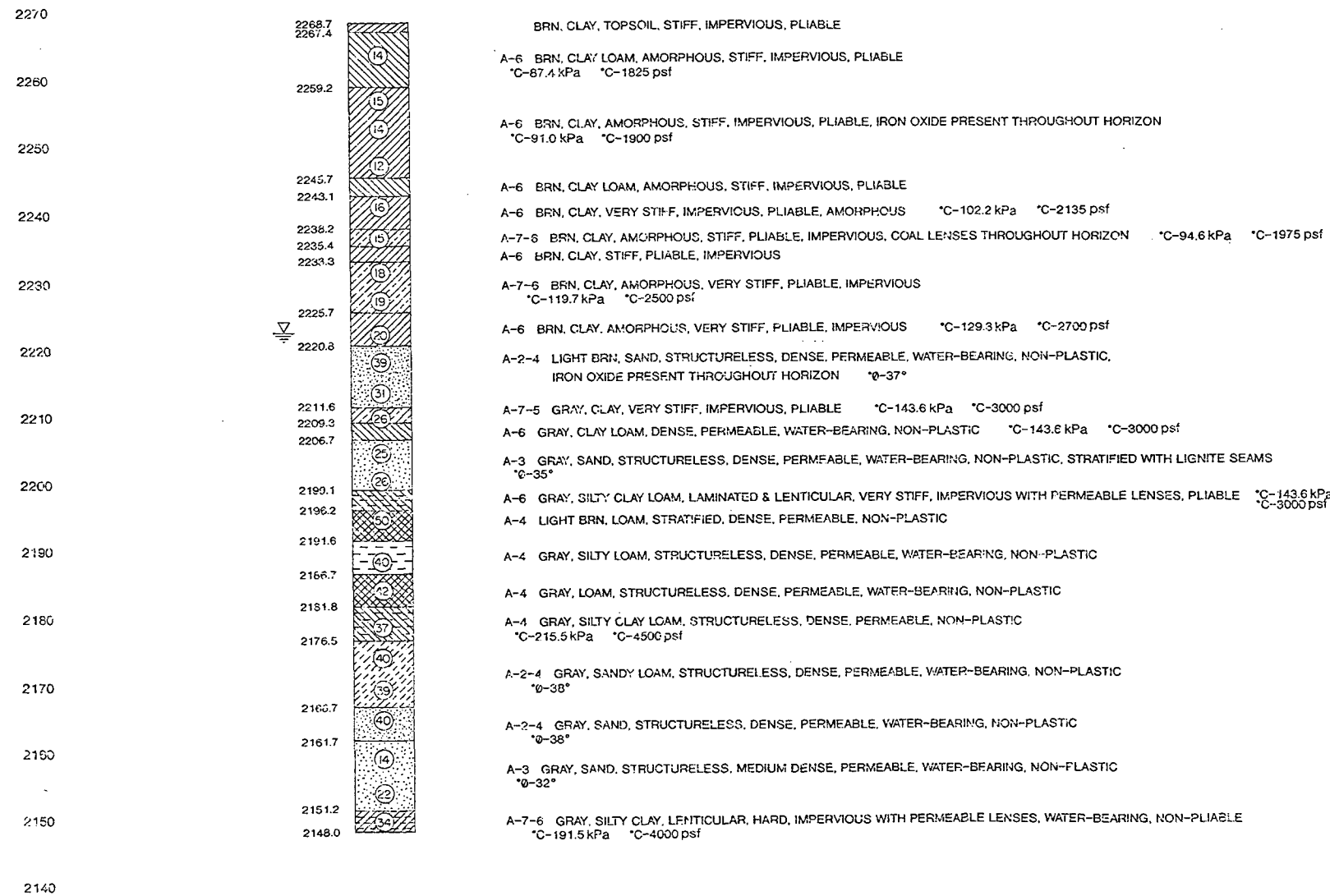
The canopy shall be of a design and material selected by the contractor and approved by the engineer. The minimum vertical clearance shall be 22' above the railroad tracks. The canopy shall project a minimum distance of 5'-0" beyond the outside edge of curb of the proposed structure.

The canopy shall project a minimum distance of 5'-0" beyond the edge of the driving lanes beneath the structure and 5'-0" beyond the edge of the railroad tracks.

After completion of the structure, the canopy shall be removed and shall remain the property of the contractor.

The roadway canopy shall be paid for at the contract lump sum unit price for "Roadway Canopy." The roadway canopy shall be measured as a lump sum item and shall include construction, maintenance, and removal.

UTILITY ACCOMMODATIONS: Northwest Communications Cooperative plans to place communication cables on the left bridge. To accommodate this, the contractor shall install 2-3" diameter conduits in the north jersey barrier of the left bridge. Northwest Communications will furnish and deliver the conduit to the project site. The contractor shall contact Myron Ranum or Gary Cox (701-568-3331) a minimum of 72 hours before the conduit is needed. The labor to install these conduit shall be incidental to the bid item "Class AAE-3 Concrete."

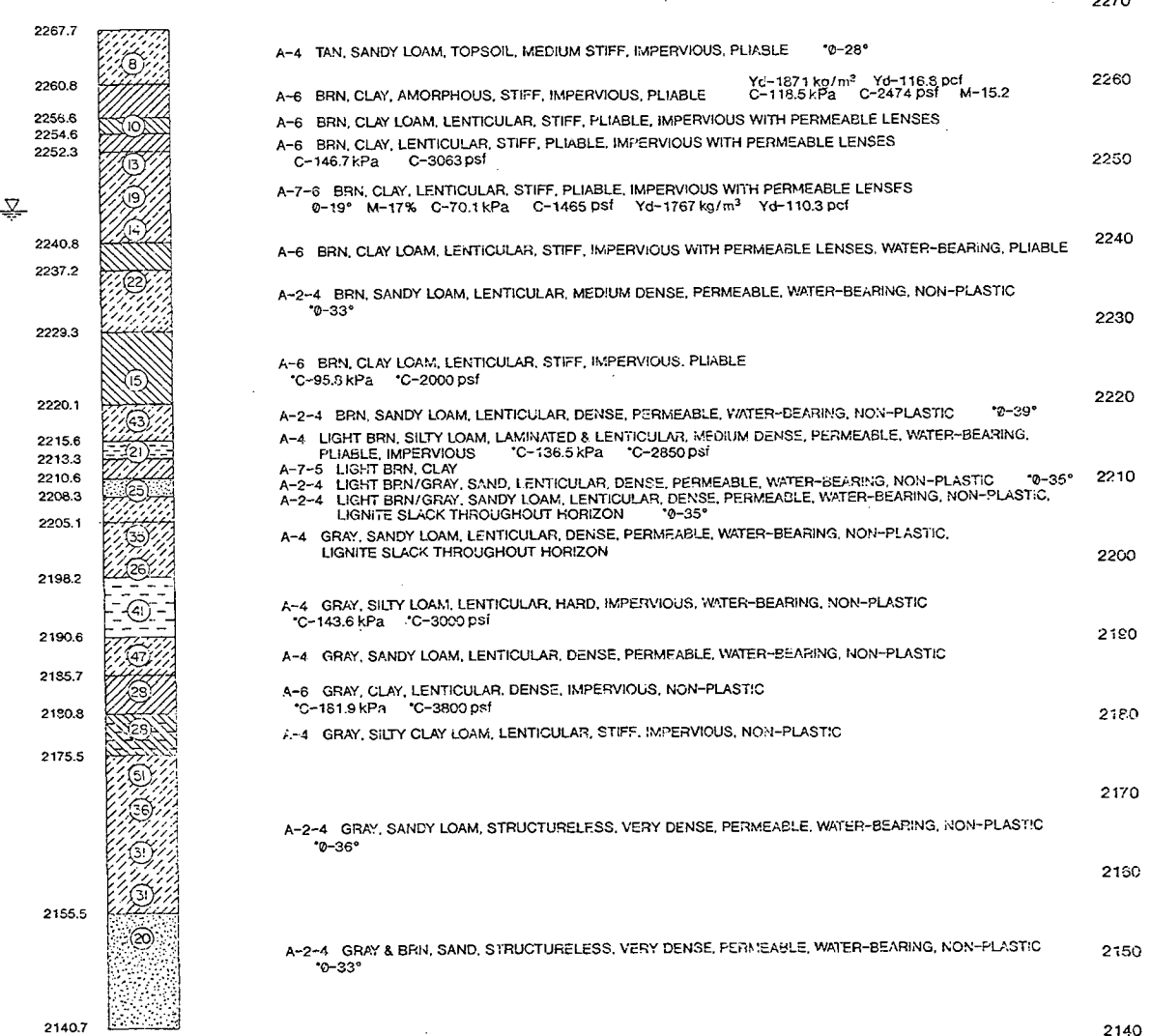


BORING NO. 1

STA. 124+51.4 - RT. 66.6' FROM EXISTING CENTERLINE

NOTES: Encircled numbers indicate the number of blows delivered by a 140 lb. hammer from a height of 30" to drive a core tube 1'-0".

The boring log data shown is for design purposes only. The state assumes no responsibility if soil conditions encountered during construction differ from those shown.



BORING NO. 2

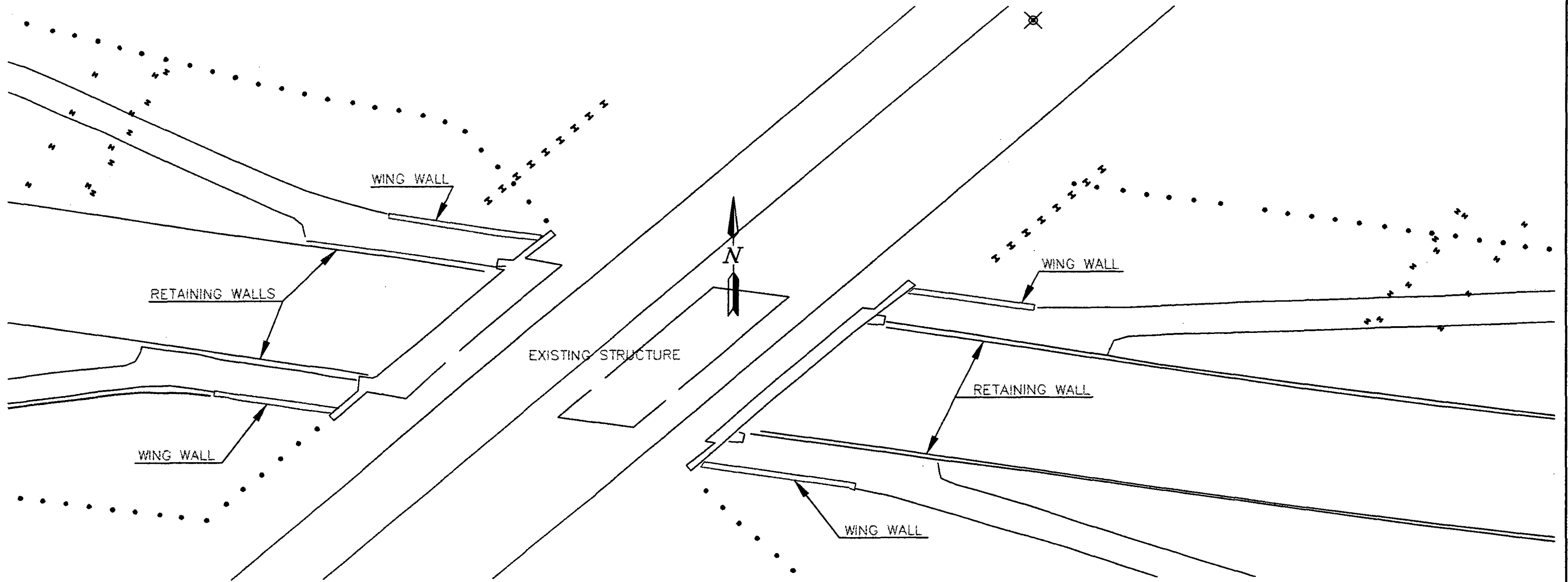
STA. 126+81.1 - LT. 51.2' FROM EXISTING CENTERLINE

SYMBOLS: 0- Angle of Internal Friction (Degrees)
C- Cohesion (Lbs./Sq. Ft.) (kPa)
M- Moisture (Percent)
Yd- Dry Weight (Lbs./Cu.Ft.) (kg/Cu.m)

*- (These cohesive values & friction angles are estimated from blow count.)

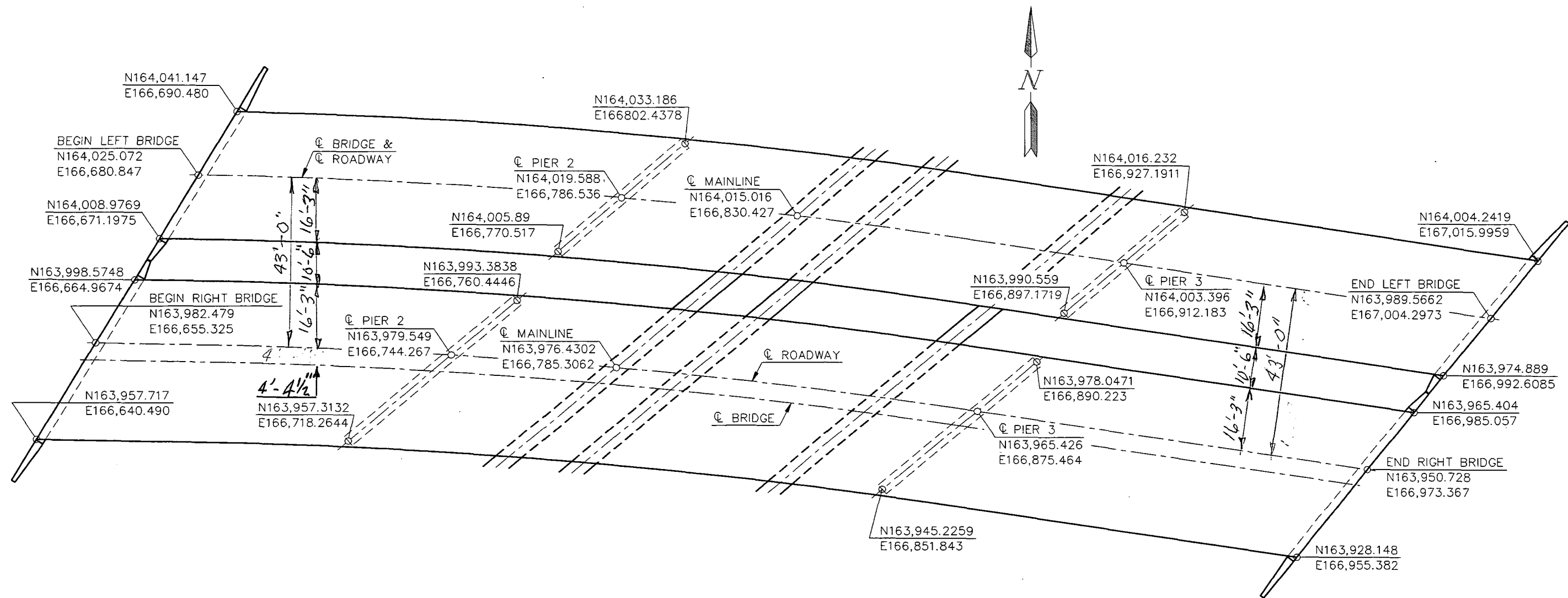
WILLIAMS COUNTY
RAY OVERPASS ON HWY 2

BORING LOG



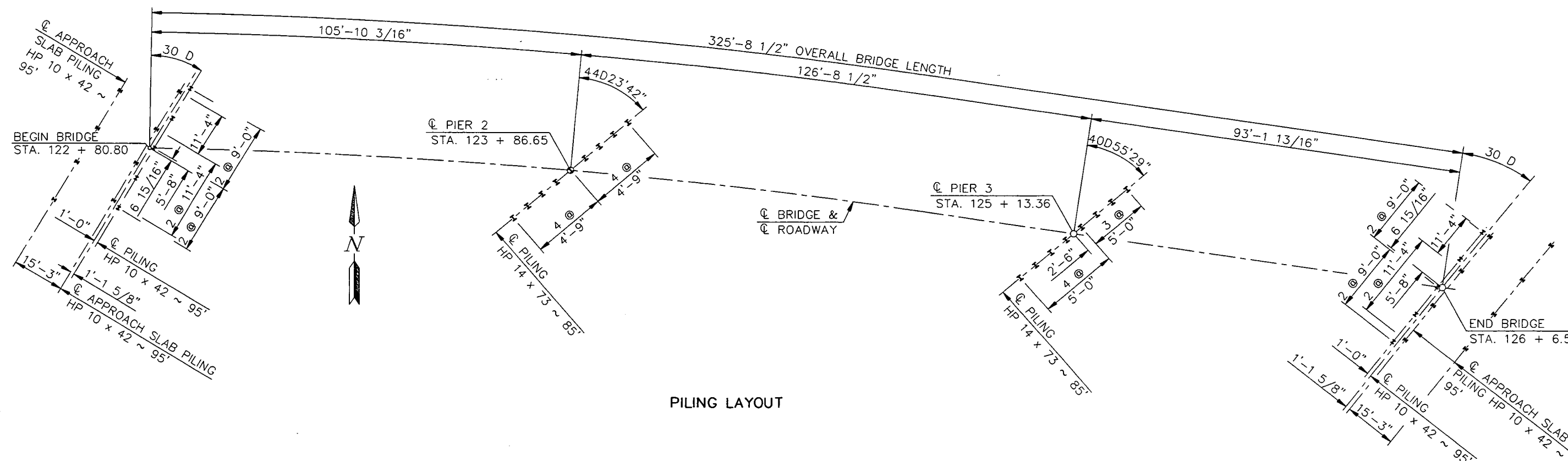
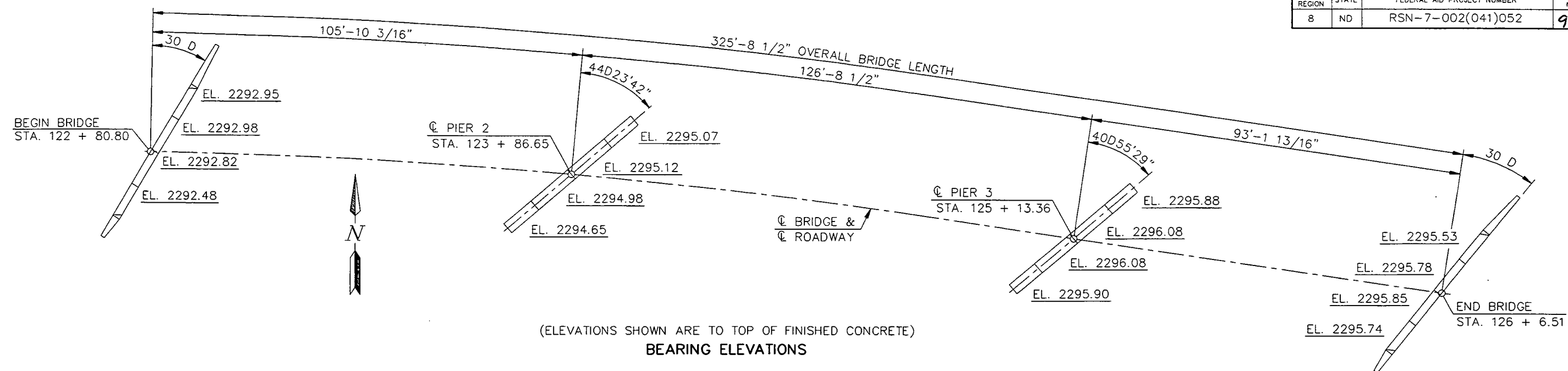
BURLINGTON NORTHERN OVERHEAD
RAY

EXISTING STRUCTURE AND
NEW PILE LOCATIONS



BURLINGTON NORTHERN OVERHEAD
RAY

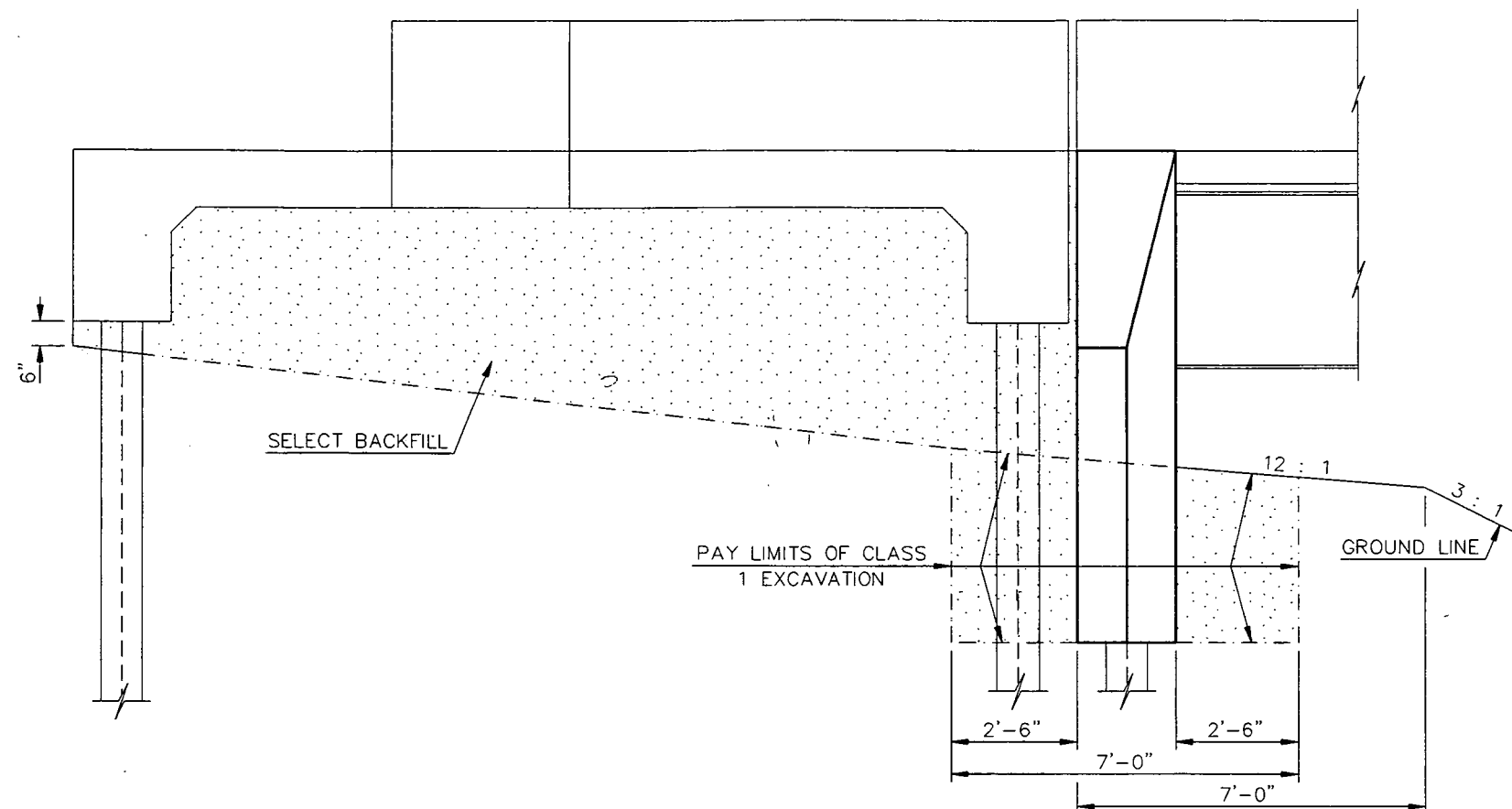
GENERAL PROJECT LAYOUT



NOTE:
THE HP 10 x 42 PILE SHALL BE DRIVEN TO 55 T. THE HP 14 x 73 PILE SHALL BE DRIVEN TO 97 T.

BURLINGTON NORTHERN OVERHEAD RAY

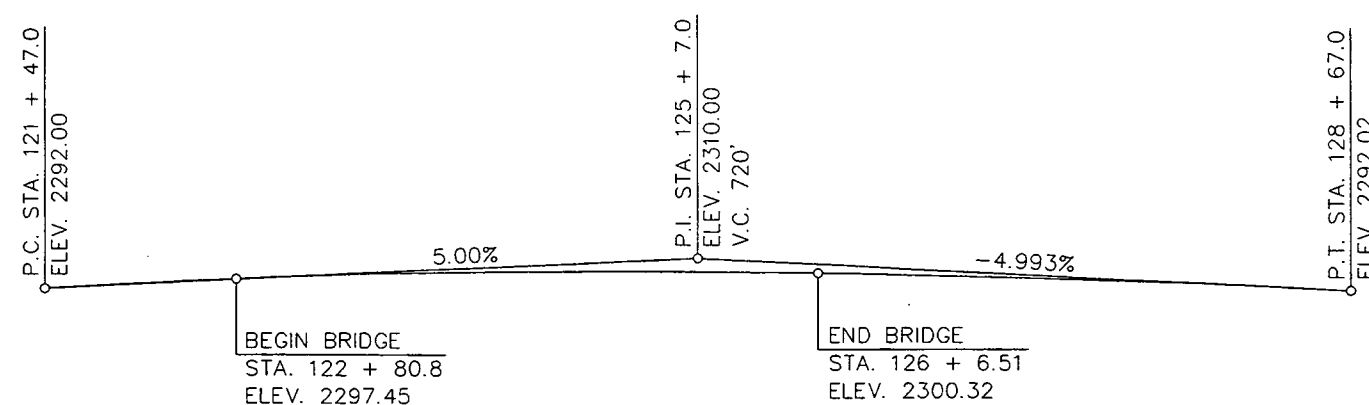
**BEARING ELEVATIONS
& PILING LAYOUT**



DETAIL AT ABUTMENT

HORIZONTAL CURVE DATA

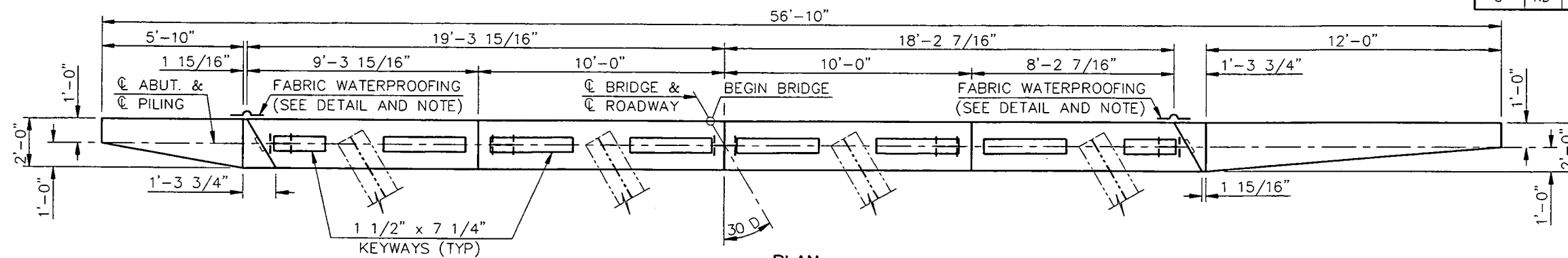
P.I. STA. 123 + 78.33
 DELTA = 7D 36' 27.24" RT
 DEGREE = 3D 58' 8.52"
 LENGTH = 191.6731'
 TANGENT = 95.9776
 RADIUS = 1443.57'
 P.C. STA. 122 + 82.35
 P.T. STA. 124 + 74.02



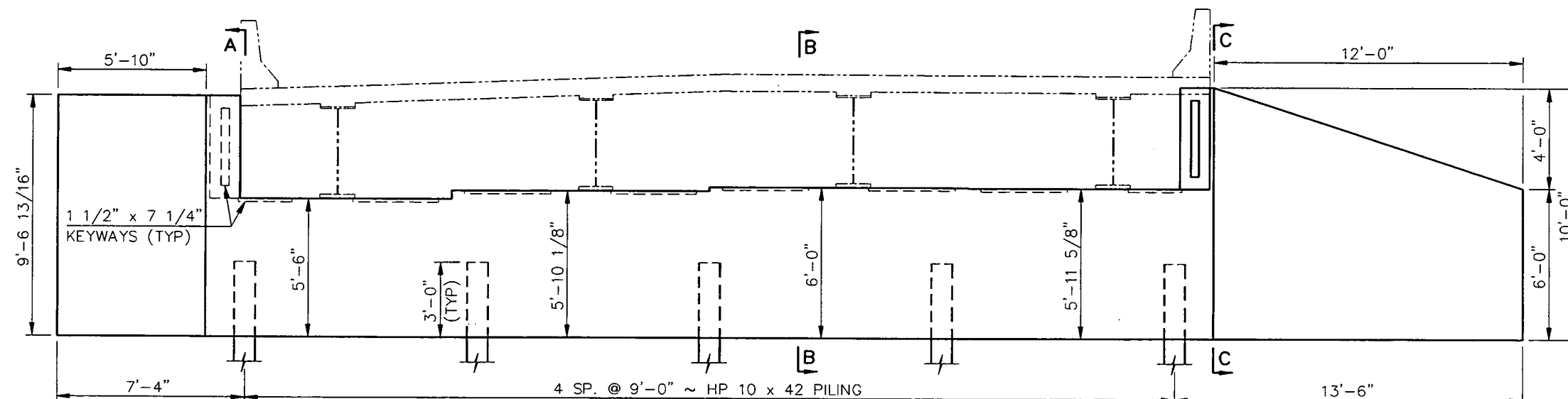
VERTICAL CURVE DATA

BURLINGTON NORTHERN OVERHEAD
RAY

DETAIL AT ABUTMENT,
VERTICAL & HORIZONTAL
CURVE DATA



PLAN



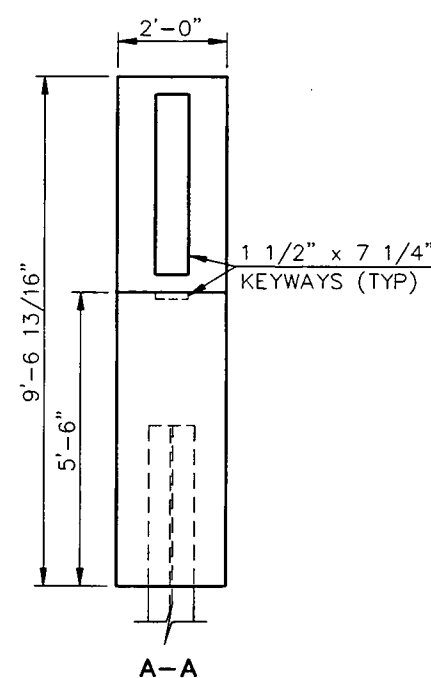
ELEVATION

WIDTH OF MATERIAL (1'-0" MIN.)

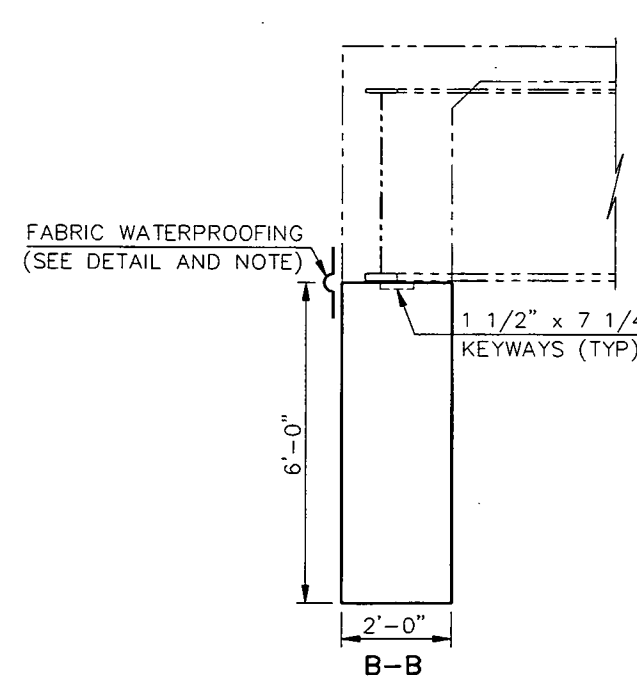
 FABRIC WATERPROOFING SHALL BE APPLIED IN ACCORDANCE WITH SECTION 740 OF THE NDDOT SPECIFICATIONS.

ALL MATERIAL AND WORK SHALL BE CONSIDERED INCIDENTAL TO THE PAY ITEM OF CLASS AE-3 CONCRETE.

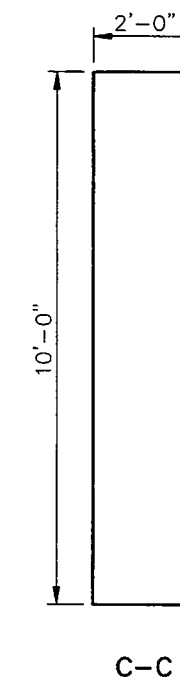
TWO-PLY FABRIC WATERPROOFING DETAIL



A-A



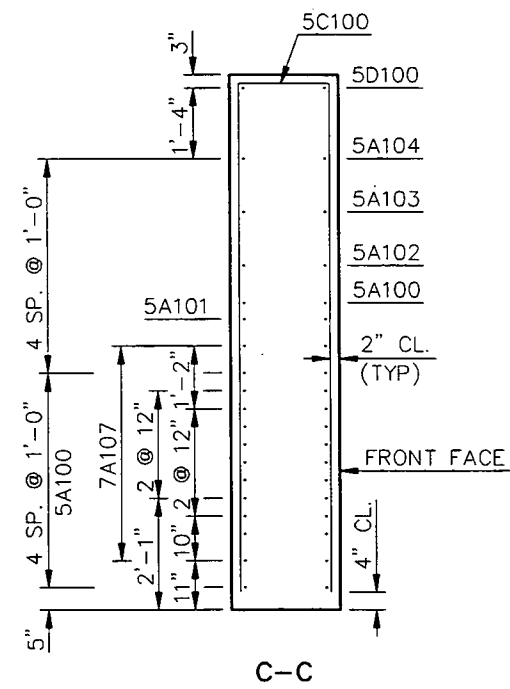
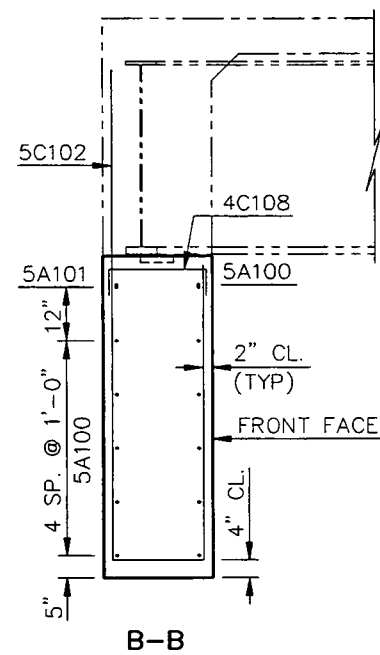
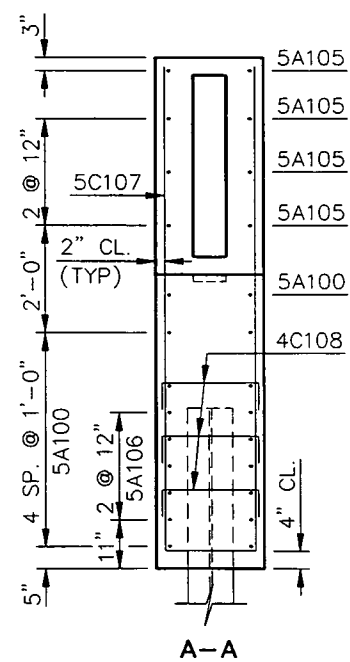
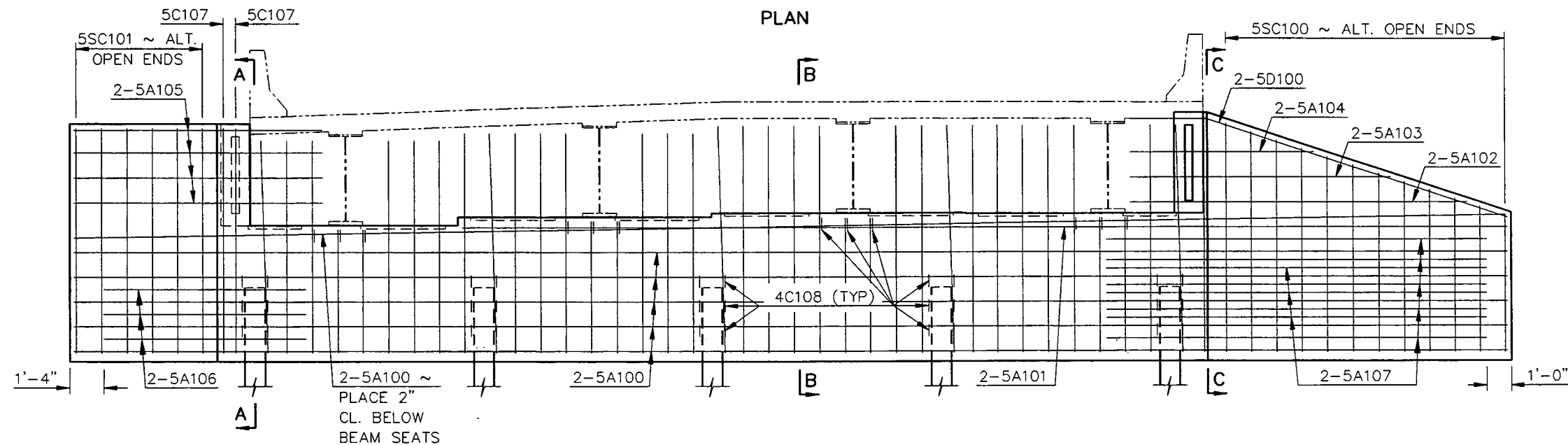
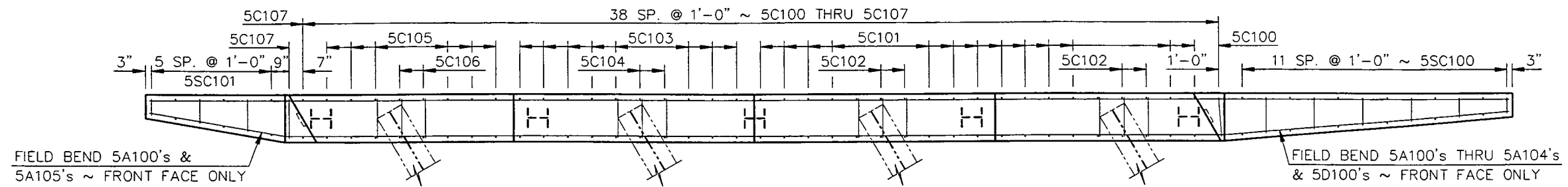
B-B



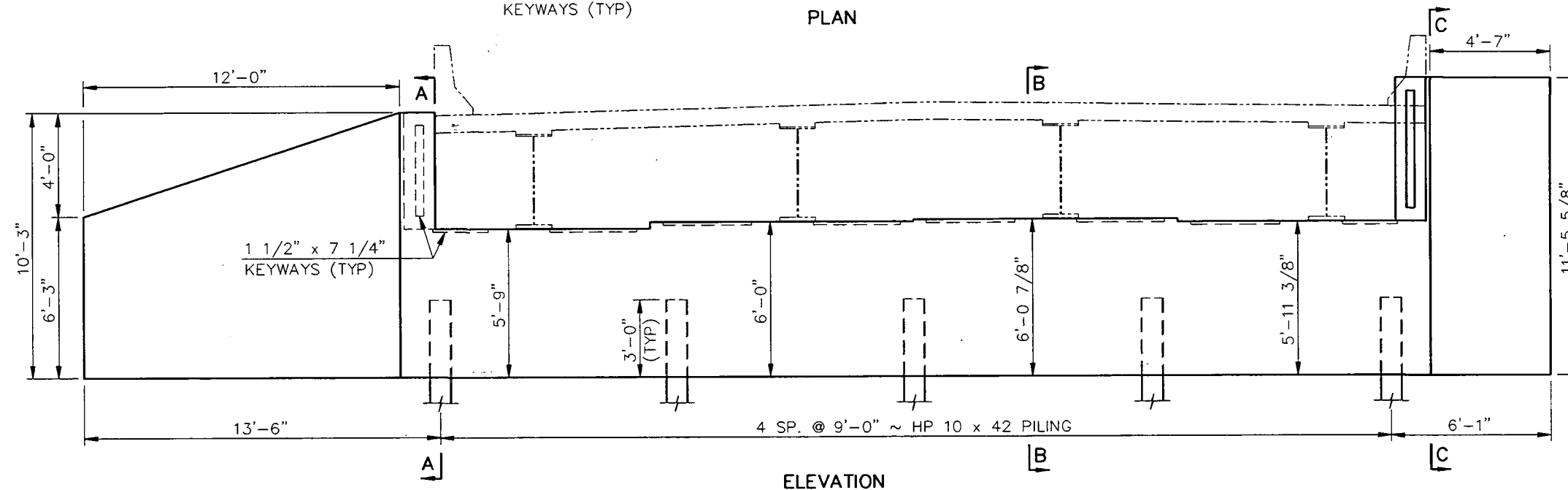
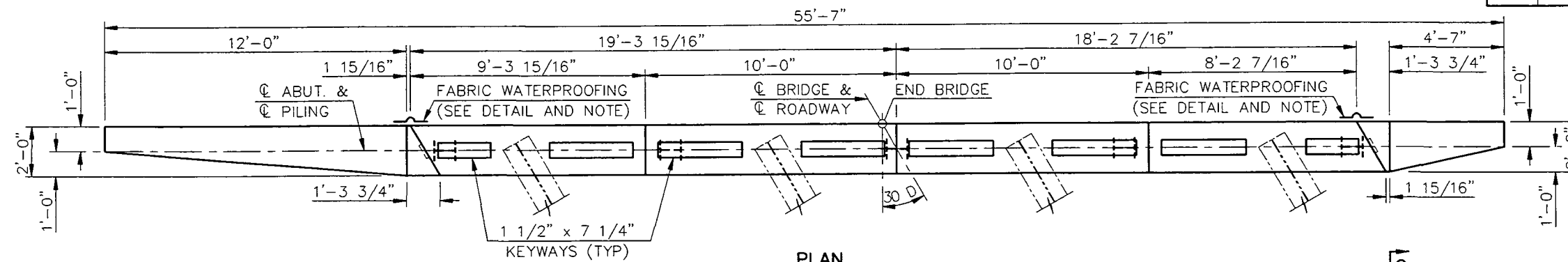
C-C

NOTE:
 THE ELEVATION OF THE TOP OF THE SOUTH WING WALL SHALL MATCH THAT OF THE NORTH WING WALL OF THE ADJACENT STRUCTURE.

QUANTITIES
SEE DWG. 2-053.731L-10
BURLINGTON NORTHERN OVERHEAD RAY
(SHOWING DIMENSIONS)
ABUTMENT 1 DETAILS



QUANTITIES	
CLASS AE-3 CONCRETE	25.8 CY
REINFORCING STEEL	2652 LB
BURLINGTON NORTHERN OVERHEAD RAY	
(SHOWING REINFORCING)	
ABUTMENT 1 DETAILS	

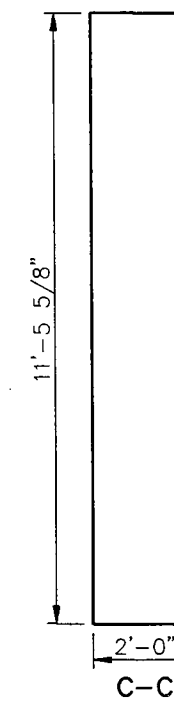
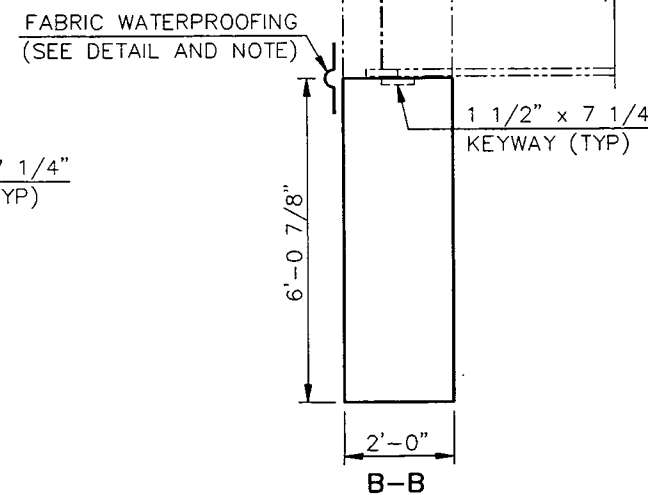
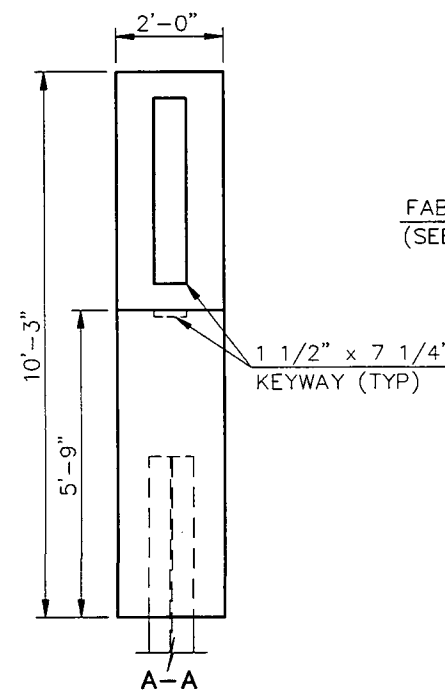


2" (MIN.) WIDTH OF MATERIAL (1'-0" MIN.) 2" (MIN.)

FABRIC WATERPROOFING SHALL BE APPLIED IN ACCORDANCE WITH SECTION 740 OF THE NDDOT SPECIFICATIONS.

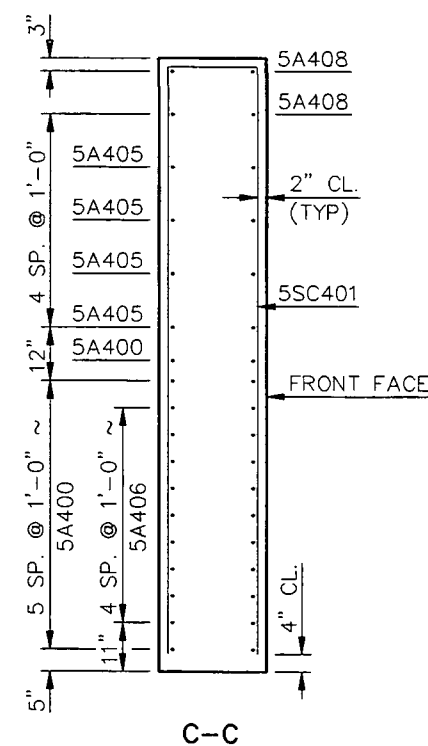
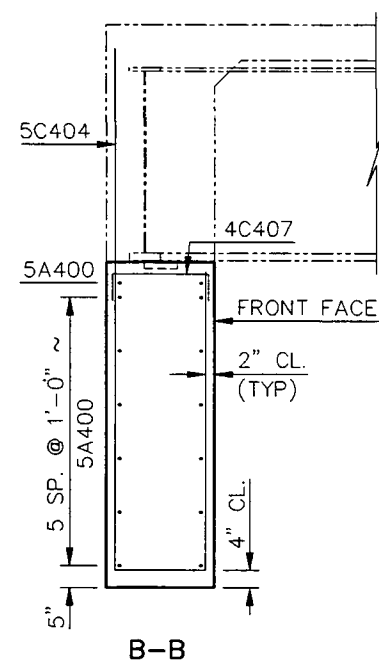
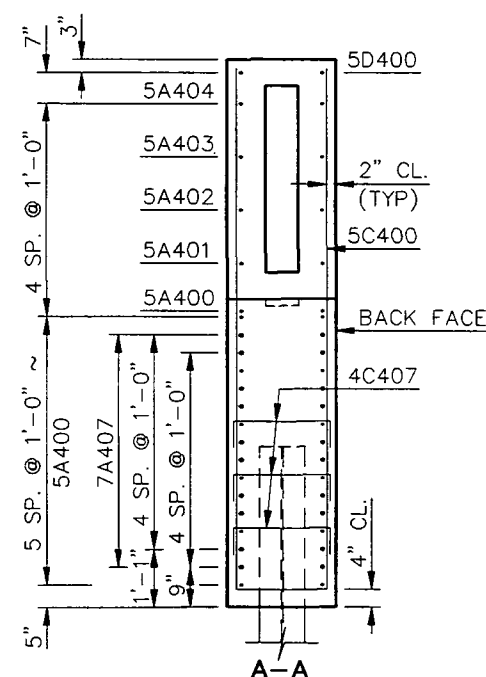
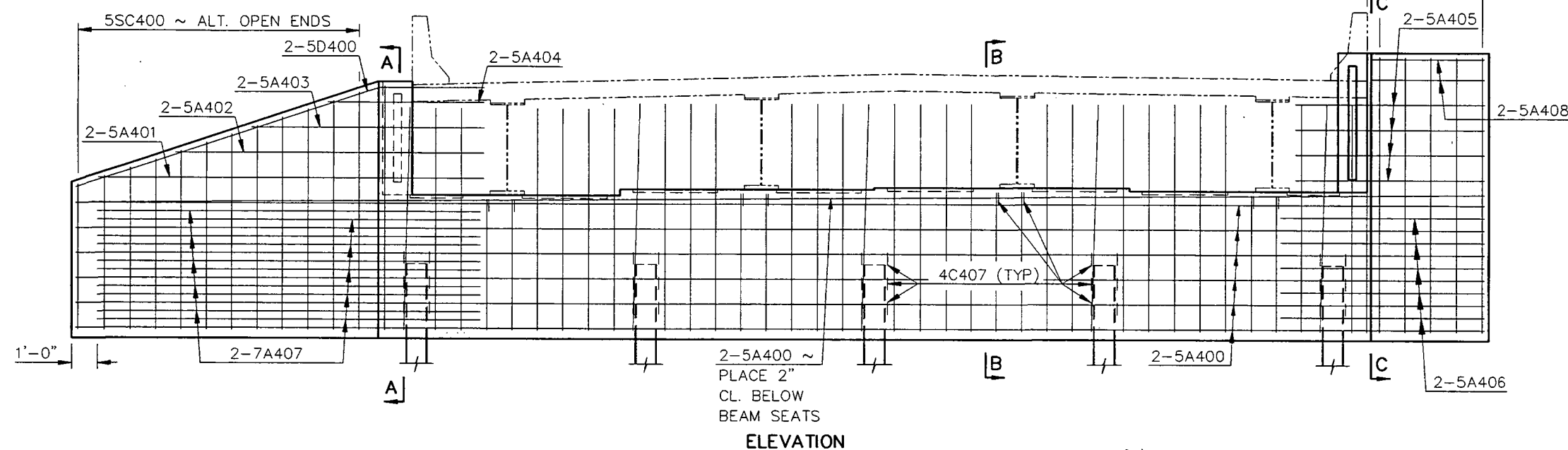
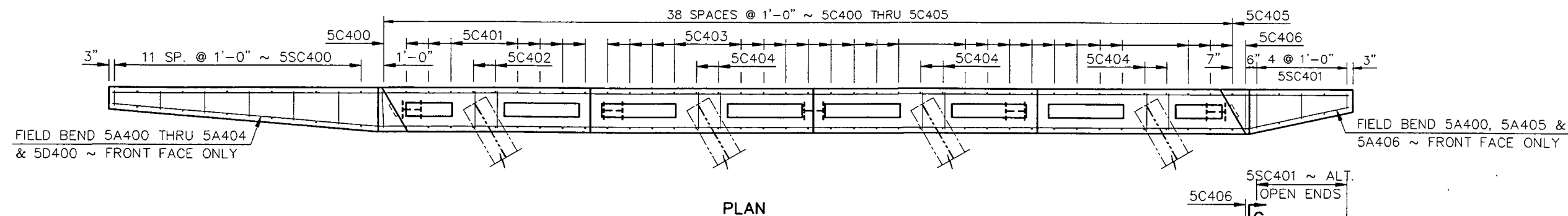
ALL MATERIAL AND WORK SHALL BE CONSIDERED INCIDENTAL TO THE PAY ITEM OF CLASS AE-3 CONCRETE.

TWO-PLY FABRIC WATERPROOFING DETAIL



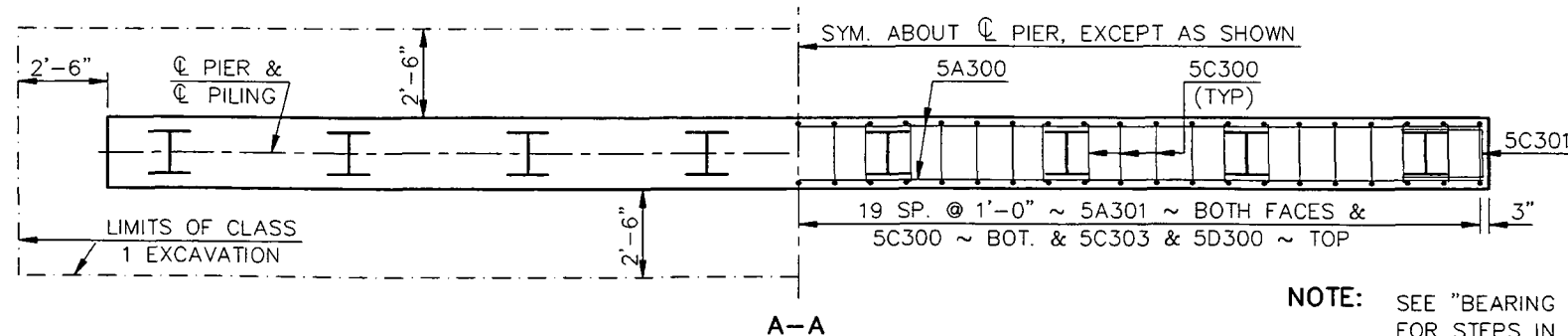
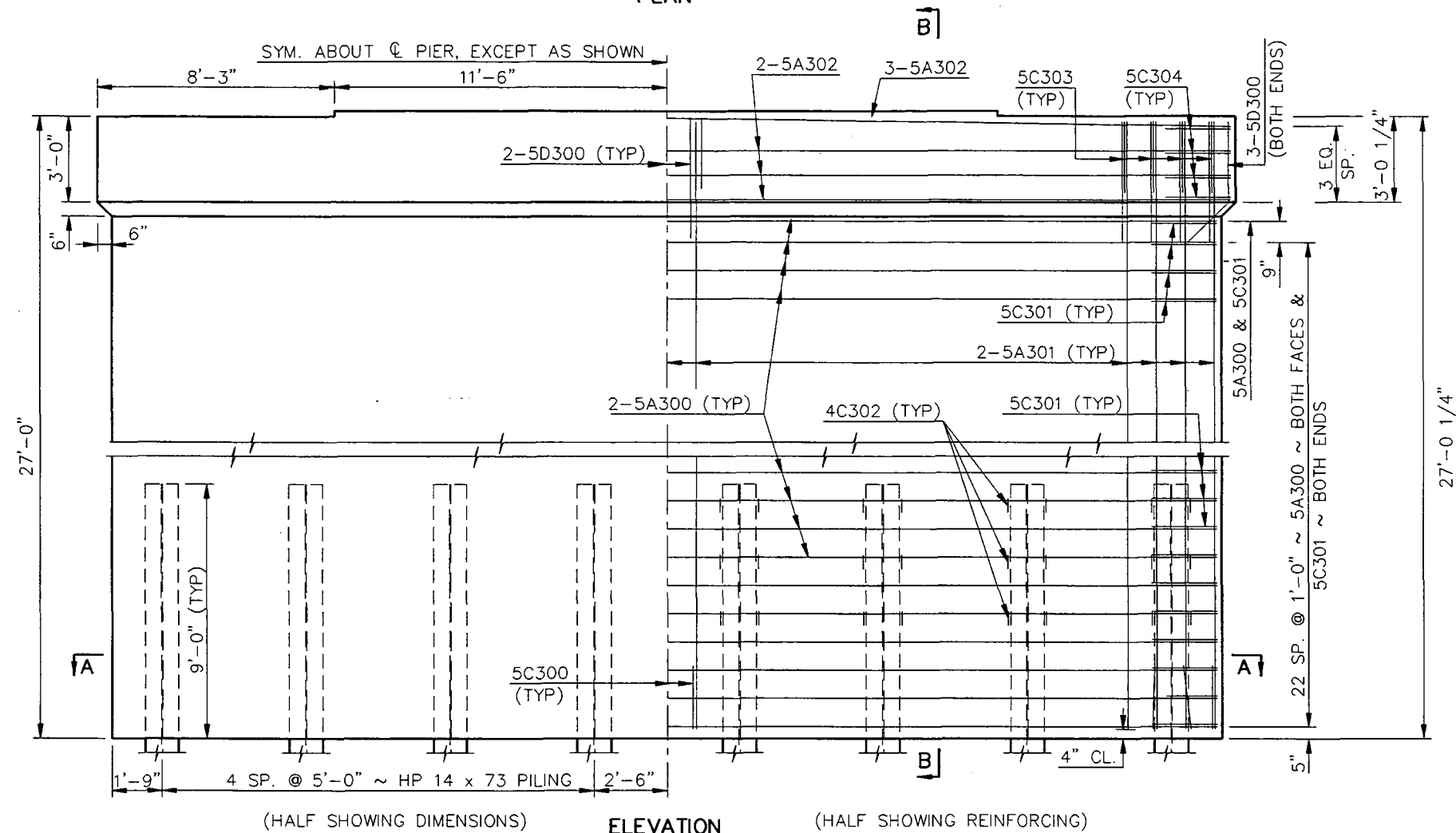
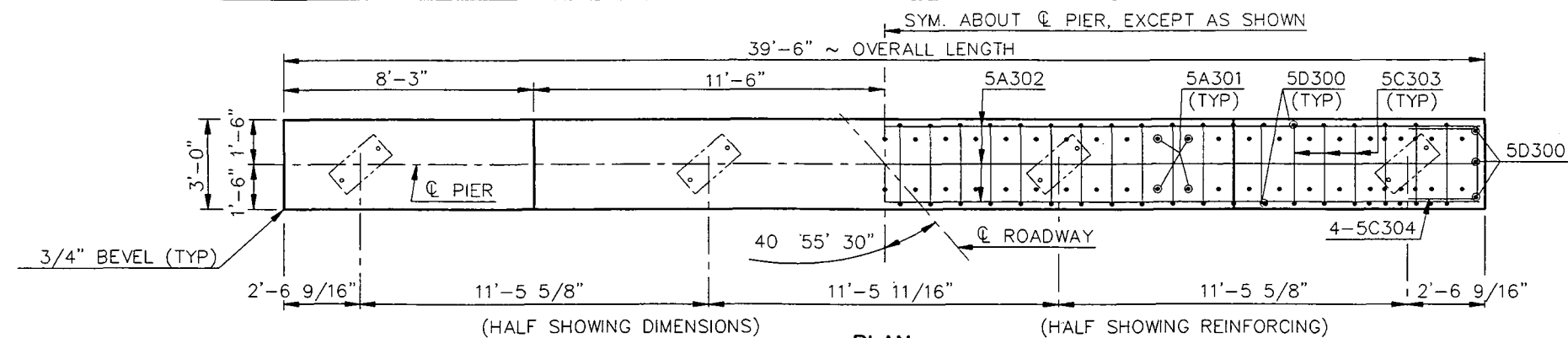
NOTE:
THE ELEVATION OF THE TOP OF THE SOUTH WING WALL SHALL MATCH THAT OF THE NORTH WING WALL OF THE ADJACENT ABUTMENT.

QUANTITIES
SEE DWG. 2-053.731L-12
BURLINGTON NORTHERN OVERHEAD RAY
(SHOWING DIMENSIONS)
ABUTMENT 4 DETAILS

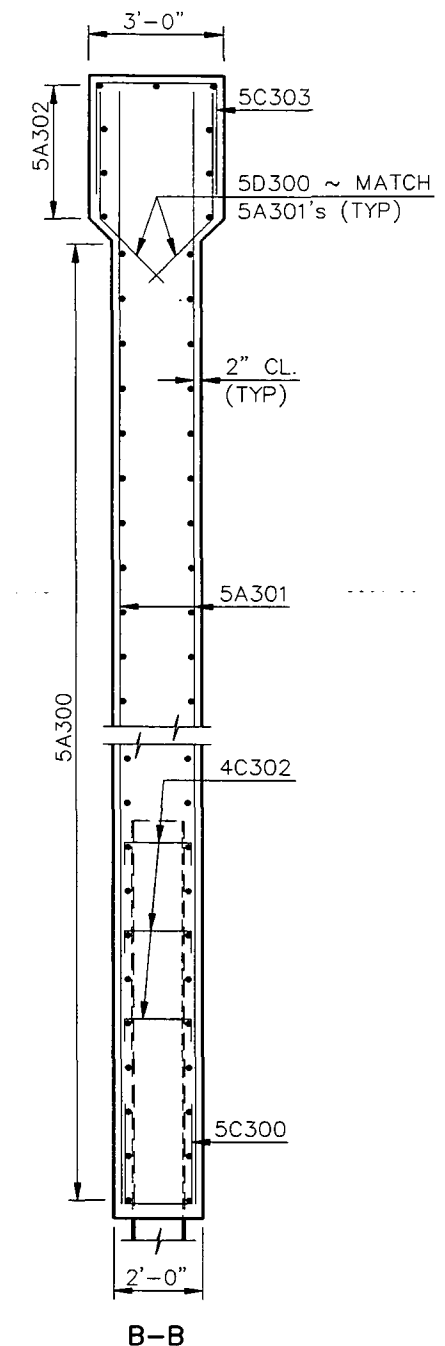


QUANTITIES	
CLASS AE-3 CONCRETE	26.2 CY
REINFORCING STEEL	2918 LB
BURLINGTON NORTHERN OVERHEAD RAY	
(SHOWING REINFORCING)	
ABUTMENT 4 DETAILS	

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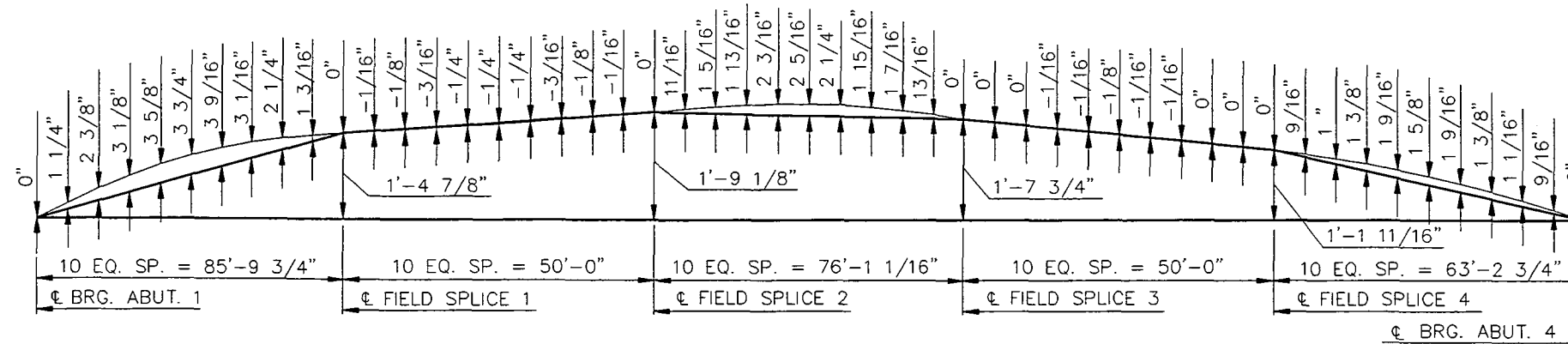
NOTE: SEE "BEARING ELEVATIONS", DWG. 2-053.731L-7 FOR STEPS IN PIER CAP.



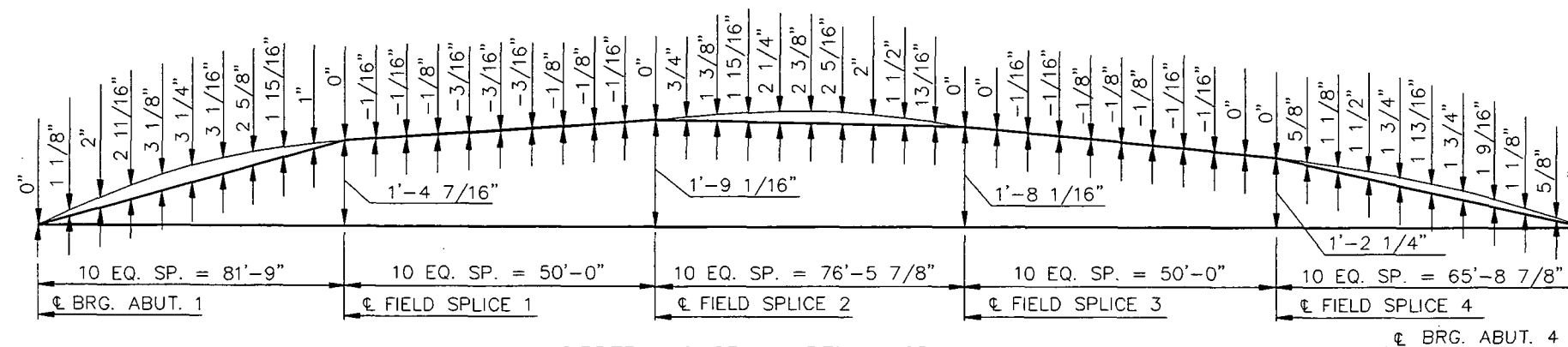
QUANTITIES	
CLASS AE-3 CONCRETE	82.5 CY
REINFORCING STEEL	5865 LB
EXCAVATION (SEE LAYOUT)	

BURLINGTON NORTHERN OVERHEAD RAY

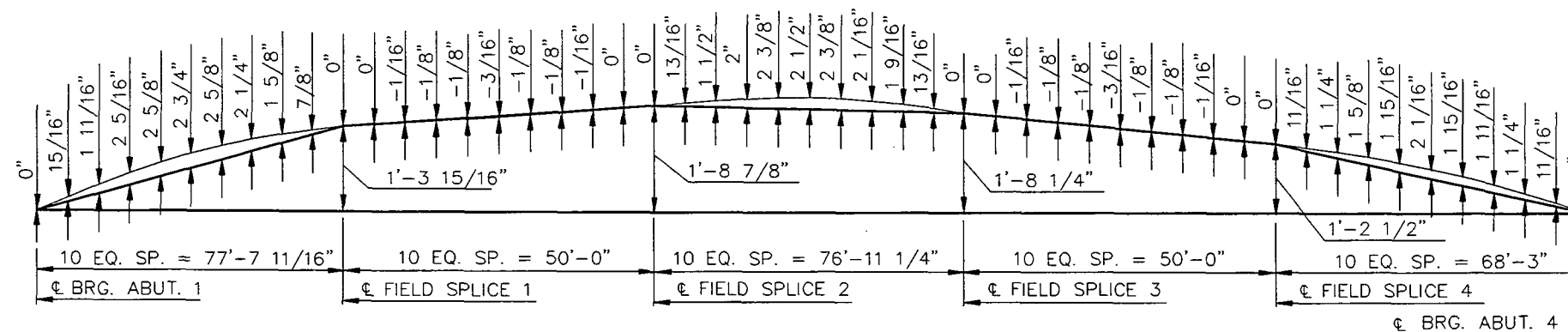
PIER 3 DETAILS



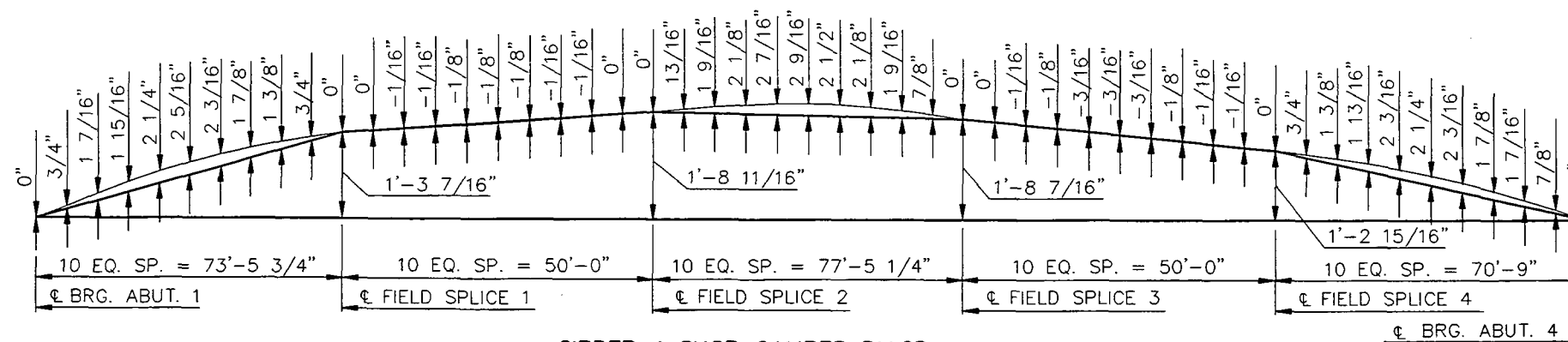
GIRDER 1 SHOP CAMBER DIAGRAM



GIRDER 2 SHOP CAMBER DIAGRAM



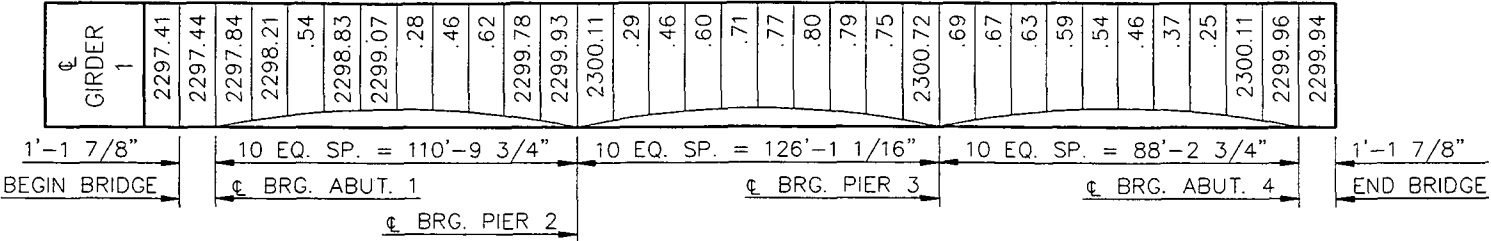
GIRDER 3 SHOP CAMBER DIAGRAM



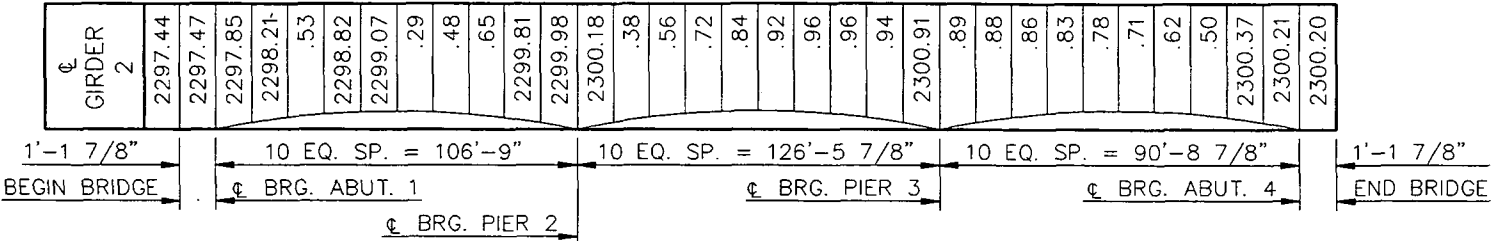
GIRDER 4 SHOP CAMBER DIAGRAM

BURLINGTON NORTHERN OVERHEAD
RAY

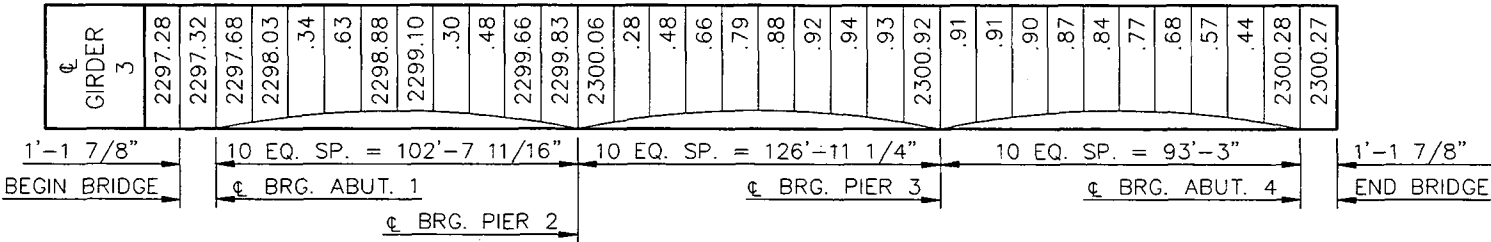
SHOP CAMBER DIAGRAMS



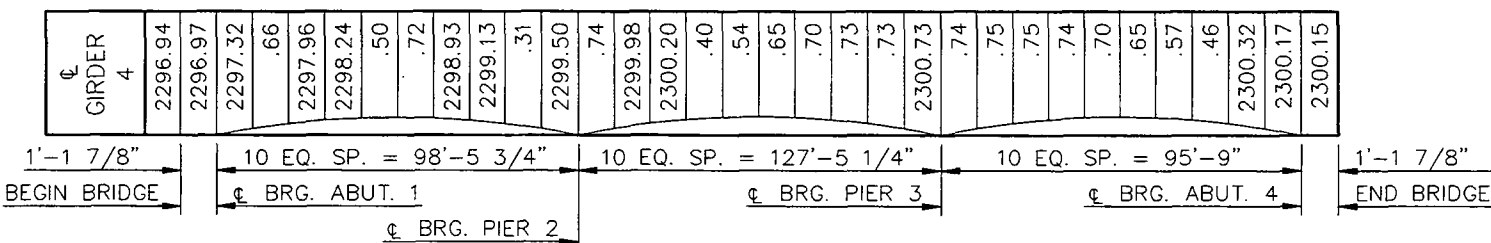
GIRDER 1 SCREED ELEVATIONS



GIRDER 2 SCREED ELEVATIONS



GIRDER 3 SCREED ELEVATIONS

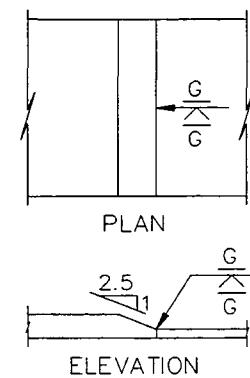
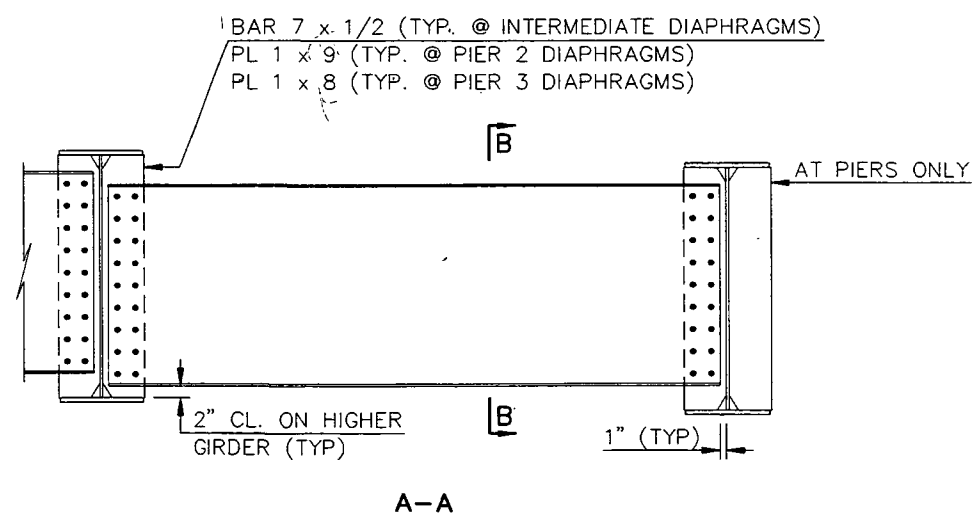
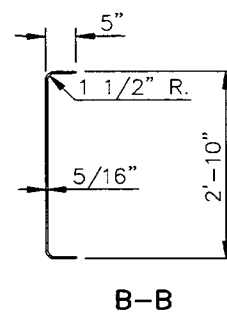
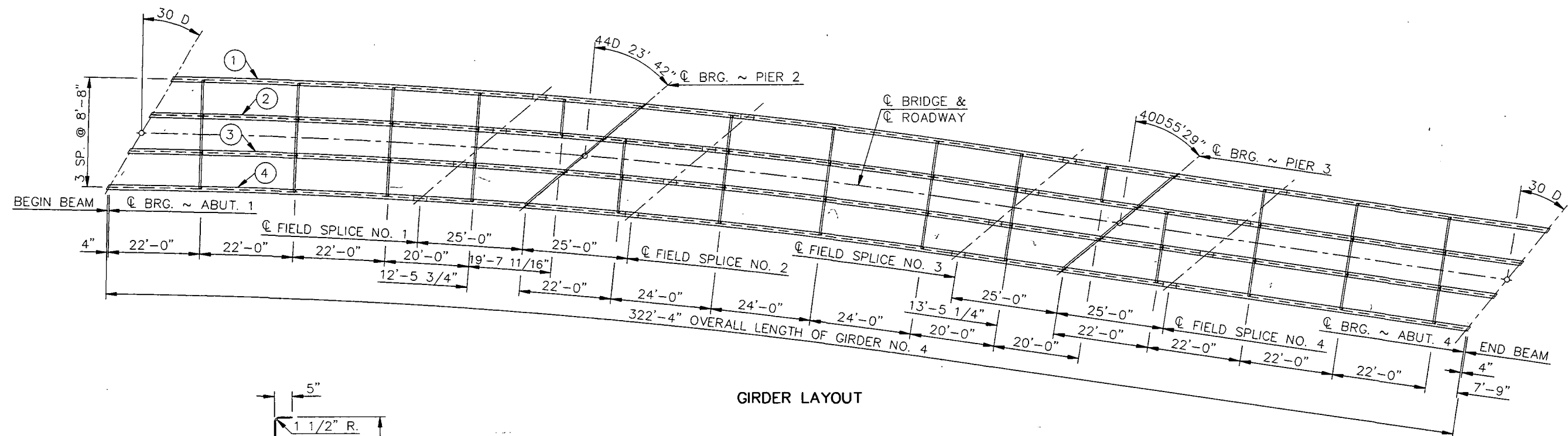


GIRDER 4 SCREED ELEVATIONS

BURLINGTON NORTHERN OVERHEAD
RAY

SCREED ELEVATION

2-053.731L-16



NOTE:

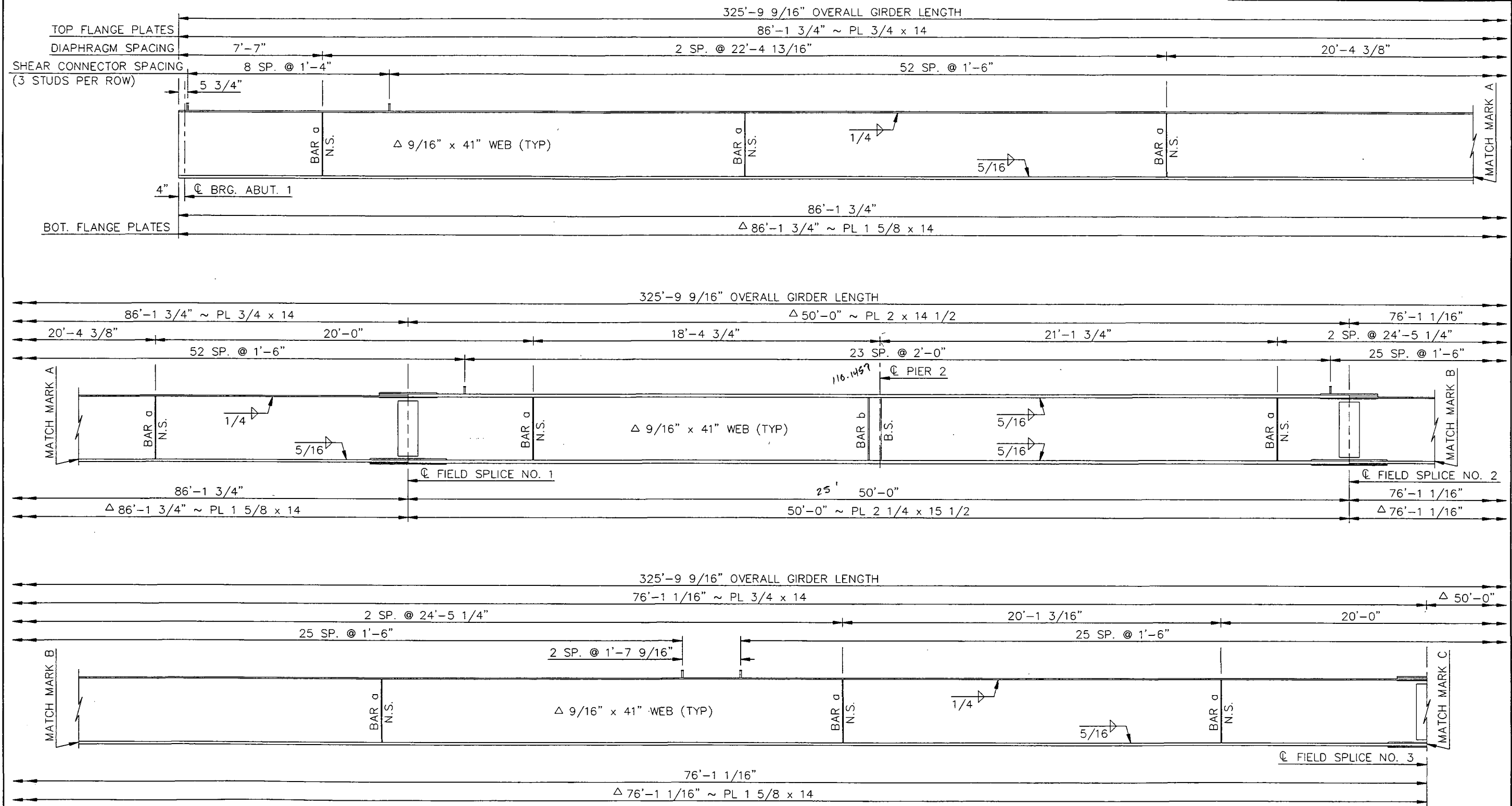
THE DIMENSIONS SHOWN ON THE GIRDER LAYOUT ARE MEASURED ALONG THE CENTERLINE OF THE OUTSIDE GIRDER, GIRDER NO. 4.

ALL INTERMEDIATE DIAPHRAGMS IN THE TANGENT SECTIONS SHALL BE PLACED PERPENDICULAR. ALL INTERMEDIATE DIAPHRAGMS IN THE CURVED SECTION SHALL BE PLACED ALONG THE RADIAL.

BURLINGTON NORTHERN OVERHEAD
RAY

GIRDER DETAILS

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
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NOTES:

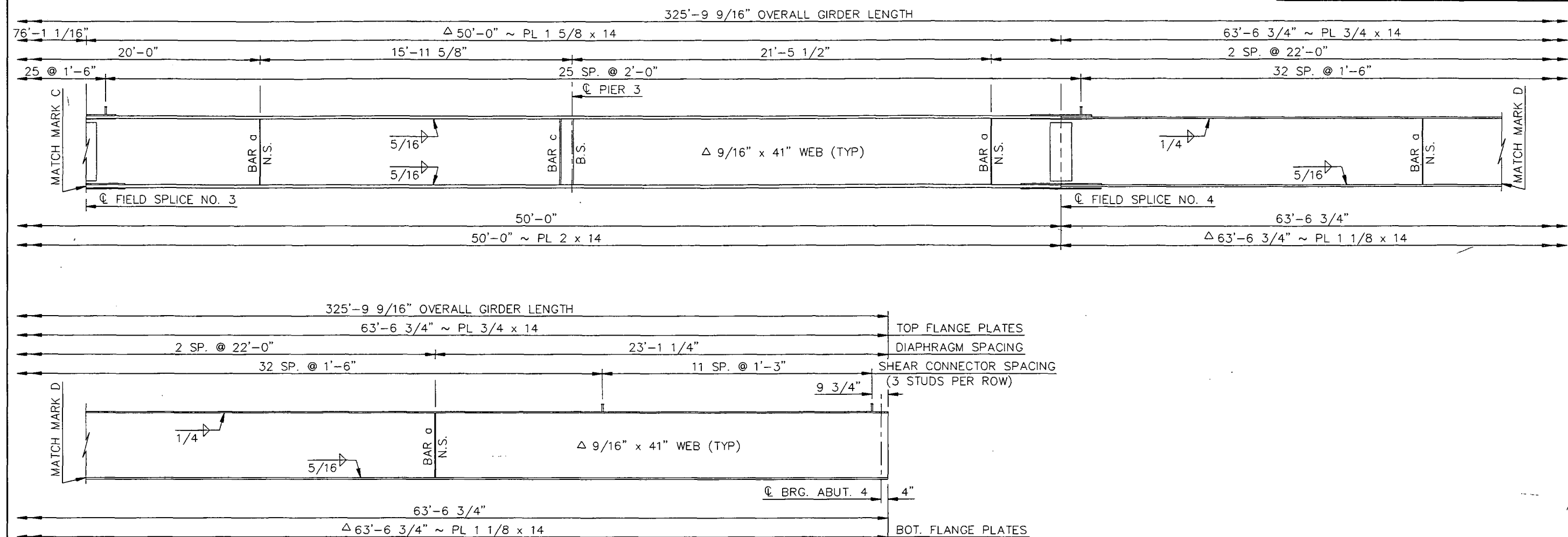
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES

SEE DWG. 2-053.731L-19 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 1 ELEVATION DETAILS



NOTES:

Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

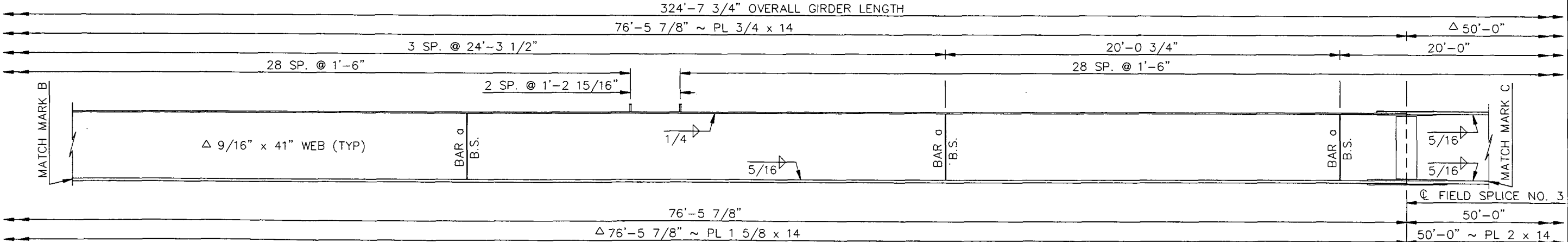
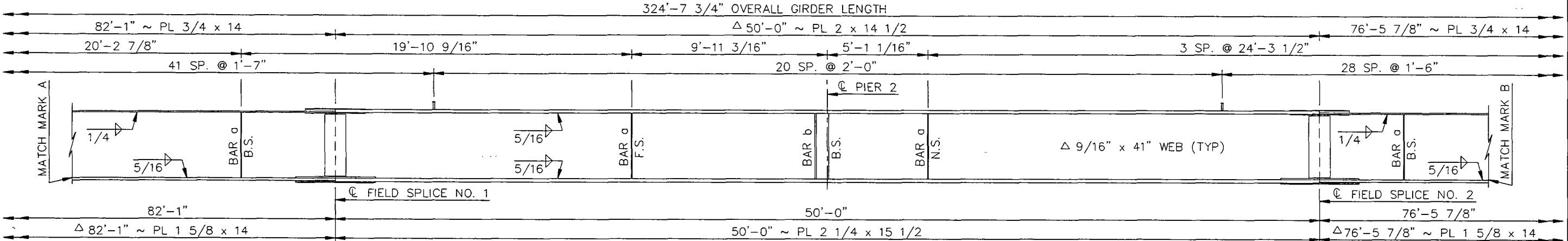
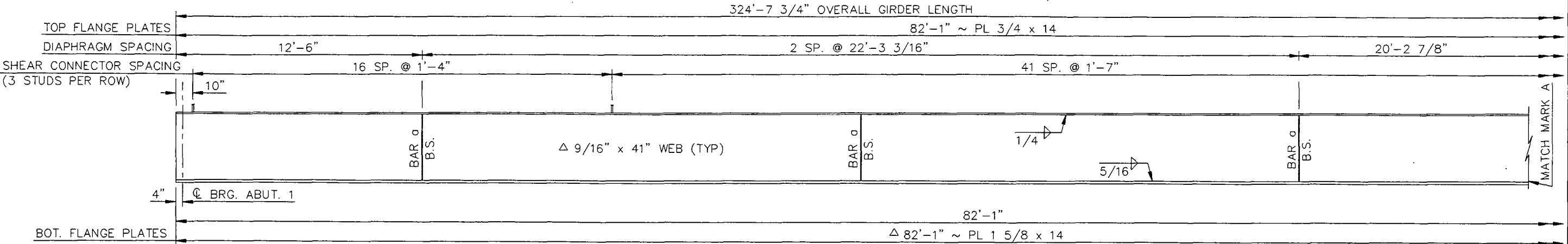
NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES

SEE DWG. 2-053.731L-18 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 1 ELEVATION DETAILS

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
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NOTES:

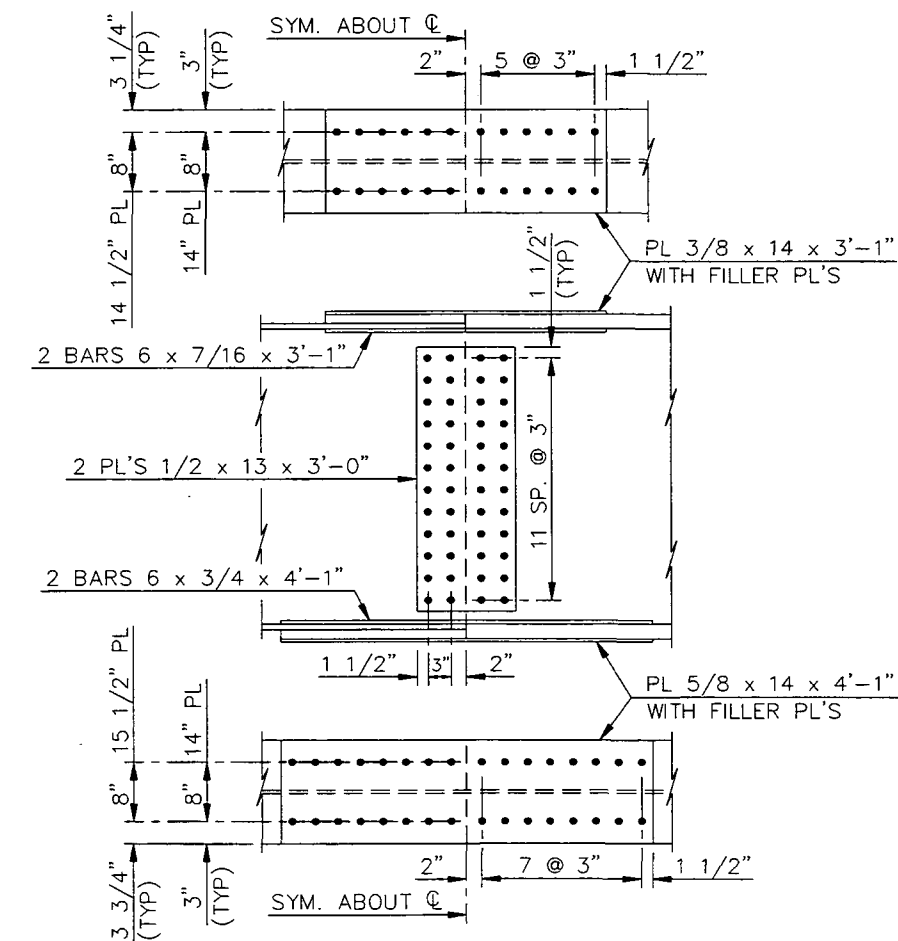
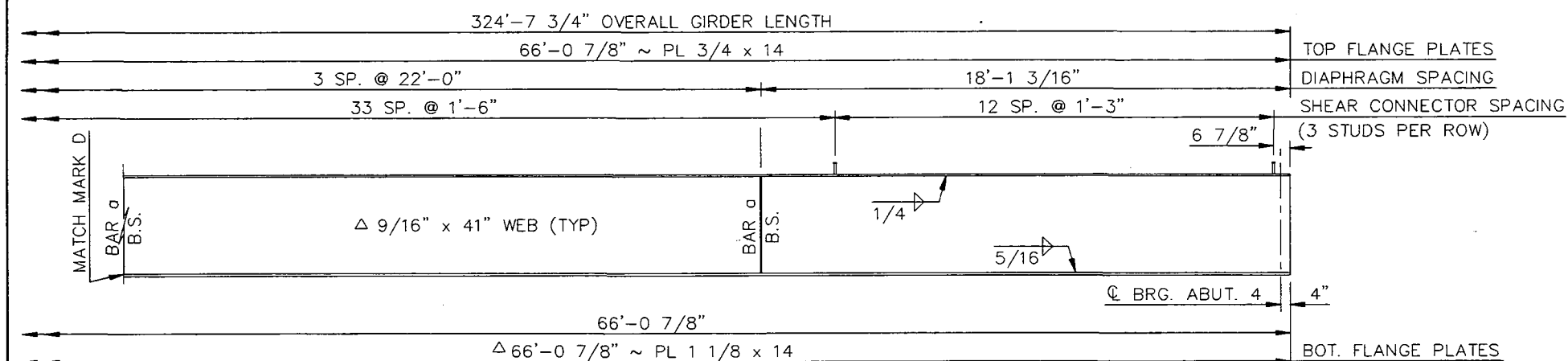
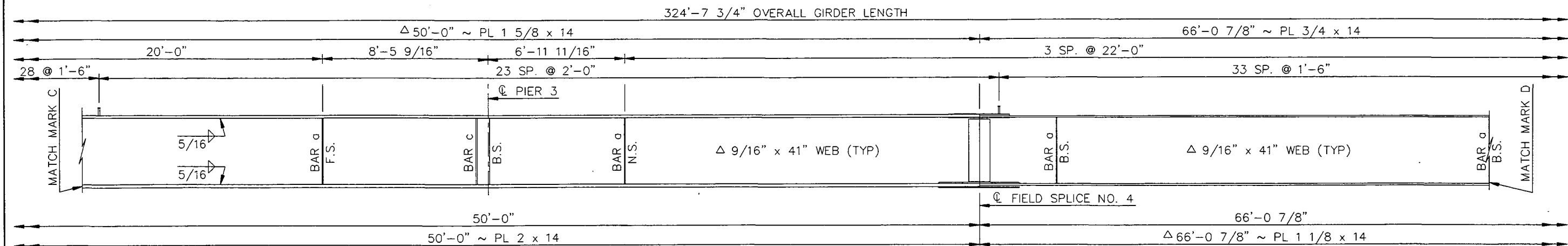
△ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

SEE DWG. 2-053.731L-21 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 2 ELEVATION DETAILS



FIELD SPLICE DETAILS

NOTES:

Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

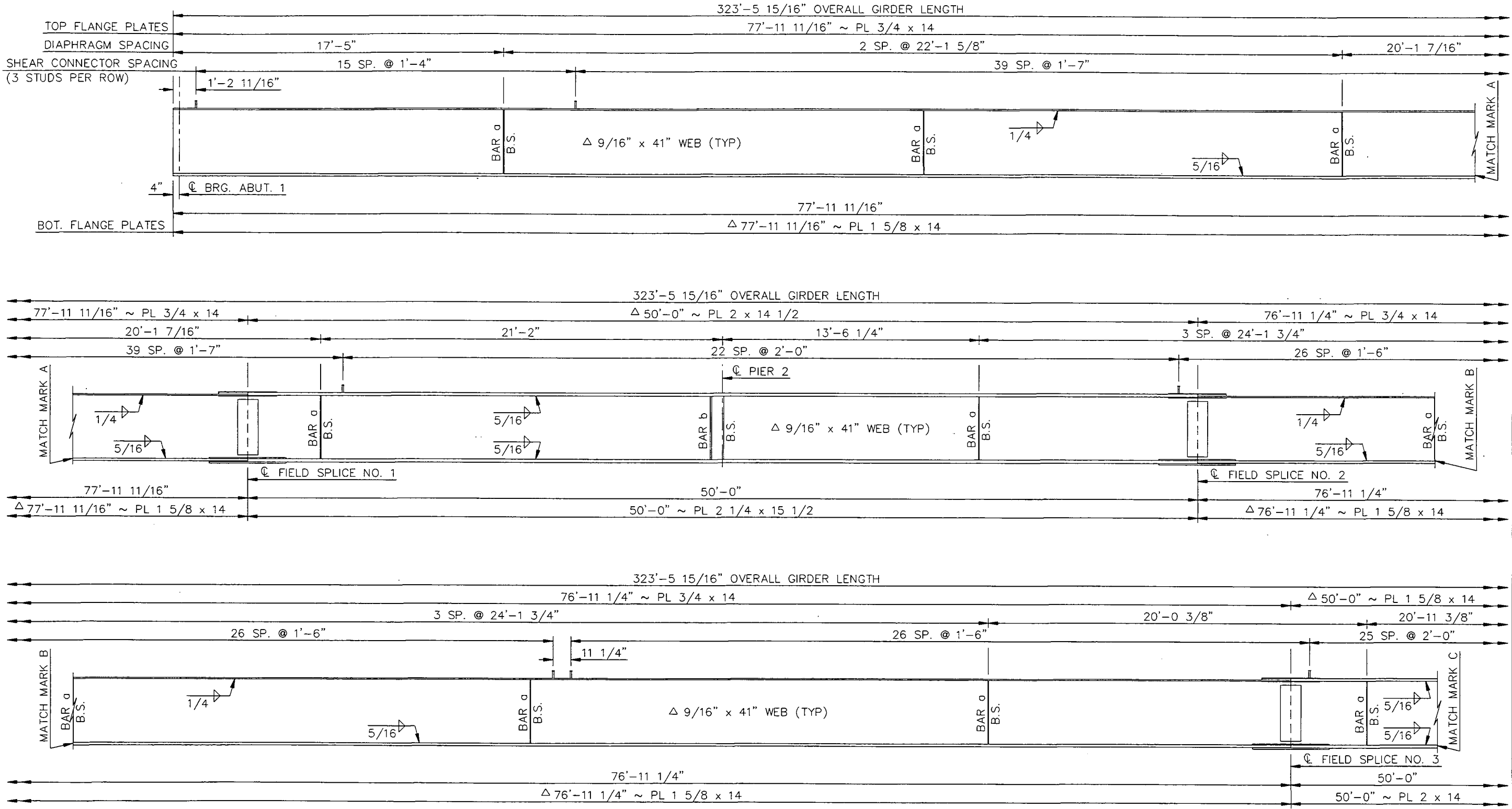
SEE DWG. 2-053.731L-20 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 2 ELEVATION DETAILS
& FIELD SPLICE DETAILS

2-053.731L-21

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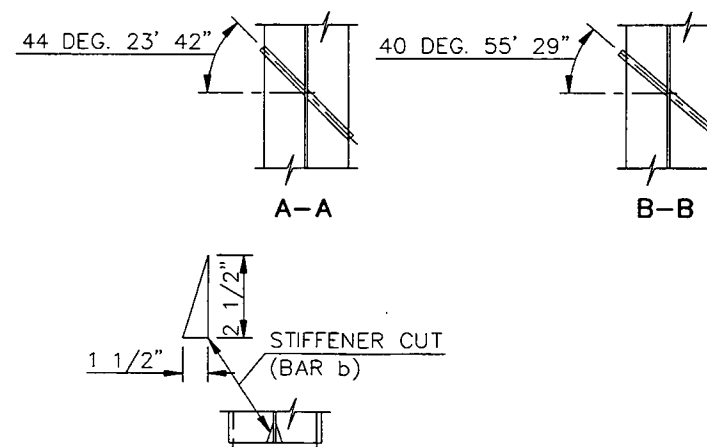
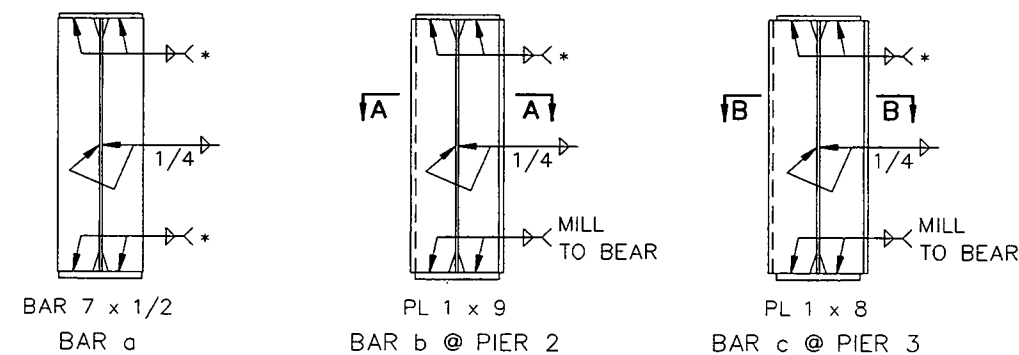
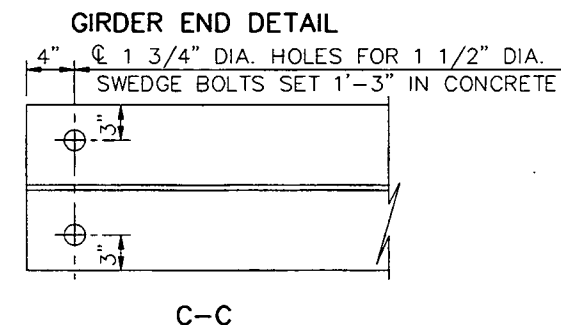
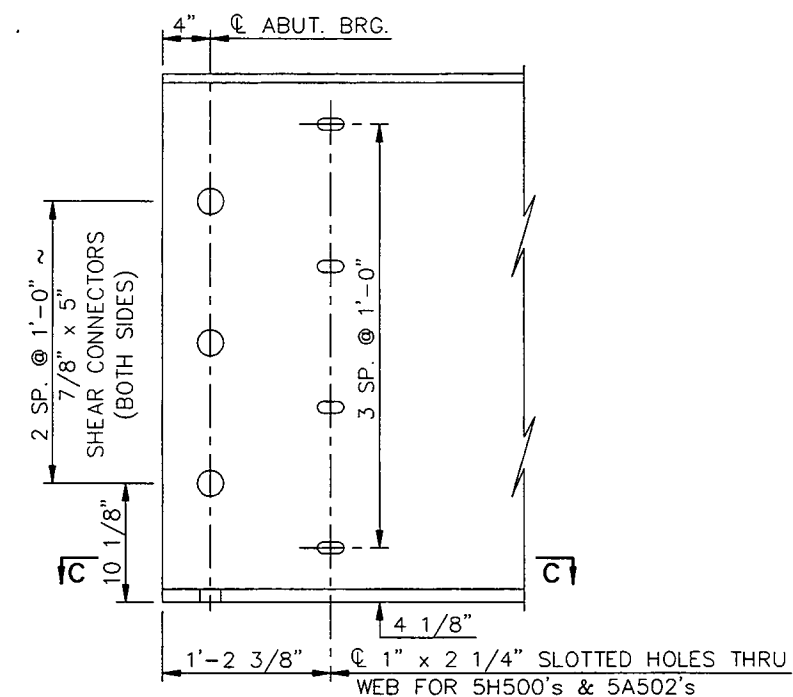
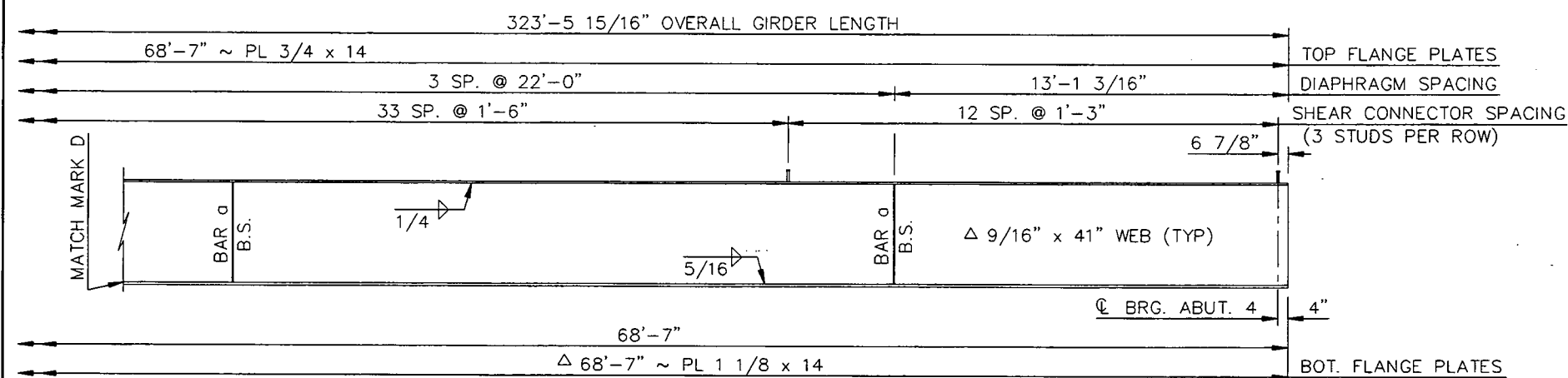
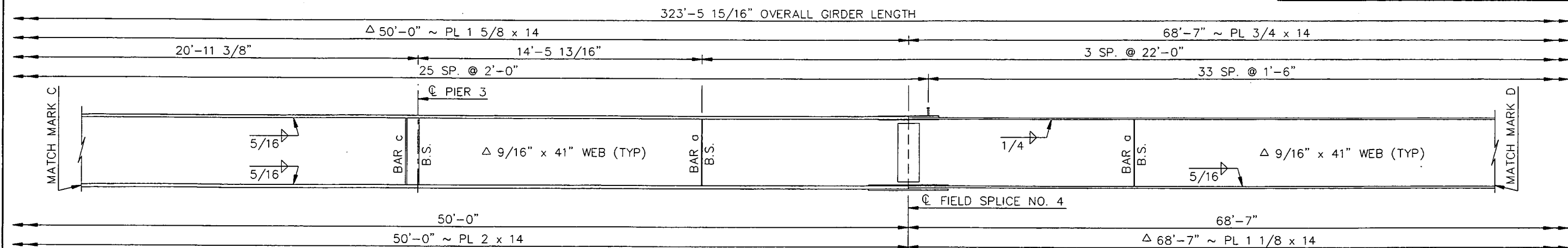
NOTES:
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

SEE DWG. 2-053.731L-23 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 3 ELEVATION DETAILS



STIFFENER & DIAPHRAGM CONNECTION PLATE DETAILS

* WELD SIZE 1/4 5/16

FLANGE THICKNESS TO 3/4 OVER 3/4

NOTES:

Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

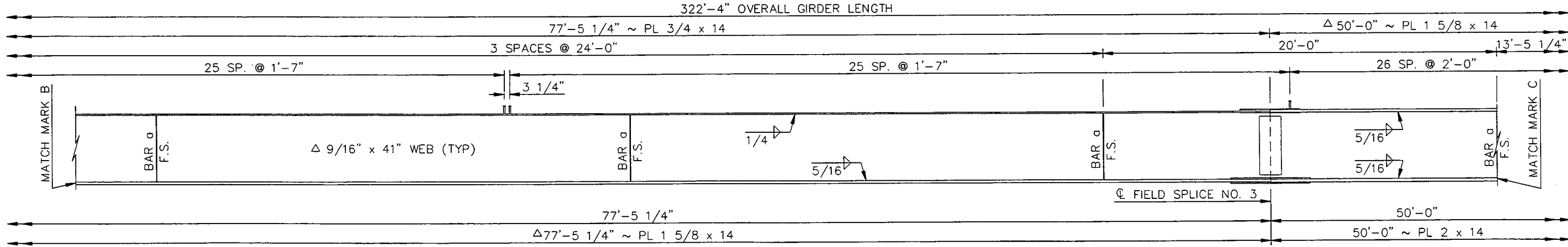
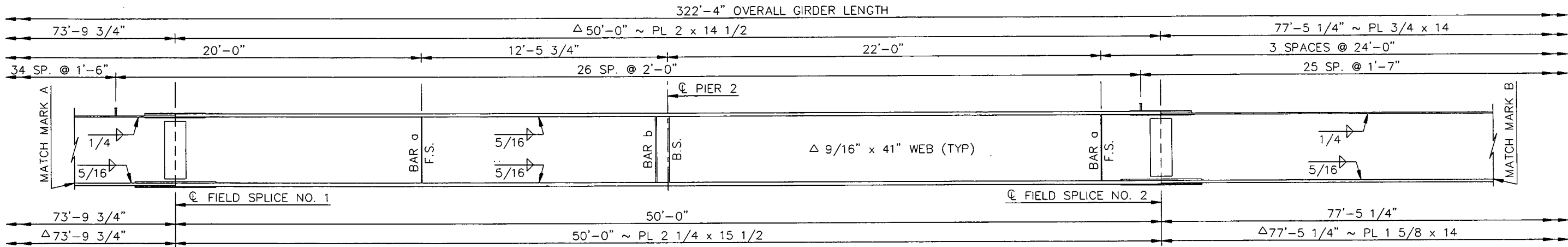
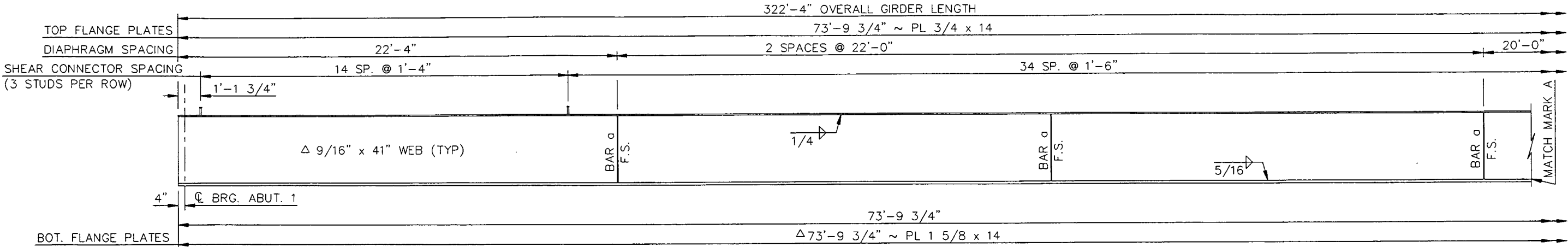
NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

SEE DWG. 2-053.731L-22 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 3 ELEVATION DETAILS, GIRDER END DETAIL, STIFFENER & DIAPHR. CONNECTION PL DETAILS

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	112



NOTES:

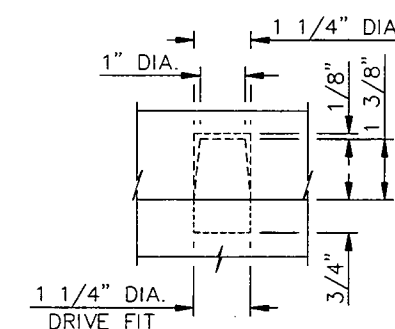
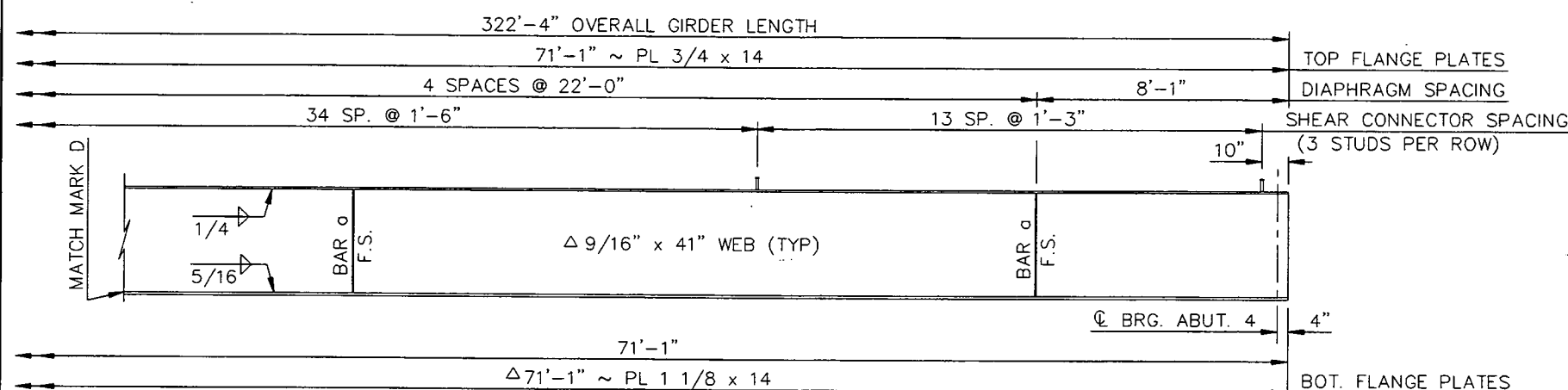
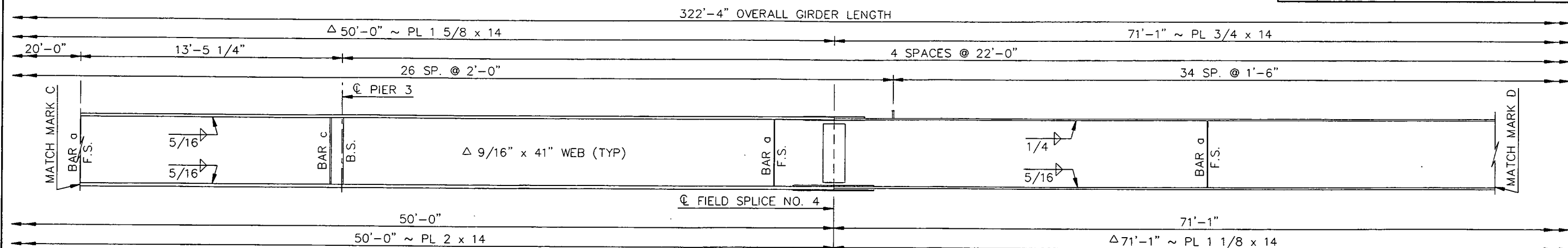
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES

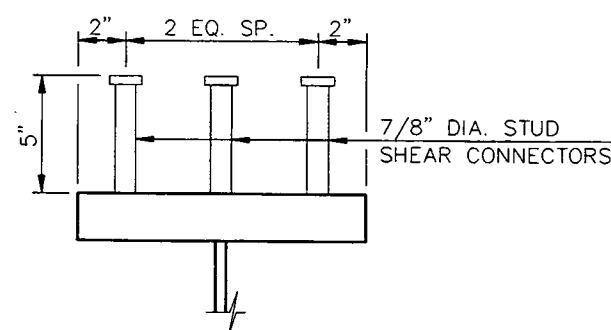
SEE DWG. 2-053.731L-25 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

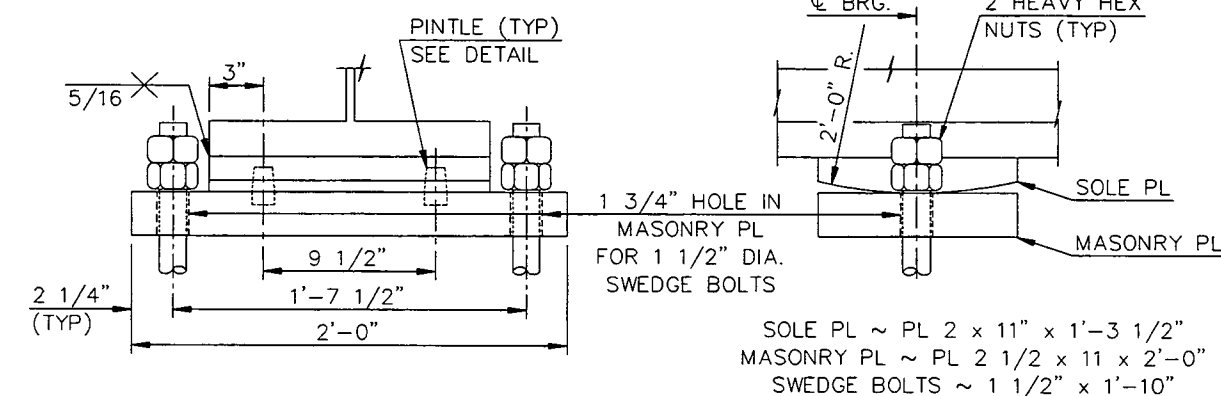
GIRDER 4 ELEVATION DETAILS



PINTLE DETAIL



(3 STUDS PER ROW)
SHEAR CONNECTOR DETAIL



PIER BEARING DETAILS

NOTES:

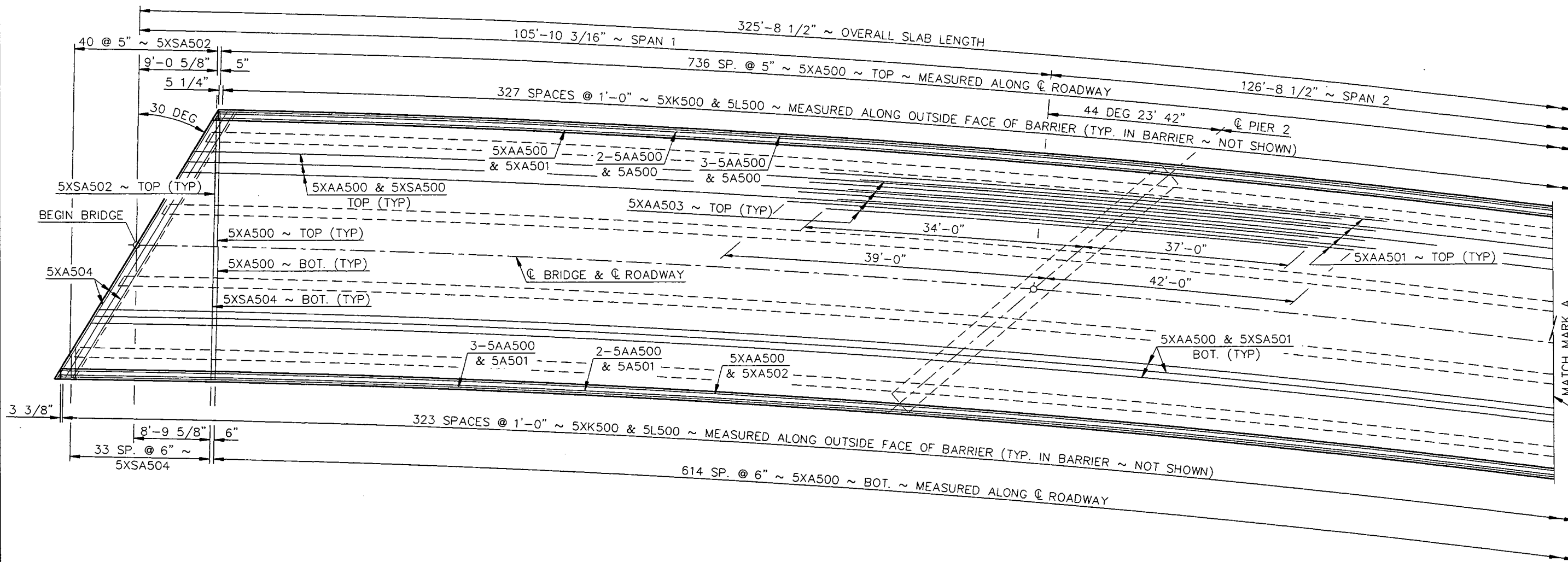
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

SEE DWG. 2-053.731L-24 FOR MATCH MARK C.

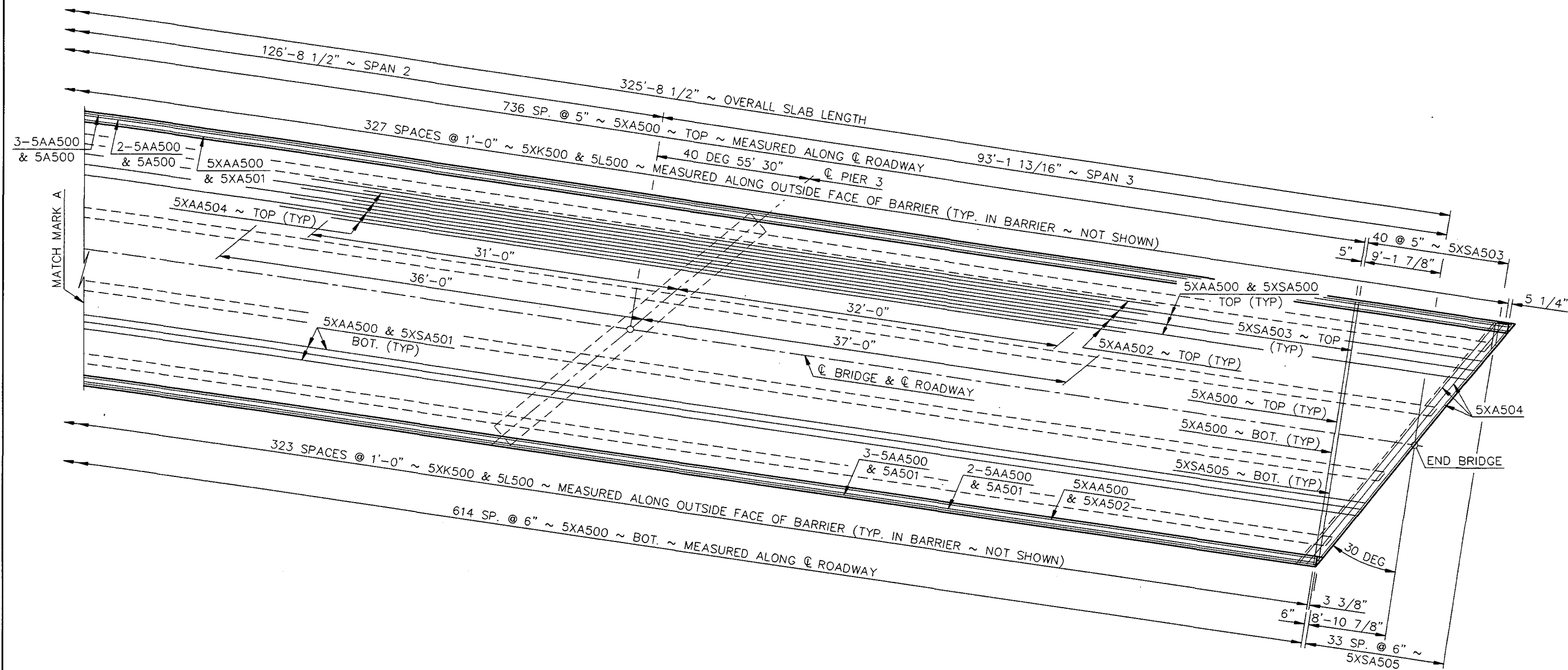
BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 4 ELEVATION DETAILS,
PIER BEARING DETAILS &
SHEAR CONNECTOR DETAIL



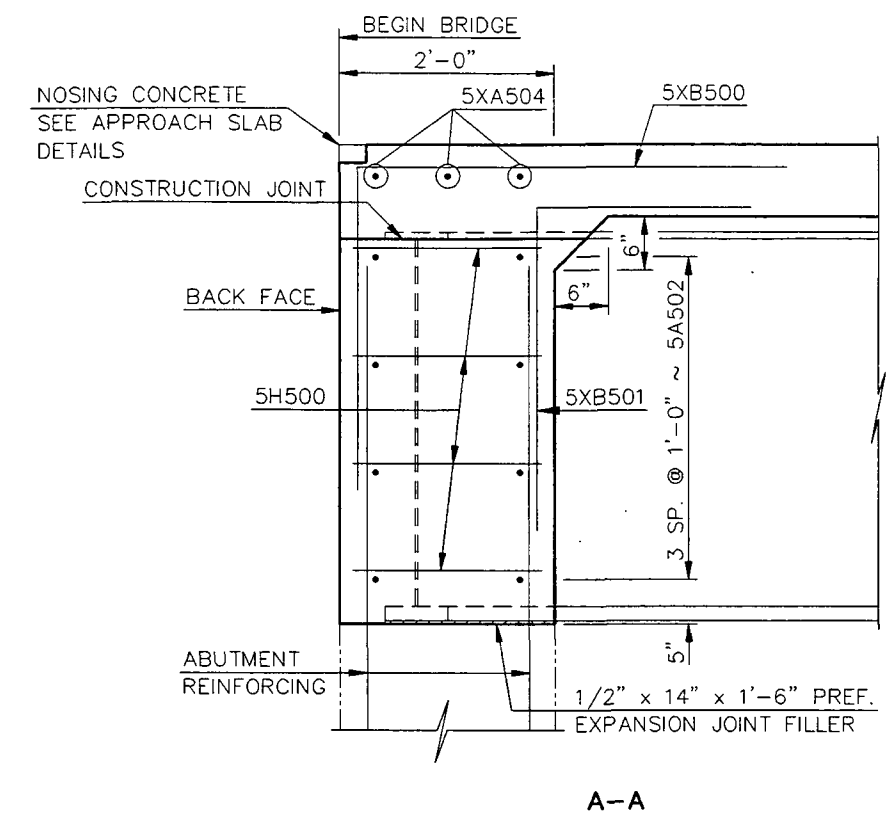
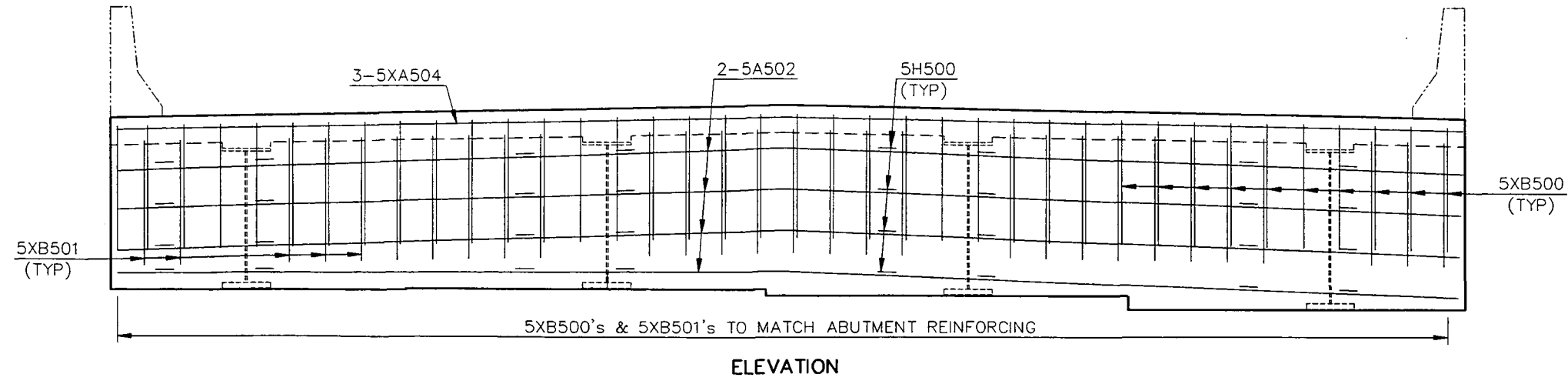
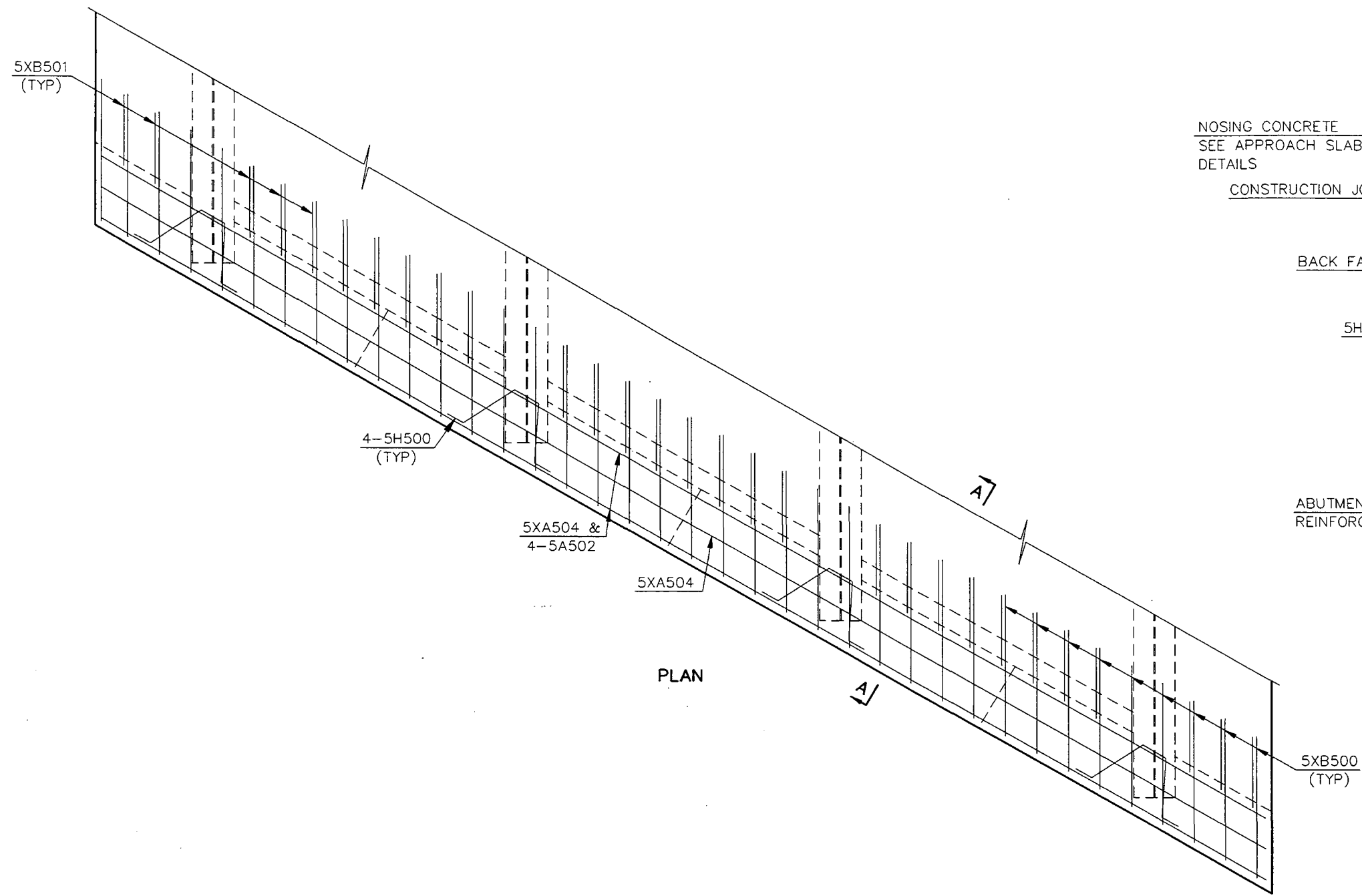
NOTE:
SEE DWG. 2-053.731L-27 FOR MATCH MARK A.

QUANTITIES
SEE DWG. 2-053.731L-29
BURLINGTON NORTHERN OVERHEAD RAY
SLAB LAYOUT



NOTE:
SEE DWG. 2-053.731L-26 FOR MATCH MARK A.

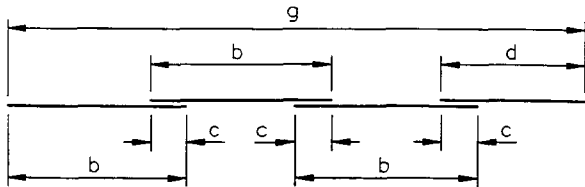
QUANTITIES
SEE DWG. 2-053.731L-29
BURLINGTON NORTHERN OVERHEAD RAY
SLAB LAYOUT



QUANTITIES
SEE DWG. 2-053.731L-29
BURLINGTON NORTHERN OVERHEAD RAY
END BEAM DETAILS

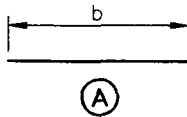
BILL OF REINFORCING STEEL, GRADE 60																							FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.		
LETTER PREFIX OF BAR MARK DENOTES SHAPE ~ SEE BAR DETAILS																							8	ND	RSN-7-002(041)052	118		
LOCATION	SIZE	MARK	NO. EACH /SET	NOMINAL LENGTH	DETAILING DIMENSIONS								LOCATION	SIZE	MARK	NO. EACH /SET	NOMINAL LENGTH	DETAILING DIMENSIONS										
					a	b	c	d	e	f	g	h						k	a	b	c	d	e	f	g	h	k	
ABUTMENT 1	5	A100	12	56'-6"		56'-6"									5	A400	14	55'-3"		55'-3"								
	5	A101	2	41'-2"		41'-2"									5	A401	2	15'-0"		15'-0"								
	5	A102	2	13'-0"		13'-0"									5	A402	2	12'-0"		12'-0"								
	5	A103	2	10'-0"		10'-0"									5	A403	2	9'-0"		9'-0"								
	5	A104	2	7'-0"		7'-0"									5	A404	2	6'-0"		6'-0"								
	5	A105	8	9'-10"		9'-10"									5	A405	8	7'-5"		7'-5"								
	5	A106	6	8'-0"		8'-0"									5	A406	10	8'-0"		8'-0"								
	7	A107	16	15'-0"		15'-0"									7	A407	20	15'-0"		15'-0"								
															5	A408	4	4'-3"		4'-3"								

- NOTES:
- FABRICATION AND TOLERANCES SHALL BE IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.
 - ALL DIMENSIONS ARE OUT TO OUT OF BARS.
 - NOMINAL LENGTH OF EACH BENT BAR OR CUT BAR IS THE SUM TOTAL OF THE DETAILING DIMENSIONS FOR THAT BAR, UNLESS OTHERWISE NOTED.
 - ADJACENT "AA" BARS SHALL BE TURNED END FOR END SO THAT THE SPLICE LOCATIONS ARE STAGGERED.

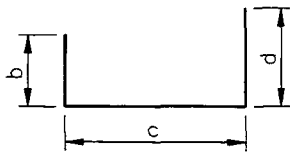


c = LAP SPLICE (TYP.)
e = NO. OF "b" LENGTH PIECES IN A SET
TOTAL LENGTH PER SET = e x b + d

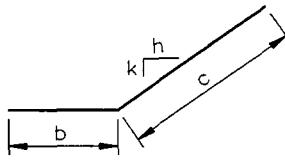
AA



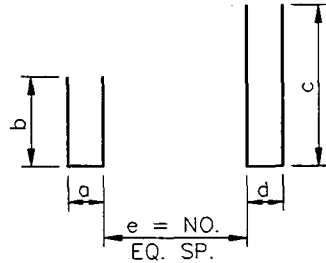
A



C



D



SC

BURLINGTON NORTHERN OVERHEAD
RAY

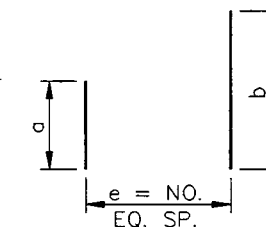
ABUTMENT 1 & ABUTMENT 4
REINFORCING BAR LIST
& DETAILS

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	119

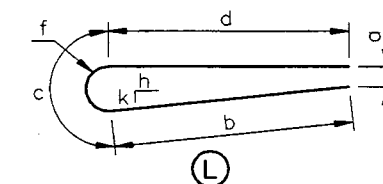
NOTES:

1. FABRICATION AND TOLERANCES SHALL BE IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.
2. ALL DIMENSIONS ARE OUT TO OUT OF BARS.
3. NOMINAL LENGTH OF EACH BENT BAR OR CUT BAR IS THE SUM TOTAL OF THE DETAILING DIMENSIONS FOR THAT BAR, UNLESS OTHERWISE NOTED.
4. ADJACENT "AA" BARS SHALL BE TURNED END FOR END SO THAT THE SPLICE LOCATIONS ARE STAGGERED.
5. THE "r" DIMENSION INDICATES THE RADIUS.
6. AN "x" PRECEDING A BAR DESIGNATION INDICATES AN EPOXY COATED BAR.

LOCAL- TION	SIZE	MARK	NO. EACH /SET	NOMINAL LENGTH	DETAILING DIMENSIONS									LOCAL- TION	SIZE	MARK	NO. EACH /SET	NOMINAL LENGTH	DETAILING DIMENSIONS									
					a	b	c	d	e	f	g	h	k						a	b	c	d	e	f	g	h	k	
PIER 2	5	A200	46	40'-8"		40'-8"																						
	5	A201	82	25'-0"		25'-0"																						
	5	A202	9	41'-8"		41'-8"																						
	5	C200	41	6'-2"		2'-3"	1'-8"	2'-3"																				
	5	C201	44	6'-0"		2'-3"	1'-6"	2'-3"																				
	4	C202	54	2'-8"		6"	1'-8"	6"																				
	5	C203	41	7'-8"		2'-6"	2'-8"	2'-6"																				
	5	C204	8	7'-0"		2'-3"	2'-6"	2'-3"																				
	5	D200	85	4'-10"		2'-10"	2'-0"																12	12				
	5	D201	3	5'-3"		3'-3"	2'-0"																12	12				
	PIER 3	5	A300	48	38'-2"		38'-2"																					
5		A301	78	26'-8"		26'-8"																						
5		A302	9	39'-2"		39'-2"																						
5		C300	39	6'-2"		2'-3"	1'-8"	2'-3"																				
5		C301	48	6'-0"		2'-3"	1'-6"	2'-3"																				
4		C302	48	2'-8"		6"	1'-8"	6"																				
5		C303	39	7'-8"		2'-6"	2'-8"	2'-6"																				
5		C304	8	7'-0"		2'-3"	2'-6"	2'-3"																				
5		D300	84	4'-10"		2'-10"	2'-0"																12	12				
SUPERSTRUCTURE		REGULAR	5	A500	5	33'-8"		33'-8"																				
			5	A501	5	29'-5"		29'-5"																				
			5	A502	16	37'-1"		37'-1"																				
	5		H500	32	5'-1"	6"	1'-8"	9"															6	12				
	5		L500	652	5'-0"	3"	2'-2"	8"	2'-2"		2.5"												12	1.25				
	5	AA500	10	300'-0"		60'-0"	1'-3"	60'-0"	4																			
	5	XA500	1352	32'-2"		32'-2"																						
	5	XA501	1	33'-7"		33'-7"																						
	5	XA502	1	29'-6"		29'-6"																						
	5	XA504	6	37'-1"		37'-1"																						
5	XB500	76	7'-0"		4'-0"	3'-0"																						
5	XB501	60	5'-0"		2'-0"	3'-0"																						
EPOXY COATED																												
	5	XK500	652	4'-11"	1'-4"	8"	11"	8"	1'-0"	2.5"	8"	8.5	12															
	5	XAA500	64	300'-0"		60'-0"	1'-3"	60'-0"	4																			
	5	XAA501	24	82'-3"		60'-0"	1'-3"	22'-3"	1																			
	5	XAA502	24	74'-3"		60'-0"	1'-3"	14'-3"	1																			
	5	XAA503	24	72'-3"		60'-0"	1'-3"	12'-3"	1																			
	5	XAA504	24	64'-3"		60'-0"	1'-3"	4'-3"	1																			
5	XSA500	1	851'-7"	29'-5"	33'-8"																							
5	XSA501	1	1103'-11"	29'-5"	33'-8"																							
5	XSA502	1	700'-5"	2'-7"	31'-7"																							
5	XSA503	1	703'-10"	2'-9"	31'-7"																							
5	XSA504	1	570'-11"	2'-5"	31'-2"																							
5	XSA505	1	573'-9"	2'-7"	31'-2"																							



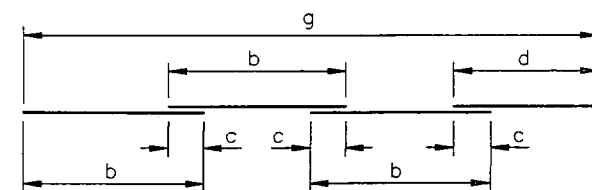
(SA)



BURLINGTON NORTHERN OVERHEAD
RAY

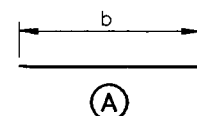
PIER & SUPERSTRUCTURE REINFORCING BAR LIST & DETAILS

2-053.731L-31

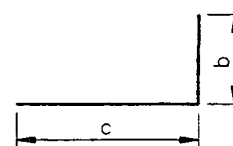


c = LAP SPLICE (TYP.)
e = NO. OF "b" LENGTH PIECES IN A SET
TOTAL LENGTH PER SET = e x b + d

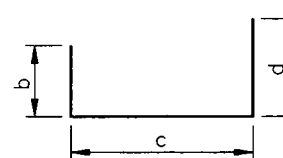
AA



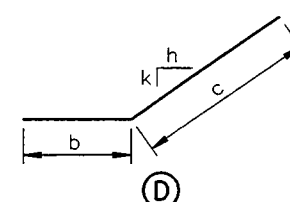
(A)



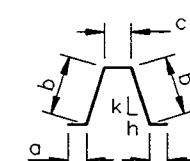
ⓑ



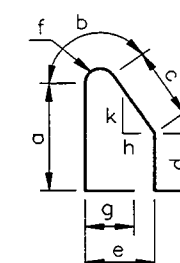
©



Ⓓ

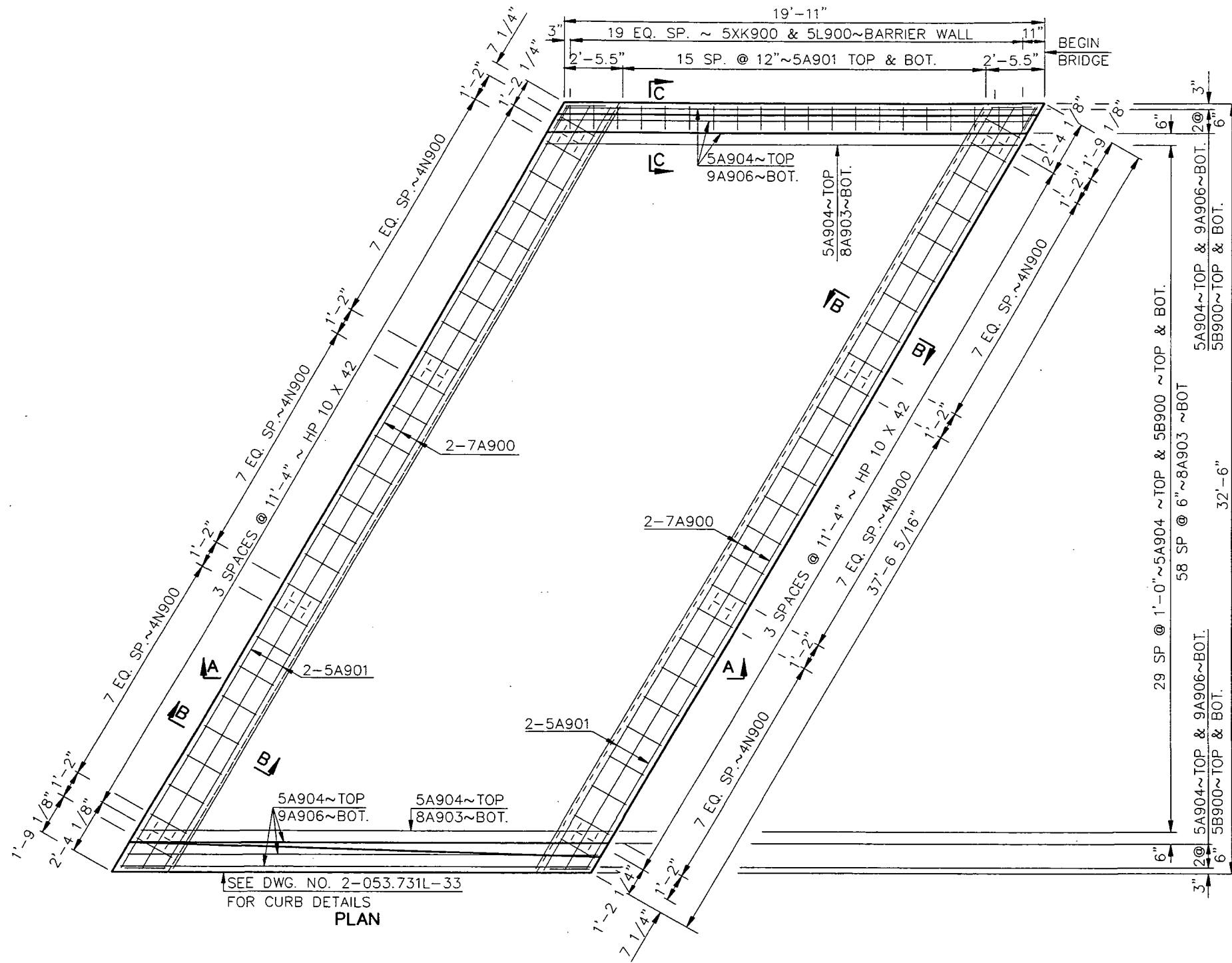


Ⓜ

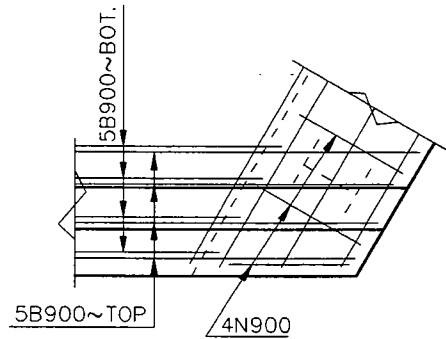
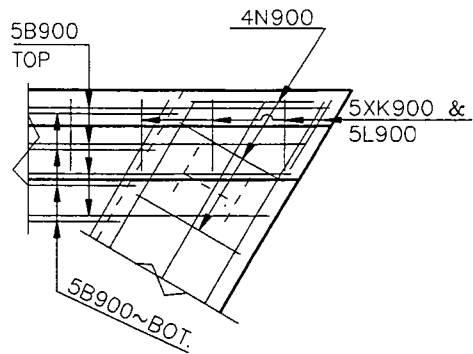
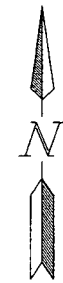
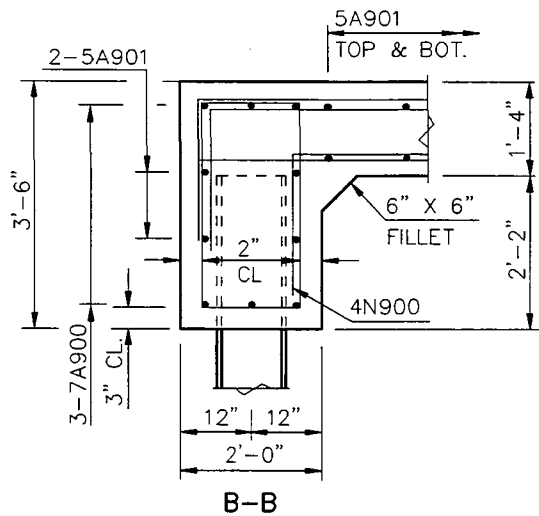
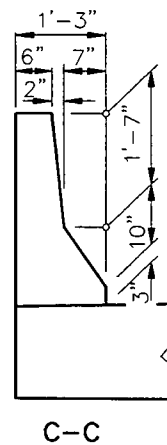
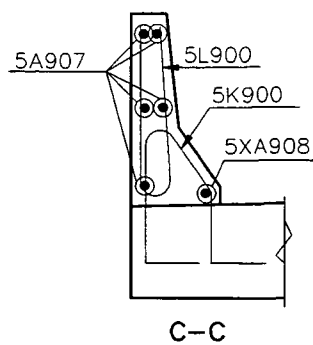
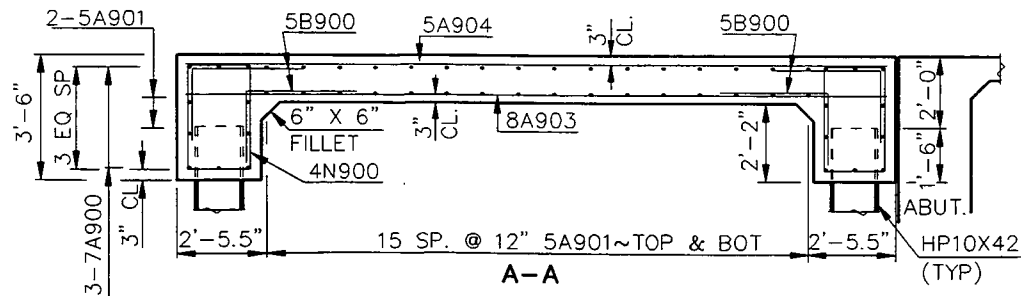


④

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041) 052	120



SEE DWG. NO. 2-053.731L-33
FOR CURB DETAILS
PLAN



QUANTITIES

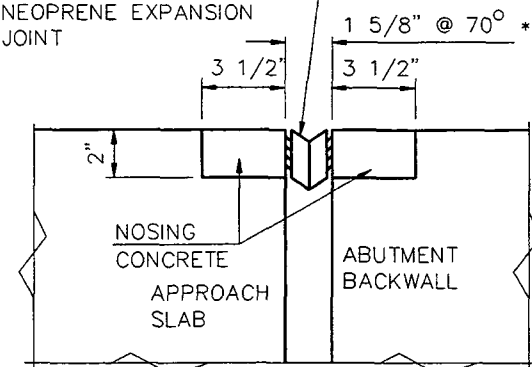
SEE DWG. NO. 2-053.731L-33

BURLINGTON NORTHERN OVERHEAD
RAY

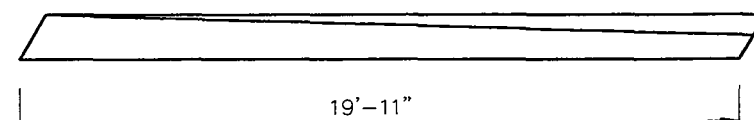
APPROACH SLAB
BEGIN END

* 1/8" CHANGE PER 10°

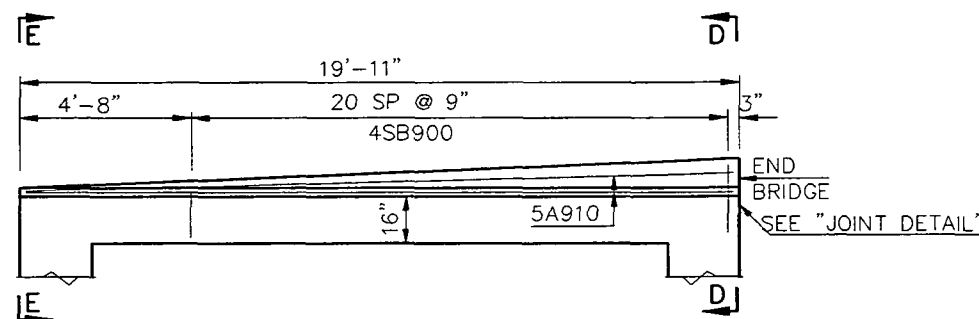
PREFORMED ELASTOMERIC
NEOPRENE EXPANSION
JOINT



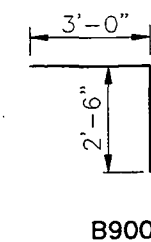
JOINT DETAIL



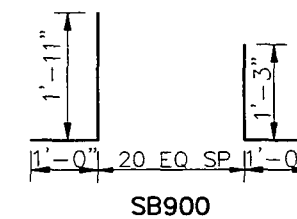
PLAN



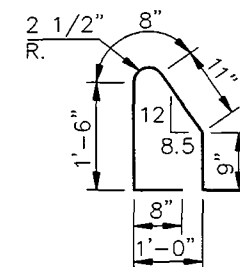
ELEVATION



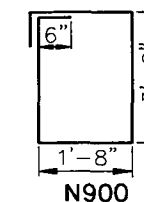
B900



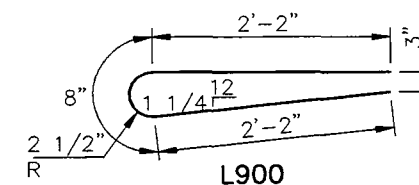
SB900



K900



N900



L900

BENT BAR DETAILS
DIMENSIONS SHOWN ARE OUT TO OUT

WIDTH = 30'-0" CL. RDWY.

SKREW ANGLE = 30°

BAR LIST - ONE SLAB

SIZE	MARK	NO.	LENGTH
7	A900	12	37'-2"
5	A901	40	37'-2"
8	A903	59	19'-7"
5	A904	36	19'-7"
9	A906	6	19'-7"
5	A907	5	19'-7"
5	XA908	1	19'-7"
5	A910	2	19'-7"
5	B900	144	5'-6"
4	SB900	2 SETS	54'-3"
5	XK900	22	5'-2"
5	L900	22	5'-0"
4	N900	56	10'-4"

ESTIMATED MATERIAL QUANTITIES

REINFORCING STEEL LBS.	CONCRETE C.Y.
8,364	46.4

X = EPOXY COATED BAR

NOTES:

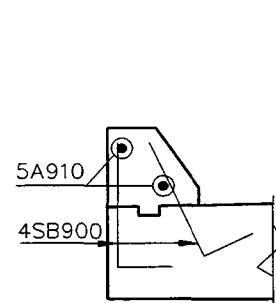
THE ESTIMATED MATERIAL QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. ALL MATERIALS, INCLUDING CONCRETE, REINFORCING STEEL AND LABOR REQUIRED TO BUILD THE APPROACH SLAB SHALL BE INCLUDED IN THE PAY ITEM "CONCRETE BRIDGE APPROACH SLAB". THE PILING, NOSING CONCRETE AND PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT ARE SEPARATE BID ITEMS. THE PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT SHALL CONTINUE UP THE FRONT FACE OF BARRIER FOR A DISTANCE OF 6" AND TERMINATE.

QUANTITIES

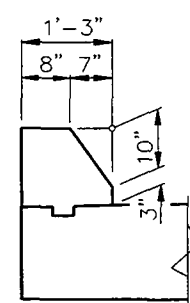
APPROACH SLAB	71.9 SY
NOSING CONCRETE	37.5 LF
PREF. ELAST. NEO. EXP. JT.	35.6 LF

BURLINGTON NORTHERN OVERHEAD
RAY

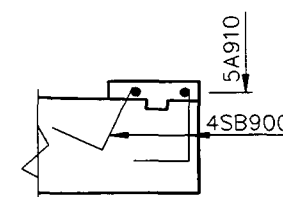
APPROACH SLAB
BEGIN BRIDGE



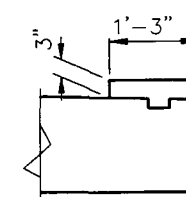
D-D



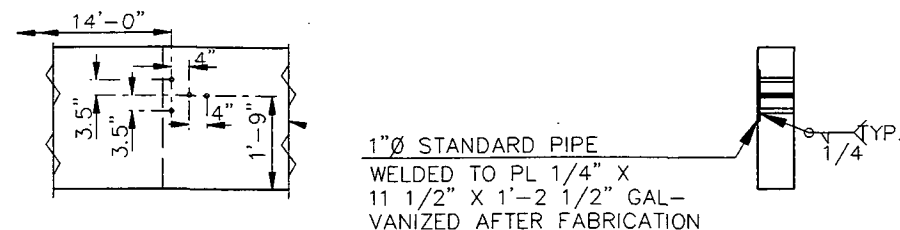
D-D



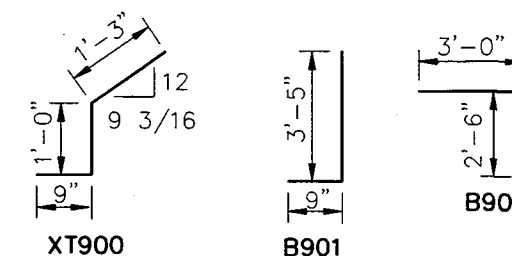
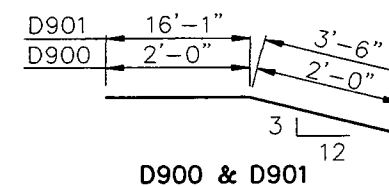
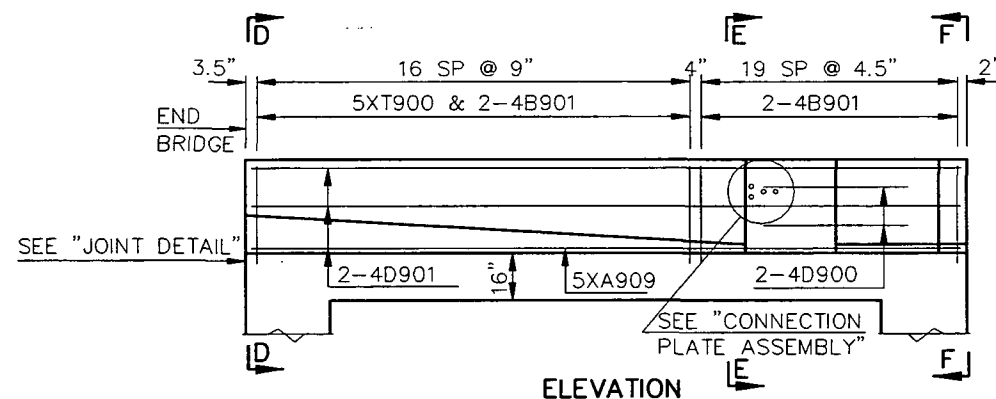
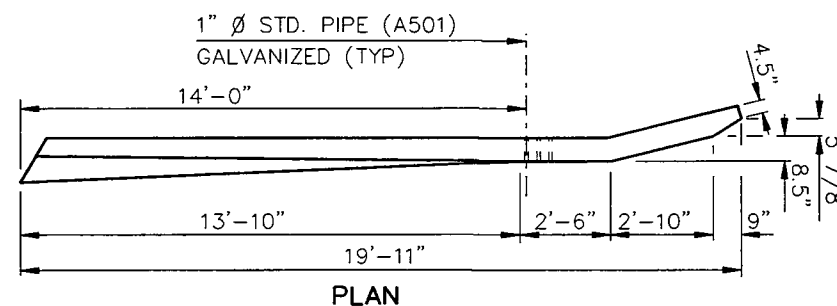
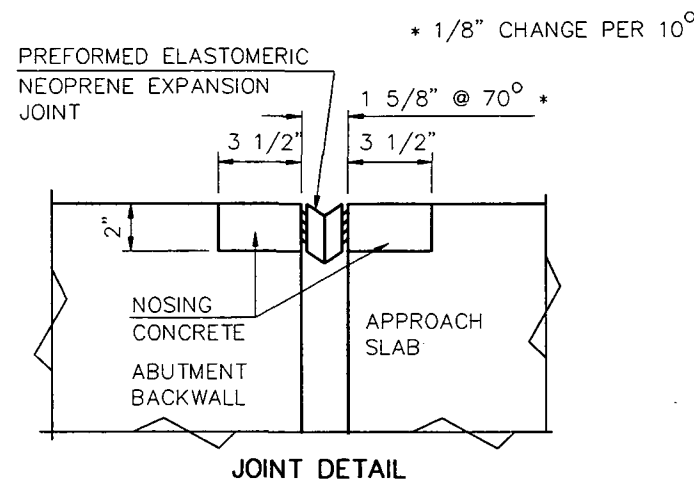
E-E



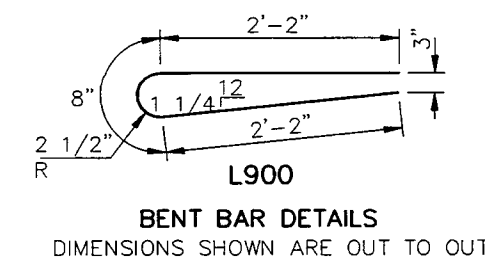
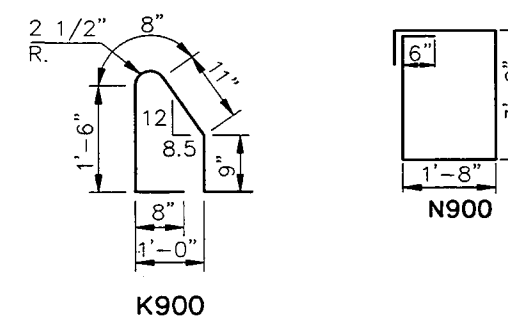
E-E



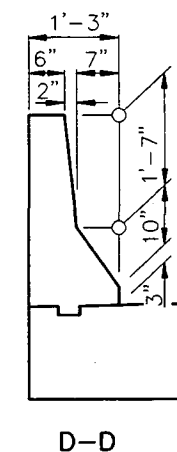
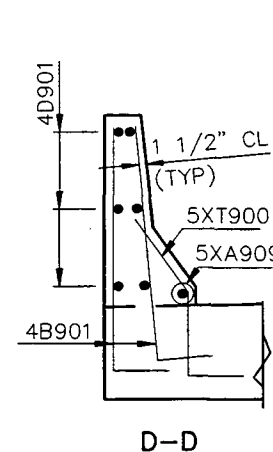
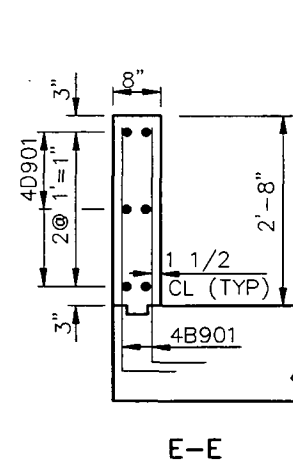
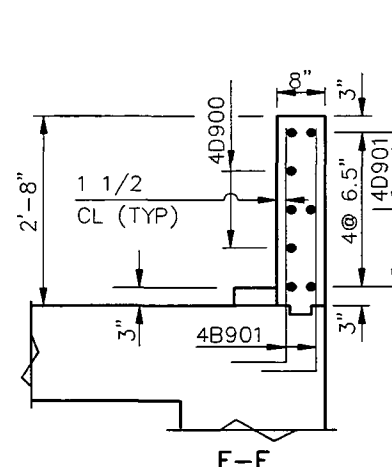
CONNECTION PLATE ASSEMBLY



FIELD BEND
TO FIT



BENT BAR DETAILS
DIMENSIONS SHOWN ARE OUT TO OUT



WIDTH = 30'-0" CL. RDWY.

SKEW ANGLE = 30°

BAR LIST - ONE SLAB

SIZE	MARK	NO.	LENGTH
7	A900	12	37'-2"
5	A901	40	37'-2"
8	A903	59	19'-7"
5	A904	36	19'-7"
5	A905	8	6'-0"
9	A906	6	19'-7"
5	A907	5	19'-7"
5	XA908	1	19'-7"
5	XA909	1	13'-10"
5	B900	144	5'-6"
4	B901	74	4'-2"
4	D900	2	4'-0"
4	D901	6	19'-7"
5	XK900	22	5'-2"
5	L900	22	5'-0"
4	N900	56	10'-4"
5	XT900	17	3'-0"

ESTIMATED MATERIAL QUANTITIES

REINFORCING STEEL LBS.	CONCRETE C.Y.
8,658	47.3

X = EPOXY COATED BAR

NOTES:

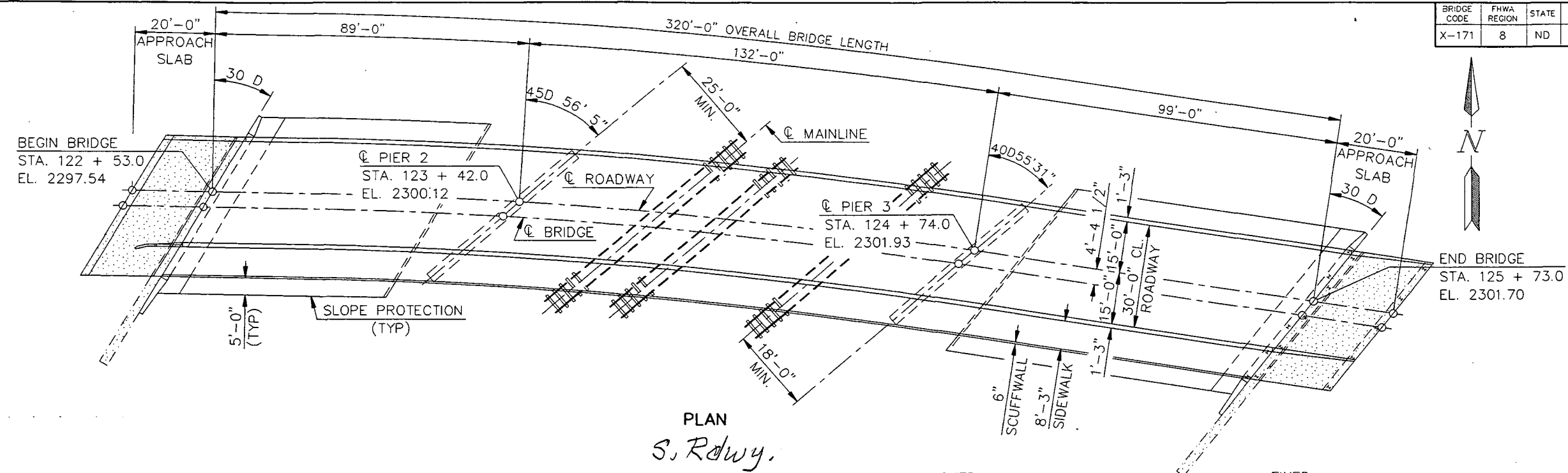
THE ESTIMATED MATERIAL QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. ALL MATERIALS, INCLUDING CONCRETE, REINFORCING STEEL AND LABOR REQUIRED TO BUILD THE APPROACH SLAB SHALL BE INCLUDED IN THE PAY ITEM "CONCRETE BRIDGE APPROACH SLAB". THE PILING, NOSING CONCRETE AND PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT ARE SEPARATE BID ITEMS.

QUANTITIES

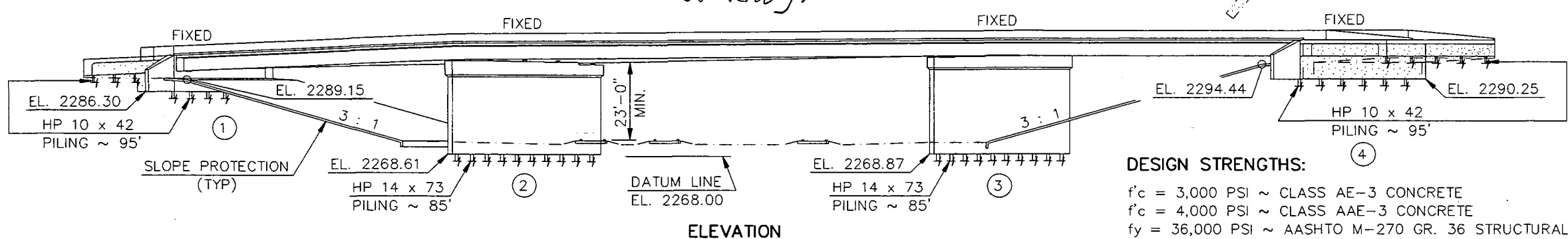
APPROACH SLAB	72.1 SY
NOSING CONCRETE	37.5 LF
PREF. ELAST. NEO. EXP. JT.	35.6 LF

BURLINGTON NORTHERN OVERHEAD
RAY

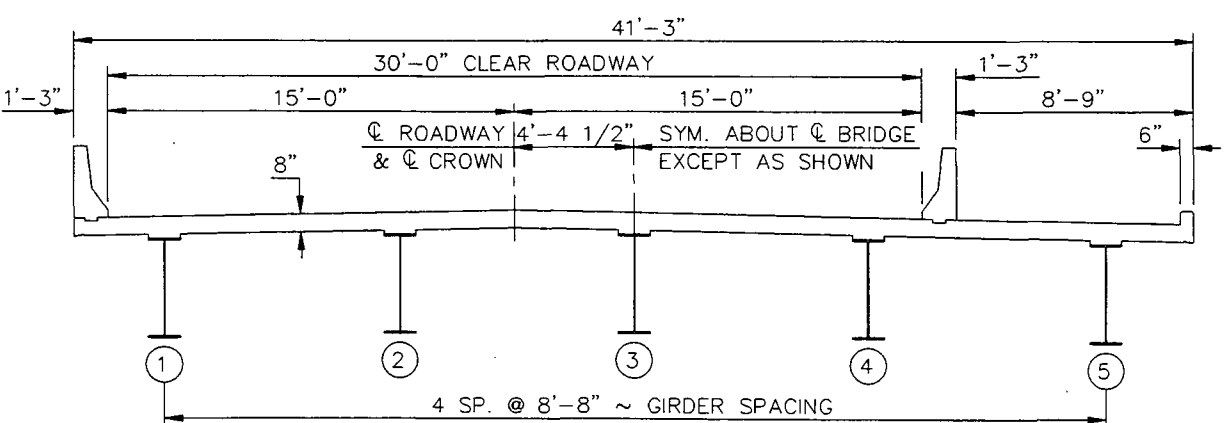
APPROACH SLAB
END BRIDGE



PLAN
S. Rdwy.



ELEVATION



TYPICAL DECK SECTION

BENCH MARKS			
NO.	DESCRIPTION	LOCATION	ELEVATION
BH7	RIVET ON CONCRETE	STA. 123 + 83 - .66' LT.	2270.92
1	TOP OF FIRE HYDRANT	STA. 125 + 53 - 239' RT.	2268.40

DESIGN STRENGTHS:

f'_c = 3,000 PSI ~ CLASS AE-3 CONCRETE
 f'_c = 4,000 PSI ~ CLASS AAE-3 CONCRETE
 f_y = 36,000 PSI ~ AASHTO M-270 GR. 36 STRUCTURAL STEEL
 f_y = 50,000 PSI ~ AASHTO M-270 GR. 50 STRUCTURAL STEEL
 f_y = 60,000 PSI ~ REINFORCING STEEL

LOAD FACTOR DESIGN

SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
107	0100	RAILWAY PROTECTION INSURANCE	L. SUM	.5
202	0105	REMOVAL OF STRUCTURE	L. SUM	.5
210	0103	CLASS 1 EXCAVATION - SITE 2	L. SUM	1
210	0198	SELECT BACKFILL	TON	548
210	0203	FOUNDATION PREPARATION - SITE 2	L. SUM	1
550	0215	CONCRETE BRIDGE APPROACH SLAB	SQ. YD.	182.6
602	0130	CLASS AAE-3 CONCRETE	CU. YD.	414.8
602	1130	CLASS AE-3 CONCRETE	CU. YD.	289.1
602	1250	PENETRATING WATER REPELLENT TR.	SQ. YD.	1066.7
612	0115	REINFORCING STEEL GRADE 60	LBS.	29,549
612	0116	REINFORCING STEEL GRADE 60 EPOXY	LBS.	101,525
616	5889	STRUCTURAL STEEL SITE 2 (APPROXIMATELY 333,018 LBS.)	L. SUM	1
622	0012	STEEL H-PILE TIPS 10x42	EA.	32
622	0016	STEEL H-PILE TIPS 14x73	EA.	20
622	0020	STEEL PILING HP 10x42	L. FT.	3040
622	0060	STEEL PILING HP 14x73	L. FT.	1700
624	0126	PEDESTRIAN CANOPY	L. FT.	316.7
708	1100	SLOPE PROTECTION, CONC.	SQ. YD.	1009
930	3000	BRIDGE BENCH MARKS	SET	.5
930	7014	ROADWAY CANOPY - SITE 2	L. SUM	1
930	8640	NOSING CONCRETE	L. FT.	95.2
930	8650	PREFORMED ELASTOMERIC NEOPRENE EXP. JT.	L. FT.	93.4



STANDARD DRAWINGS

D-900-1, D-708-1, D-622-1

F.W.S. 15 PSF

HS 25 DESIGN LOADING

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION

BURLINGTON NORTHERN OVERHEAD
RAY

BRIDGE LAYOUT

PROJECT: RSN-7-002(041)052
STATION 124 + 13.0

WILLIAMS COUNTY

19 SEP 96
DATE

Larry L. Schwartz
FOR - BRIDGE ENGINEER

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	RSN-7-002(041)052	125

BRIDGE NOTES

- 100 SCOPE OF WORK: This project consists of building a new 3-span steel girder bridge 320' long with clear roadway width of 30' and an 8'-9" sidewalk. Part of the new structure is on a tangent alignment and part is on a curve.

100 GENERAL: The cost of furnishing and placing preformed expansion joint filler, concrete inserts, tie wire, bar spacers, bar supports, and other miscellaneous items shall be included in the price bid for Class AE-3 and AAE-3 concrete.

105 The existing structural steel is painted with lead-based paint. Certain contractor operations could expose employees to hazardous levels of lead. The contractor shall plan accordingly and shall inform employees of the hazards of lead-based paint.

202 REMOVAL OF STRUCTURE: The existing structure is an underpass 107' long with a clear roadway of 30 feet. The substructures are made of concrete, and the superstructure is made of steel. The contractor may have to remove portions of the existing wingwall footings and existing retaining walls to allow for pile driving. The contractor shall remove a minimum amount of material from the wingwall footings. The contractor shall place compacted fill in the existing underpass up to an elevation 6' below the bottom of the steel beams. The contractor shall entirely fill the pedestrian tunnels and the voids under the sidewalks of the existing structure with either earth or flowable fill. Burlington Northern (BN) Railroad will remove the superstructure and portions of the substructure. BN Railroad will place the removed material on the railroad right of way at a location agreed to by BN Railroad and the engineer. All materials removed by the contractor and BN Railroad shall become the property of the contractor and shall be disposed of properly off the right of way. All labor and materials required to perform the work described above shall be included in the bid item "Removal of Structure."

203 EMBANKMENT: The embankment at the abutments shall be in place for a minimum of 30 days before piling are driven.

The contractor will be required to bore through the fill at the abutments before driving piling. All pilot holes not completely filled by the piles shall be backfilled with sand or fine gravel before the substructure is placed.

In lieu of the above waiting period, the contractor may begin pile driving operations as soon as the embankment is in place, but all pilot holes shall be extended to a depth of 15 feet into the original ground.
- 210 EXCAVATION: The excavation at the abutments, as shown, and the excavation required to build the piers shall be included in the lump sum bid item, "Class 1 Excavation, Site 2."

210 SELECT BACKFILL: Select backfill shall meet the requirements of Section 816.03, Class 3. The backfill shall be placed in layers of not more than 6 inches, moistened or dried as required, and thoroughly compacted with mechanical tamping equipment.

550 BRIDGE APPROACH SLABS: Mechanical finishing of the approach slabs shall be required. A mechanical or hand-held transverse metal tine finish shall be applied. Tining shall start 6" from the beginning and end of the approach slabs. A surface tolerance of 3/16" in 10 feet is also required.

602 Exterior beams shall be braced to prevent rotation. The strength and spacing of the bracing shall be dependent on the forces induced by the weight of the concrete, forms, equipment, and workers.

602 END BEAMS: The end beam concrete shall be placed before the deck concrete. The concrete shall cure for at least 72 hours before deck placement.

602 SURFACE FINISH "D": Surface Finish "D" shall be required for the inside, outside, and top surfaces of the barrier, and on the outside edge of the deck.

602 DECK CONCRETE: Beams and girders have slight variations in the anticipated camber. To build the deck to the designated thickness will require slight adjustments in deck elevation and/or riser dimensions. These adjustments result in minor concrete quantity discrepancies. The contractor shall consider this quantity discrepancy when he bids the unit price for Class AAE-3 Concrete. The Department will pay plan quantity of Class AAE-3 Concrete.

602 Deflection of the deck shoring shall be computed using the total dead load plus the weight of the finishing machine. The forming shall be adjusted properly to accommodate the deflection and thereby maintain the total slab thickness specified in the plans.

602 PENETRATING WATER REPELLENT TREATMENT: Penetrating water repellent shall be applied to the driving surface of the concrete deck.

602 BARRIERS: Barriers shall be constructed according to the provisions of Section 602.03 B.4 except that there shall be no expansion or deflection joints. Make 3/4" V-grooves in all faces of the barriers at each pier and at equal spaces

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
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BRIDGE NOTES

- between substructures at approximately 10-foot spacing.
- 602 DECK TINING: Tining shall begin 6 inches from the beginning and the end of the deck and 6 inches from each deck joint.
- 602 SIDEWALK BROOMING: The sidewalk on the bridge shall not be tined. The sidewalk shall be transversely broomed to slightly roughen the surface.
- 612 REINFORCING STEEL: Dimensions for bent bars are given out to out and to tangent intersections unless otherwise noted.
- 612 The bar fabricator shall add a prefix to all bar designations to differentiate between the different structures on this project.
- 612 All reinforcing steel shall be Grade 60.
- 616 STRUCTURAL STEEL: Structural steel shall be AASHTO M 270, Grade 50T2, except the diaphragms and swedge bolts may be grade 36T2. The requirement for Charpy V-Notch is waived for the bearings, diaphragms, and diaphragm connection material.
- 616 Shear connectors on splice plates shall be moved to clear bolt holes.
- 616 Field connections shall be made with 7/8 inch diameter, AASHTO M 164 high-strength bolts unless otherwise shown.
- 616 Temporary or permanent attachments or devices that are not shown on the plans as part of the structure shall not be welded to the structural steel members during the fabrication and construction process.
- 616 The cost of swedge bolts shall be included in the total cost of structural steel.
- 622 PILING: Piling shall be driven with a steam, air, or diesel hammer with a rated energy and ram weight not less than 55,154 foot-pound-tons, as computed by the formula $W(E-13,444)+.821E$, where W is the weight of the ram in tons and E is the rated hammer energy. In no case shall the ram weight be less than 2700 pounds.
- 622 PILING: Toothed cast steel pile tips (ASTM A148 steel) shall be required on all pile driven.
- 630 PAINT AND PAINTING: The structural steel shall be painted according to the specifications. The finish coats shall be blue, color number 25240, and shall meet Federal Standard No. 595B colors.

- 708 SLOPE PROTECTION: The concrete slope protection will be limited to the cast-in-place type shown on Standard D-708-1.
- SHOP DRAWINGS: CAD-generated shop drawings may be submitted on 11-inch by 17-inch detail sheets. The contractor shall submit the following shop drawings to the Construction office for approval:
1. Structural Steel.
 2. Preformed Elastomeric Neoprene Expansion Joint.
- DESIGN STRENGTH: F'C 3,000 PSI Cl. AE-3 Concrete
F'C 4,000 PSI Cl. AAE-3 Concrete
FY 36,000 PSI Structural Steel M270 Grade 36T2
FY 50,000 PSI Structural Steel M270 Grade 50T2
FY 60,000 PSI Grade 60 Reinforcing Steel Load Factor Design
- 930 PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT: The neoprene joint shall have a profile that allows thermal cycles and dynamic loading up to a total range of movement of two inches while providing watertight protection. The neoprene profile shall be bonded to the concrete surfaces with an epoxy adhesive that is chemically resistant to oxidation and corrosion, is impervious to weather extremes and is compatible with the nosing concrete. During installation, the neoprene profile shall be pressurized with compressed air to maximize the adhesion against the gap walls. After curing, the air pressure shall be permanently released. This shall be a Jeene Structural Sealing Joint System (2 W) or an approved equal.
- 930 NOSING CONCRETE: The nosing concrete material shall be an elastomeric concrete or a polymeric concrete that will provide a durable edge that can withstand live-load traffic without chipping or spalling. The nosing concrete material shall be Ceva Crete 95, manufactured by E-Poxy Industries Inc.; Elastomeric Concrete, as manufactured by the D.S. Brown Company; Silspec 900, as manufactured by Construction Materials Inc.; Elastomeric Concrete as manufactured by Harris Speciality Chemicals Inc., or an approved equal. The nosing shall be mixed and installed according to the manufacturer's recommendations. The cost for the equipment, materials, and labor to install the nosing concrete on each side of the joint shall be included in the lineal foot bid item "Nosing Concrete."
- 930 ROADWAY CANOPY: The contractor shall construct a canopy above the railroad under the structure to protect traffic from falling material. The canopy is an added safeguard and does not relieve the contractor of any responsibility

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	RSN-7-002 (041) 052	127

BRIDGE NOTES

for the safety of the public.

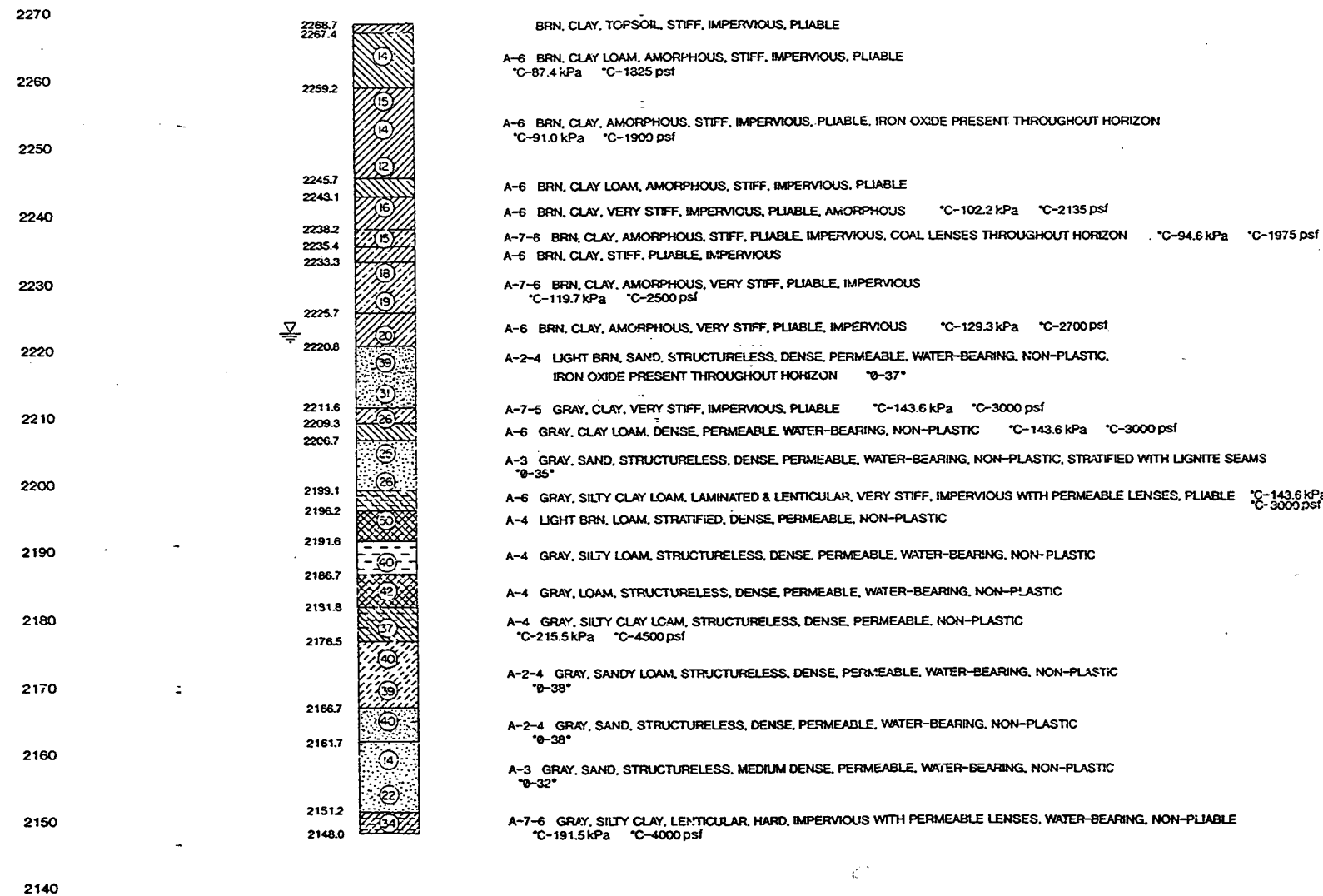
The canopy may be supported from the ground or suspended from the girders. The erection of the canopy and the girder shall be completed in a minimum amount of time and with the least inconvenience to the public and the railroad.

The canopy shall be of a design and material selected by the contractor and approved by the engineer. The minimum vertical clearance shall be 22' above the railroad tracks. The canopy shall project a minimum distance of 5'-0" beyond the outside edge of curb of the proposed structure.

The canopy shall project a minimum distance of 5'-0" beyond the edge of the driving lanes beneath the structure and 5'-0" beyond the edge of the railroad tracks.

After completion of the structure, the canopy shall be removed and shall remain the property of the contractor.

The roadway canopy shall be paid for at the contract lump sum unit price for "Roadway Canopy." The roadway canopy shall be measured as a lump sum item and shall include construction, maintenance, and removal.

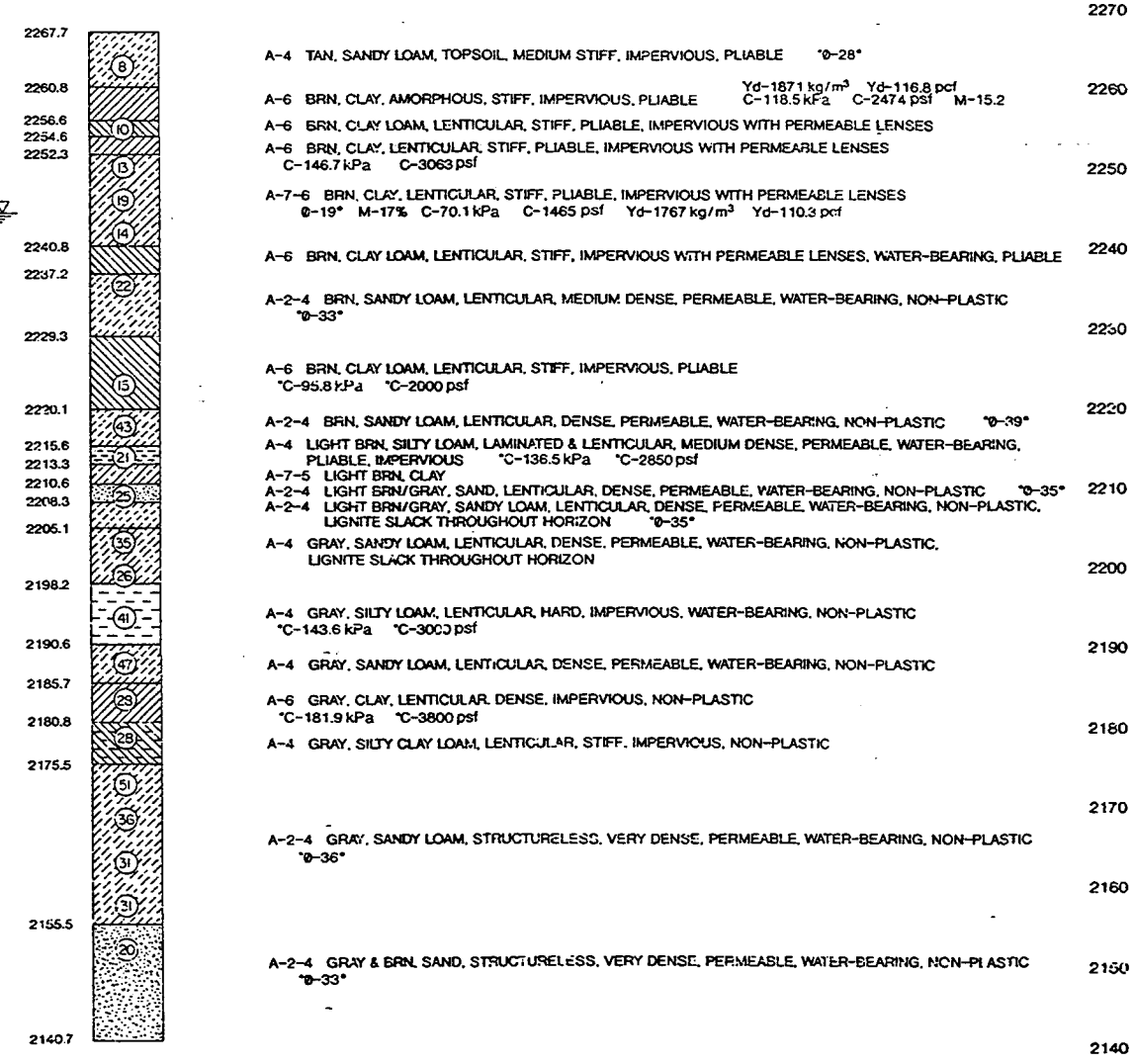


BORING NO. 1

STA. 124+51.4 - RT. 66.6' FROM EXISTING CENTERLINE

NOTES: Encircled numbers indicate the number of blows delivered by a 140 lb. hammer from a height of 30" to drive a core tube 1"-C".

The boring log data shown is for design purposes only. The state assumes no responsibility if soil conditions encountered during construction differ from those shown.



BORING NO. 2

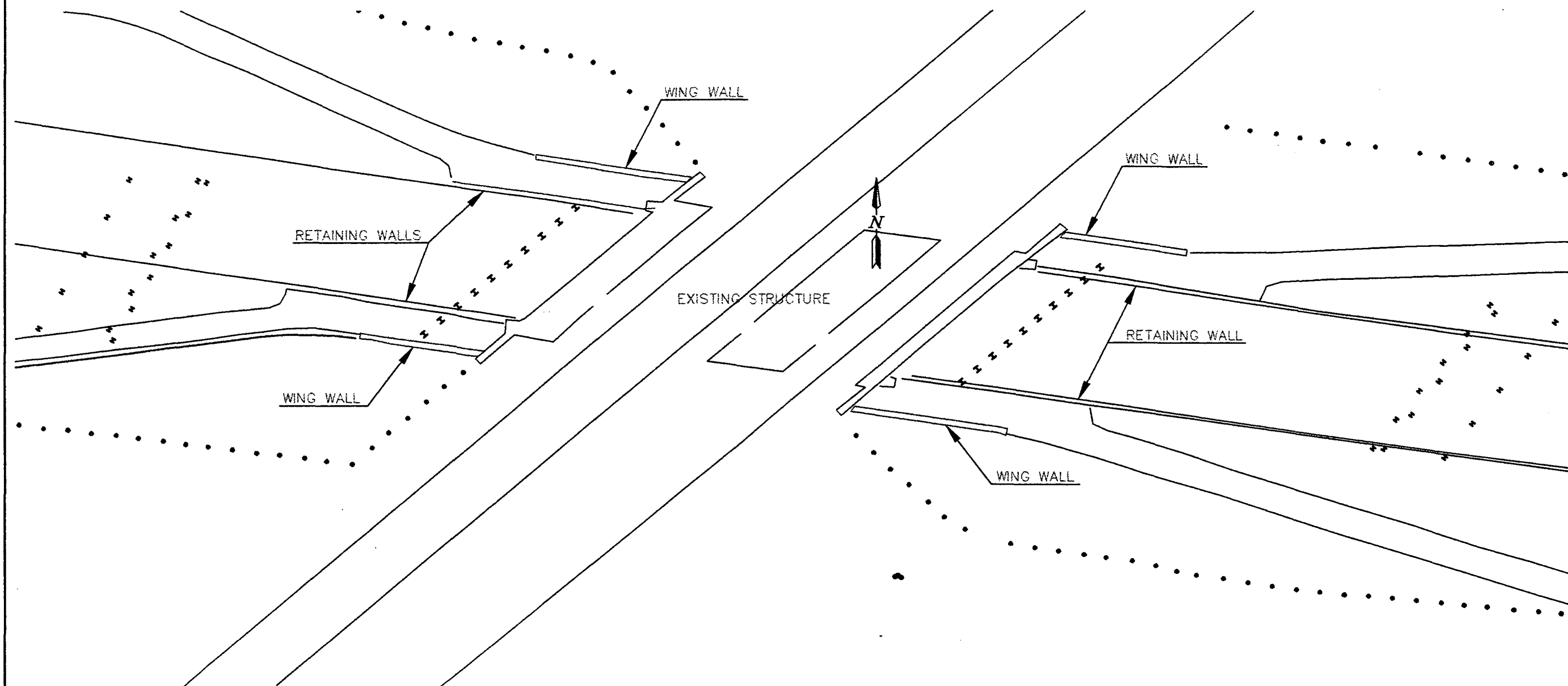
STA. 126+81.1 - LT. 51.2' FROM EXISTING CENTERLINE

SYMBOLS: θ- Angle of Internal Friction (Degrees)
C- Cohesion (Lbs./Sq. Ft.) (kPa)
M- Moisture (%)
Yd- Dry Weight (Lbs./Cu.Ft.) (kg/Cu.m)
*- (These cohesive values & friction angles are estimated from blow count.)

WILLIAMS COUNTY
RAY OVERPASS ON HWY 2

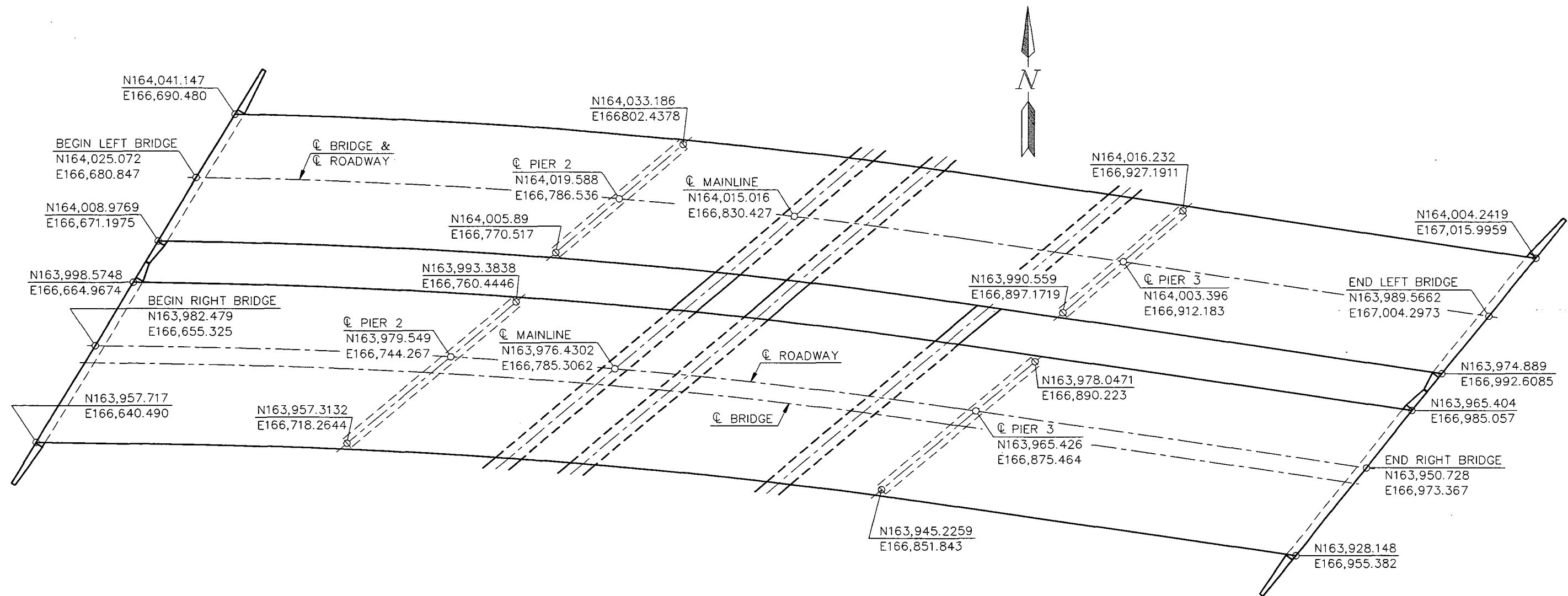
BORING LOG

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	129



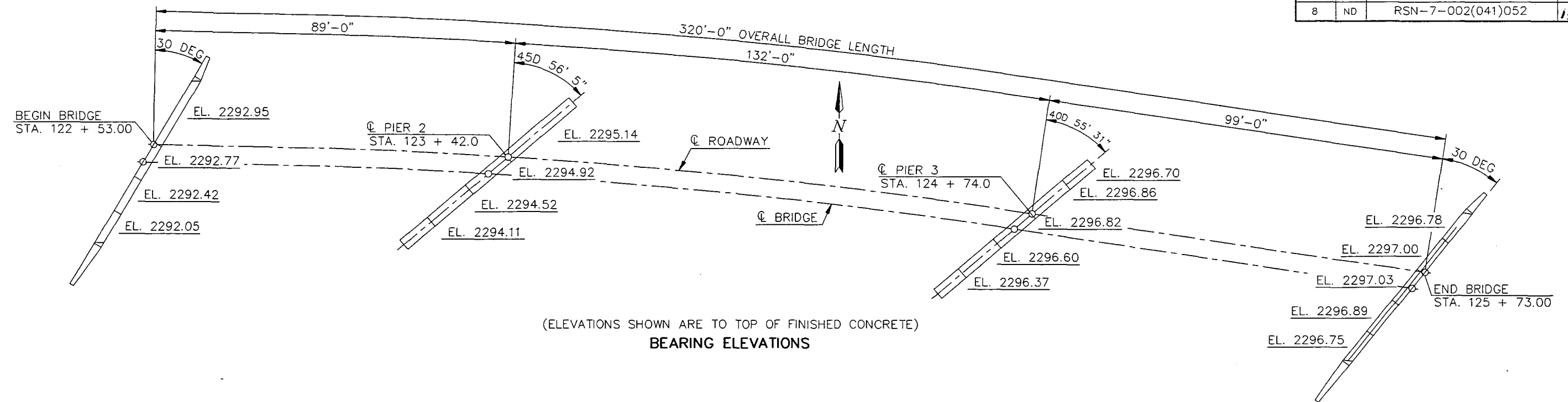
BURLINGTON NORTHERN OVERHEAD
RAY

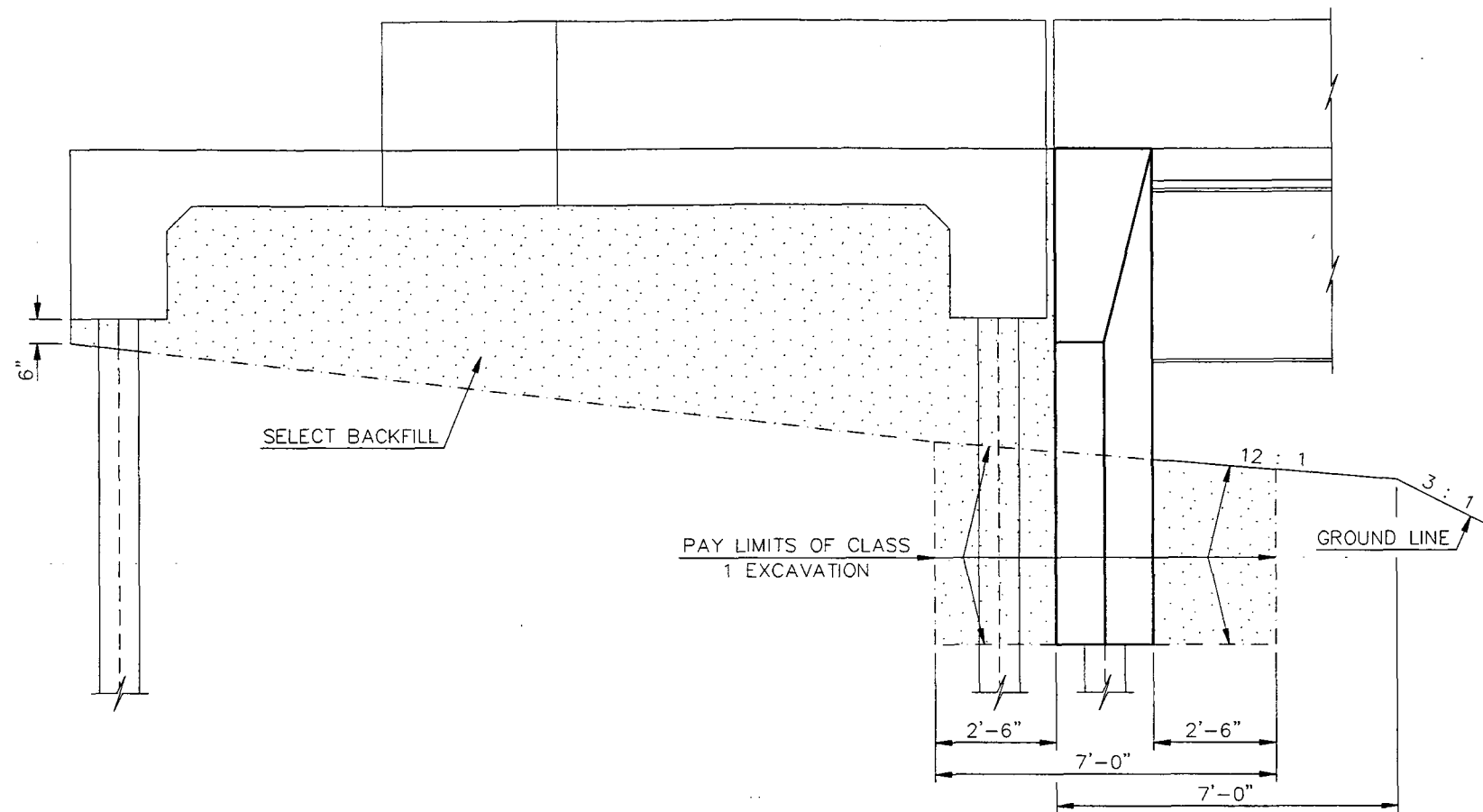
EXISTING STRUCTURE AND
NEW PILE LOCATIONS



BURLINGTON NORTHERN OVERHEAD
RAY

GENERAL PROJECT LAYOUT

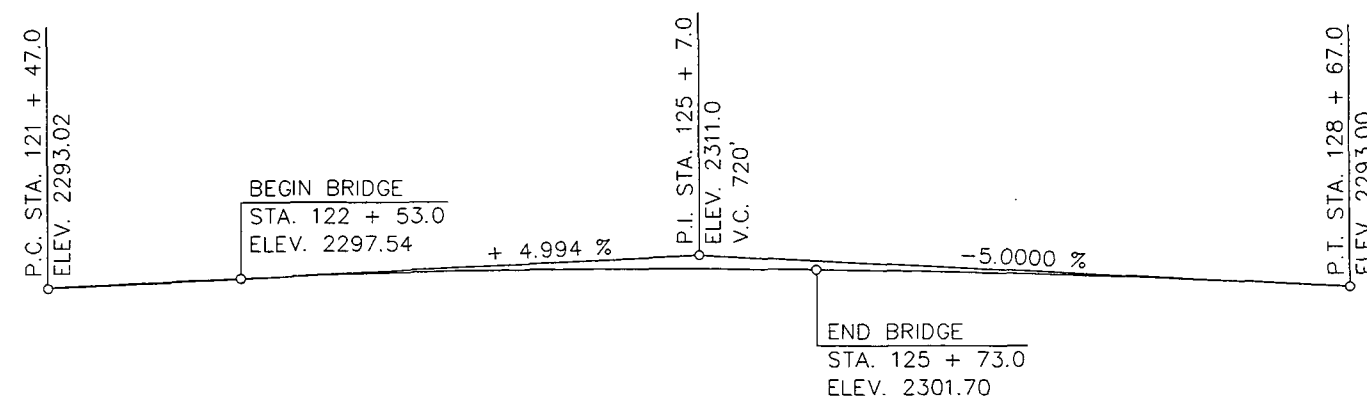




DETAIL AT ABUTMENT

HORIZONTAL CURVE DATA

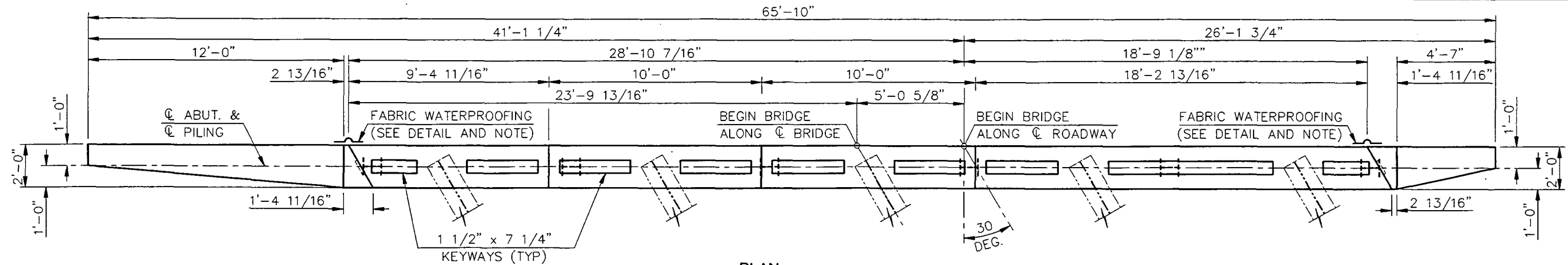
P.I. STA. 123 + 72.5
 DELTA = 7D 36' 26.49" RT
 DEGREE = 3D 58' 8.52"
 LENGTH = 191.6677'
 TANGENT = 95.9749
 RADIUS = 1443.57'
 P.C. STA 122 + 76.53
 P.T. STA. 124 + 68.2



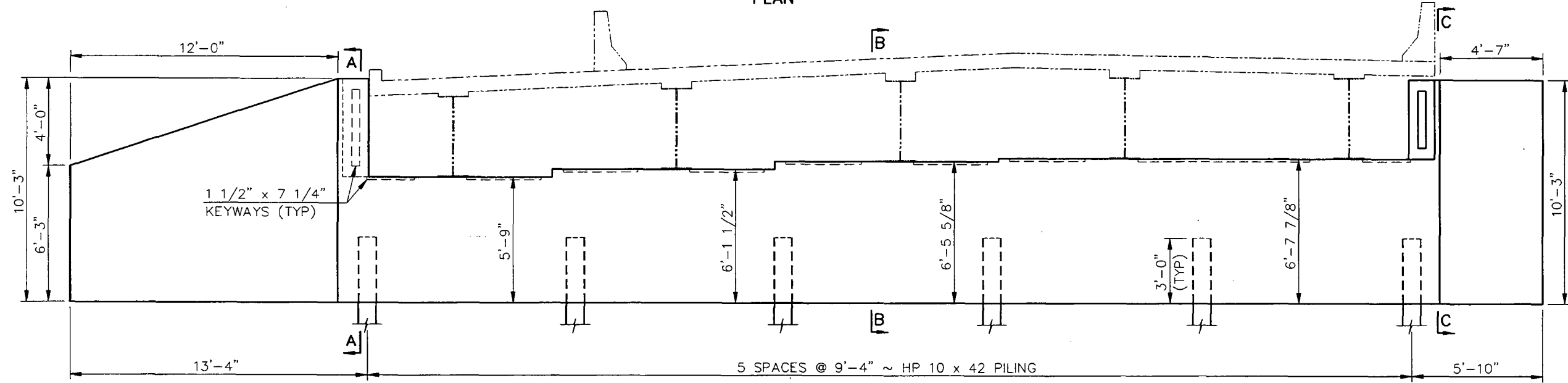
VERTICAL CURVE DATA

BURLINGTON NORTHERN OVERHEAD
 RAY

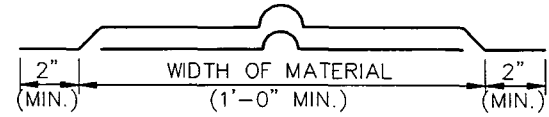
DETAIL AT ABUTMENT
 & VERTICAL CURVE DATA

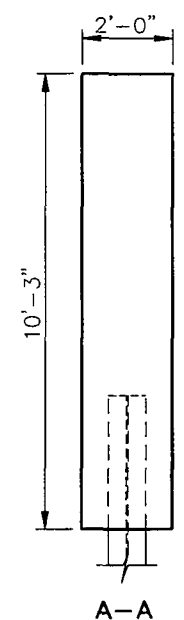


PLAN

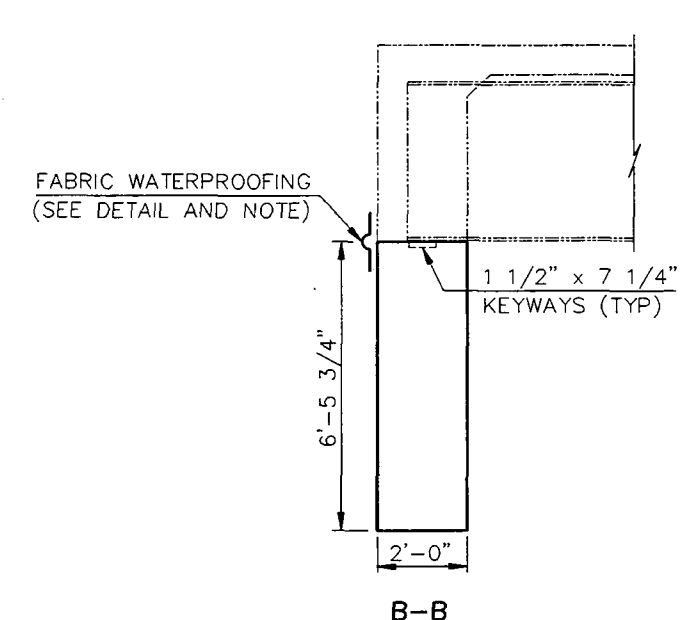


ELEVATION

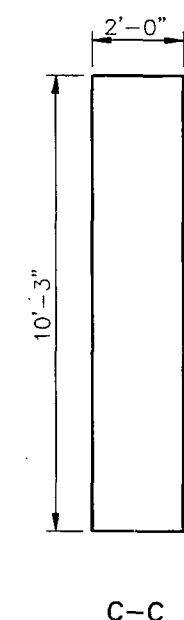

 FABRIC WATERPROOFING SHALL BE APPLIED IN ACCORDANCE WITH SECTION 740 OF THE NDDOT SPECIFICATIONS.
 ALL MATERIAL AND WORK SHALL BE CONSIDERED INCIDENTAL TO THE PAY ITEM OF CLASS AE-3 CONCRETE.
TWO-PLY FABRIC WATERPROOFING DETAIL



A-A



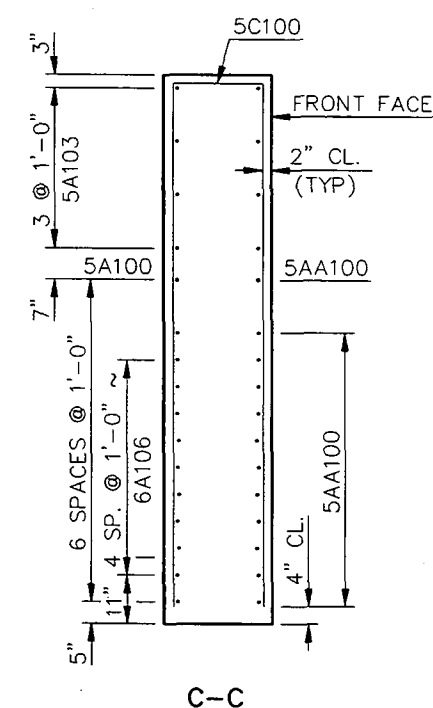
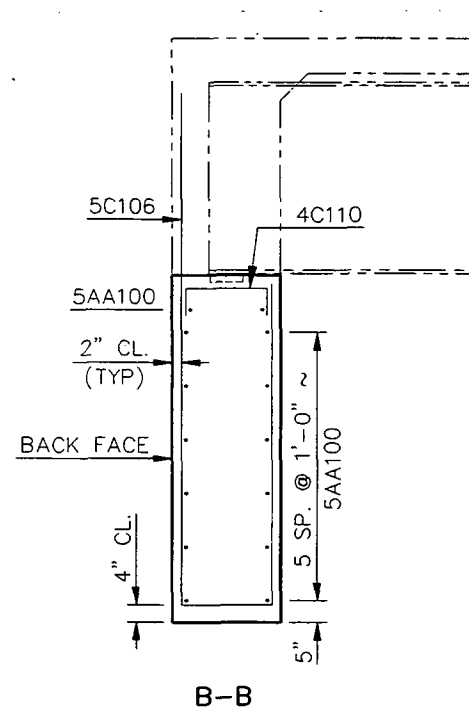
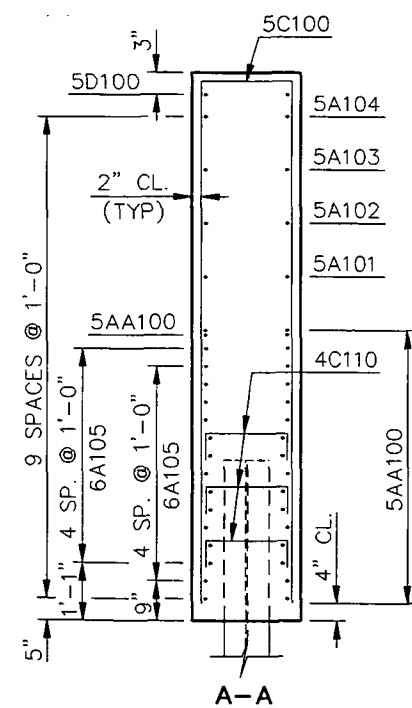
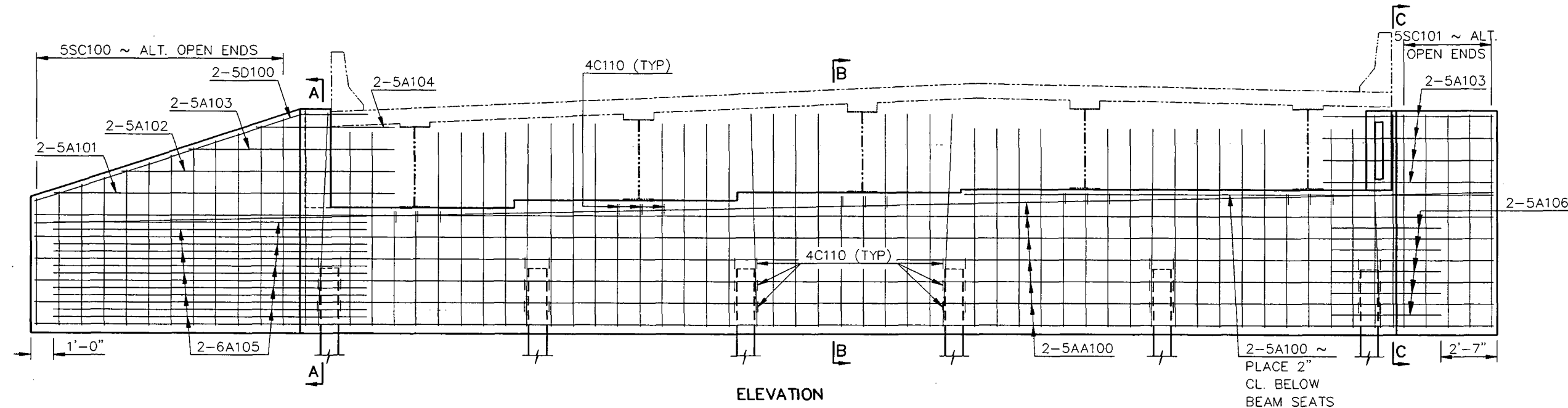
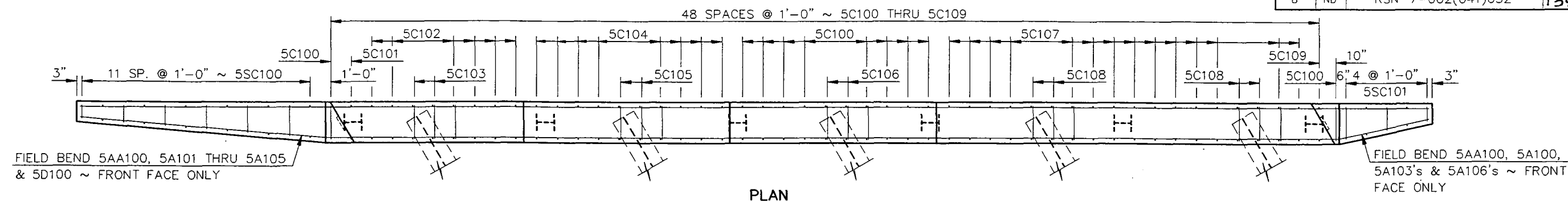
B-B



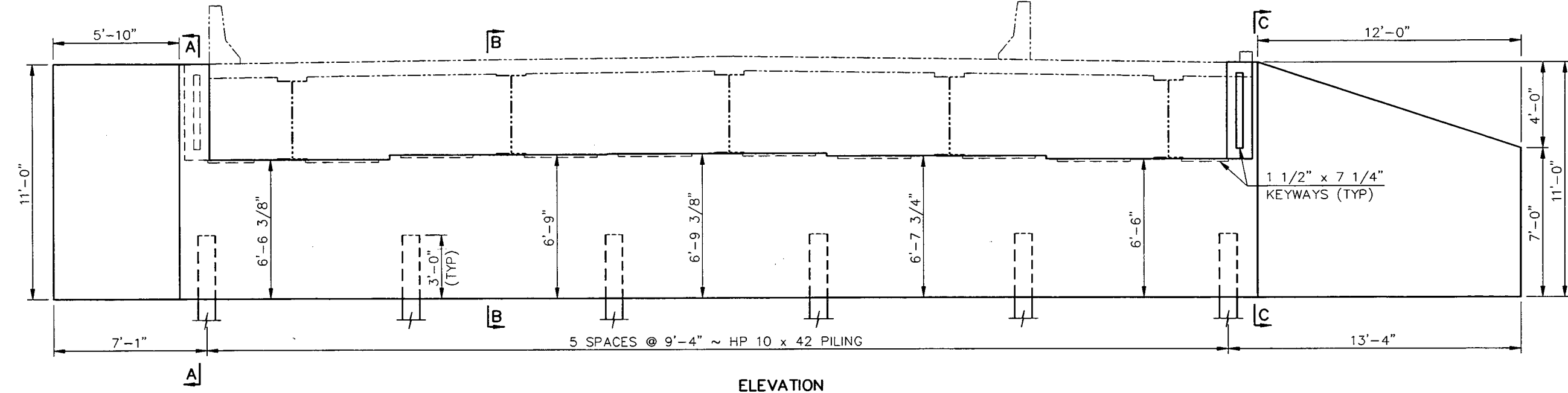
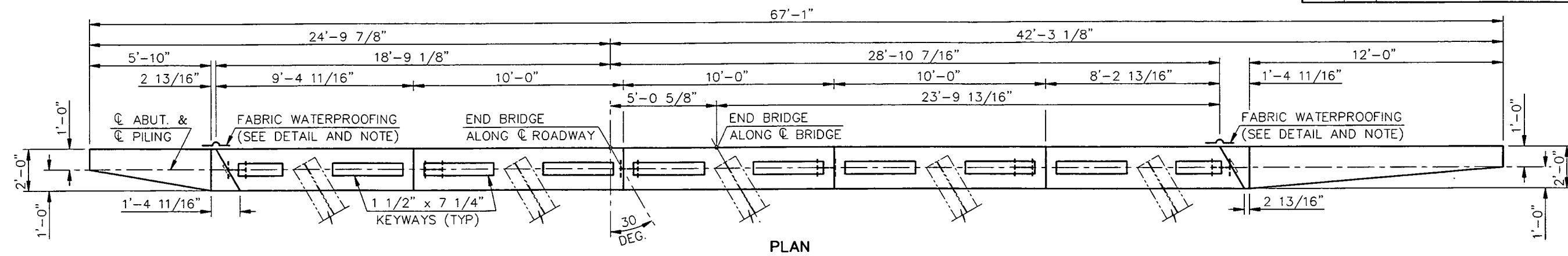
C-C

NOTE:
 THE ELEVATION OF THE TOP OF THE NORTH WING WALL SHALL MATCH THAT OF THE SOUTH WING WALL OF THE ADJACENT ABUTMENT.

QUANTITIES
SEE DWG. 2-053.731R-10
BURLINGTON NORTHERN OVERHEAD RAY
(SHOWING DIMENSIONS)
ABUTMENT 1 DETAILS



QUANTITIES	
CLASS AE-3 CONCRETE	31.7 CY
REINFORCING STEEL	3098 LB
BURLINGTON NORTHERN OVERHEAD RAY	
(SHOWING REINFORCING)	
ABUTMENT 1 DETAILS	

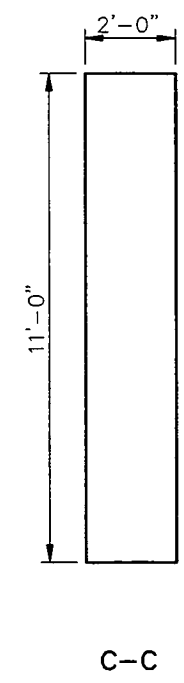
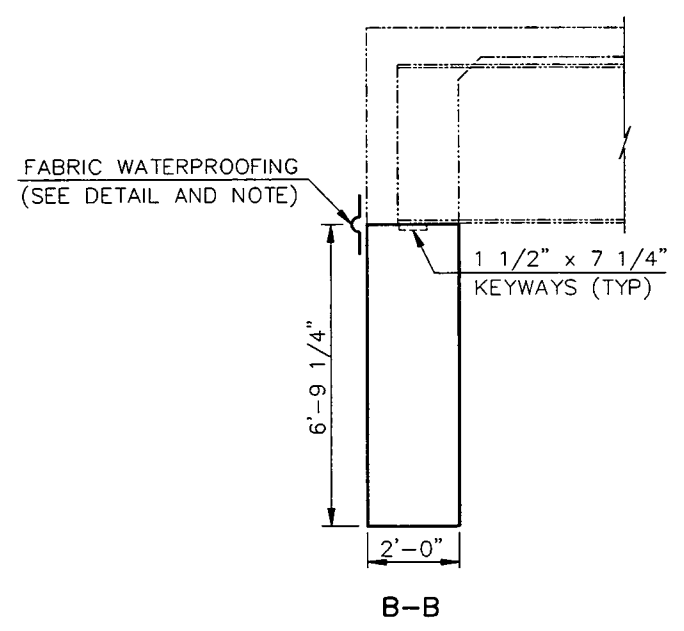
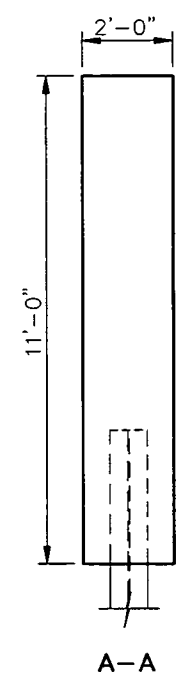


2" (MIN.) WIDTH OF MATERIAL (1'-0" MIN.) 2" (MIN.)

FABRIC WATERPROOFING SHALL BE APPLIED IN ACCORDANCE WITH SECTION 740 OF THE NDDOT SPECIFICATIONS.

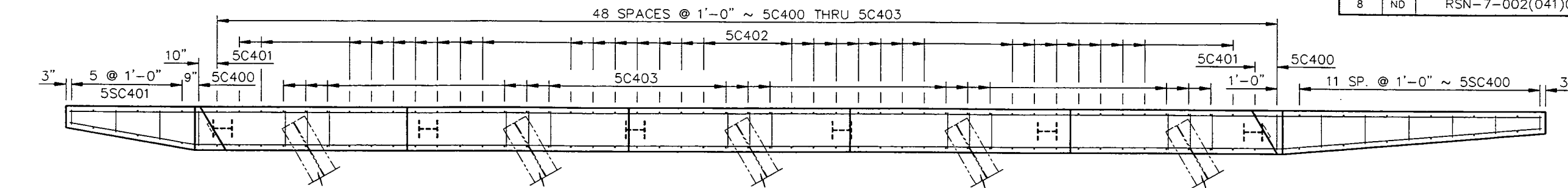
ALL MATERIAL AND WORK SHALL BE CONSIDERED INCIDENTAL TO THE PAY ITEM OF CLASS AE-3 CONCRETE.

TWO-PLY FABRIC WATERPROOFING DETAIL

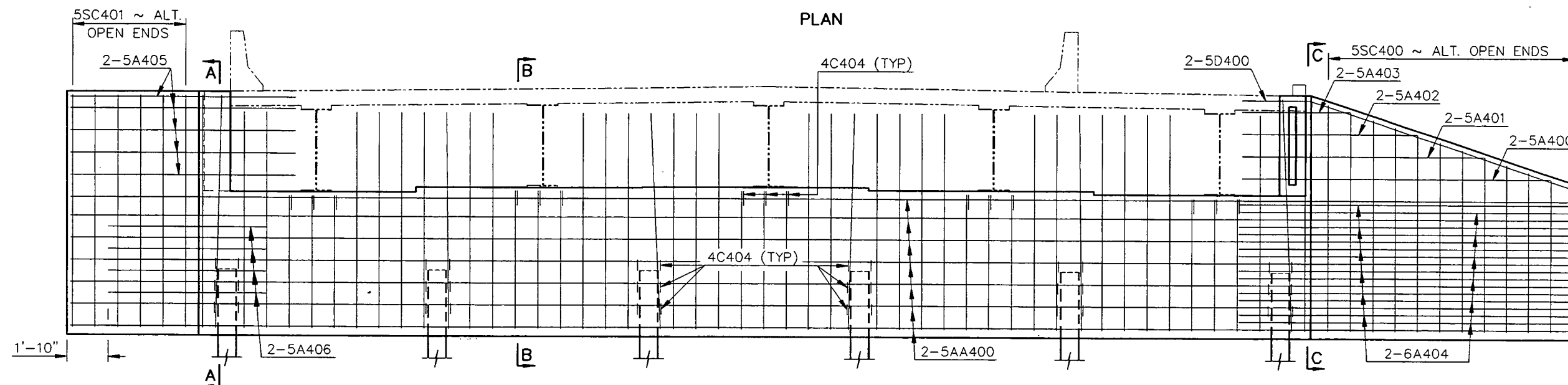


NOTE:
THE ELEVATION OF THE TOP OF THE NORTH WING WALL SHALL MATCH THAT OF THE SOUTH WING WALL OF THE ADJACENT ABUTMENT.

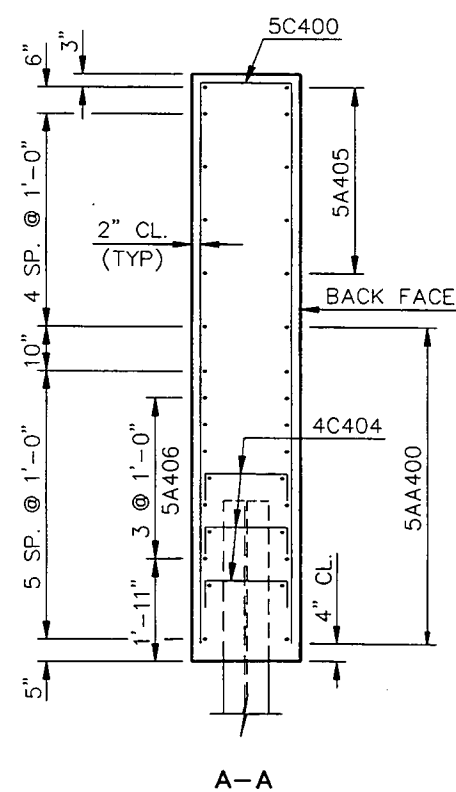
QUANTITIES
SEE DWG. 2-053.731R-12
BURLINGTON NORTHERN OVERHEAD RAY
(SHOWING DIMENSIONS)
ABUTMENT 4 DETAILS



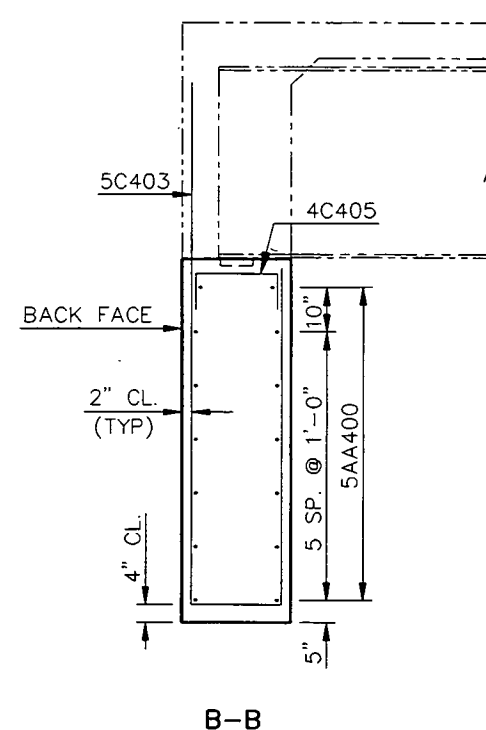
PLAN



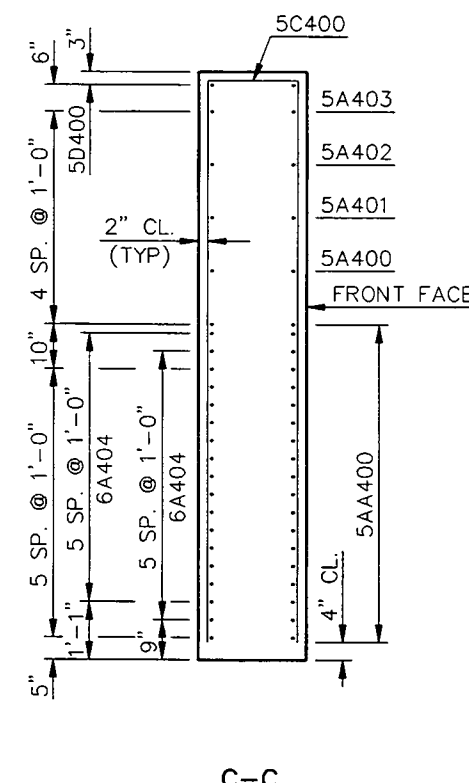
ELEVATION



A-A

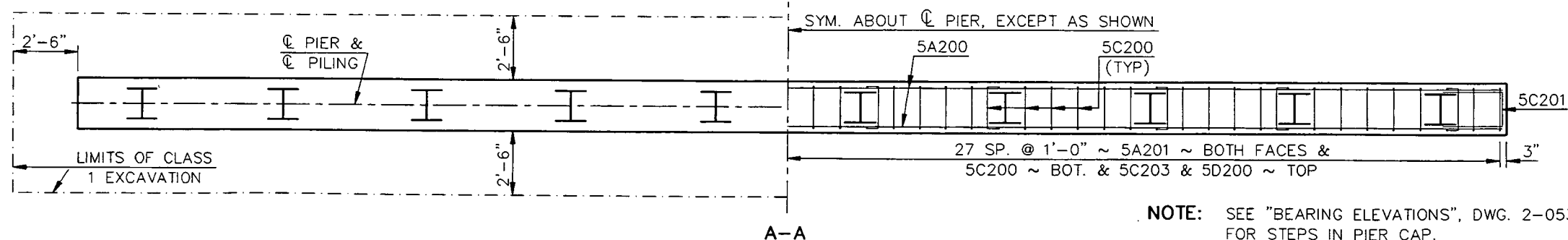
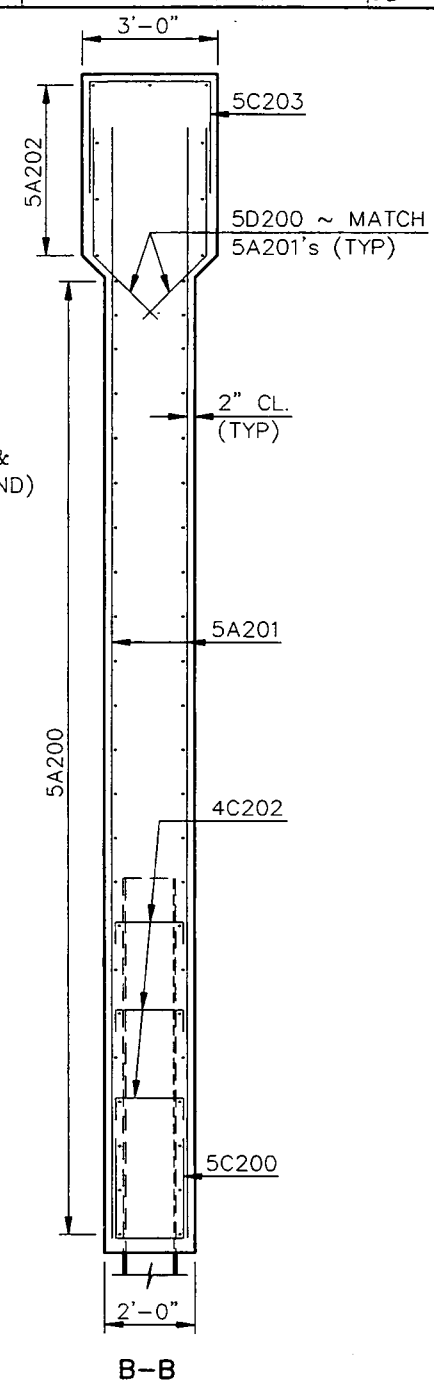
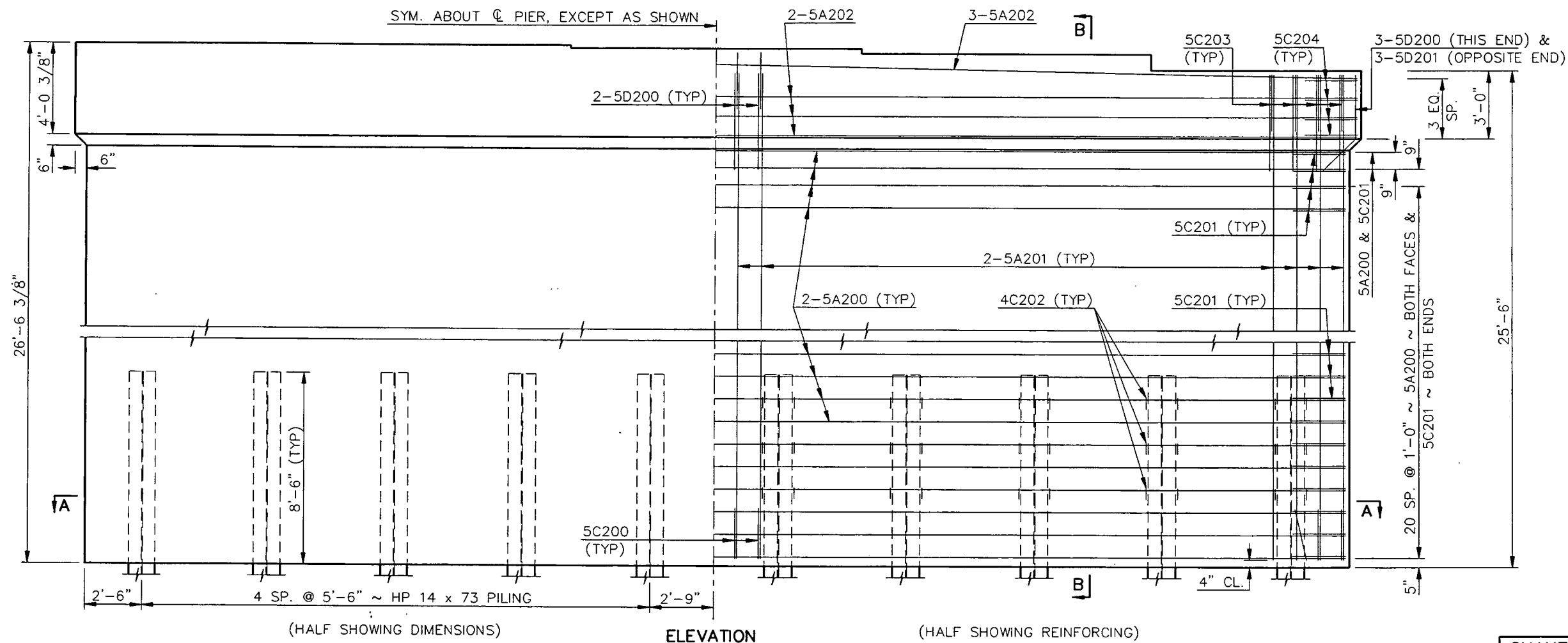
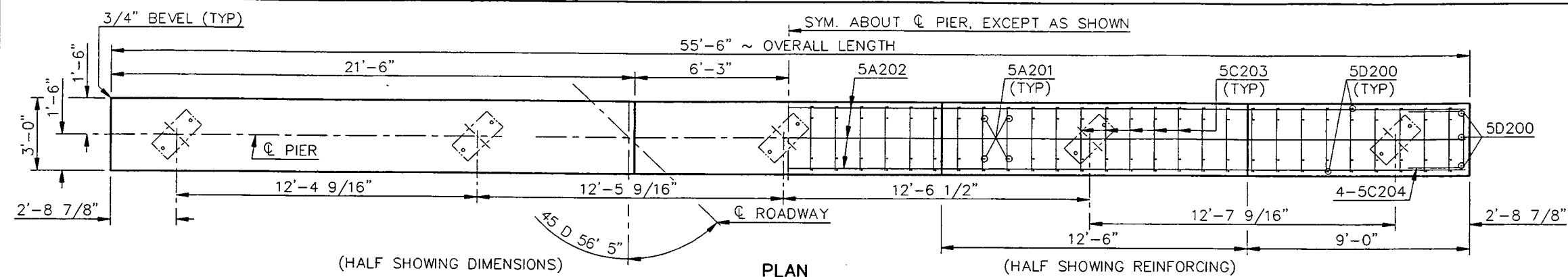


B-B



C-C

QUANTITIES	
CLASS AE-3 CONCRETE	34.4 CY
REINFORCING STEEL	3308 LB
BURLINGTON NORTHERN OVERHEAD RAY	
(SHOWING REINFORCING)	
ABUTMENT 4 DETAILS	



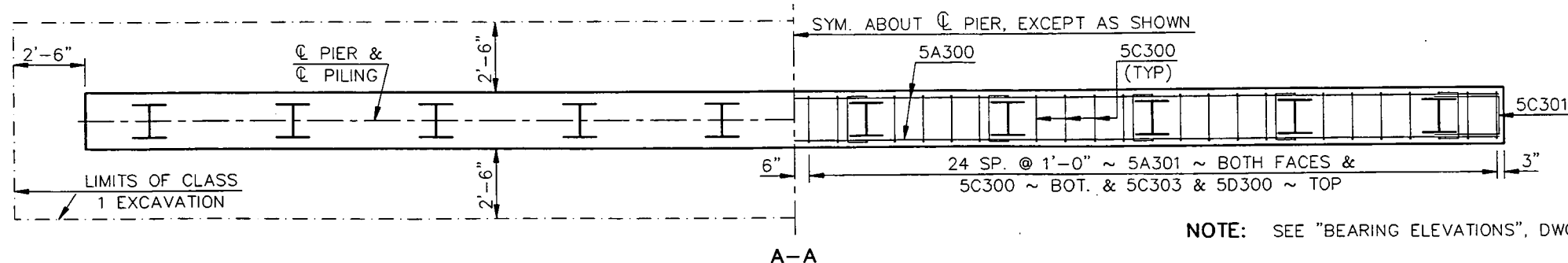
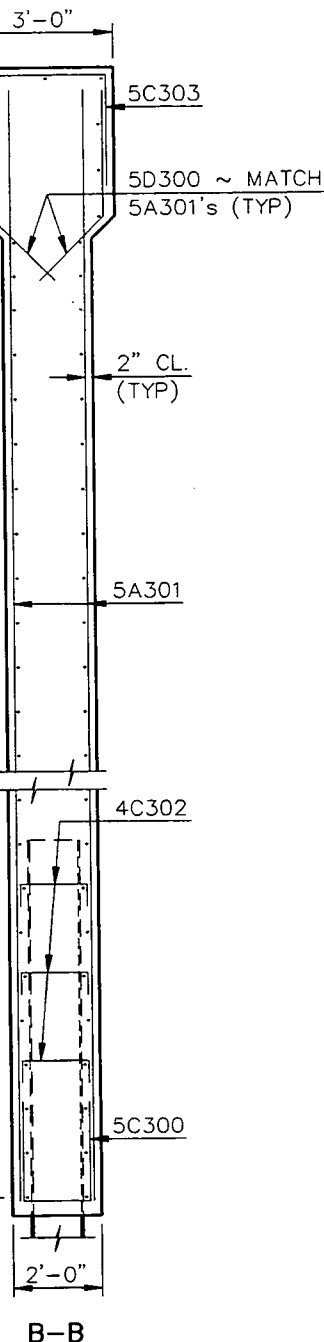
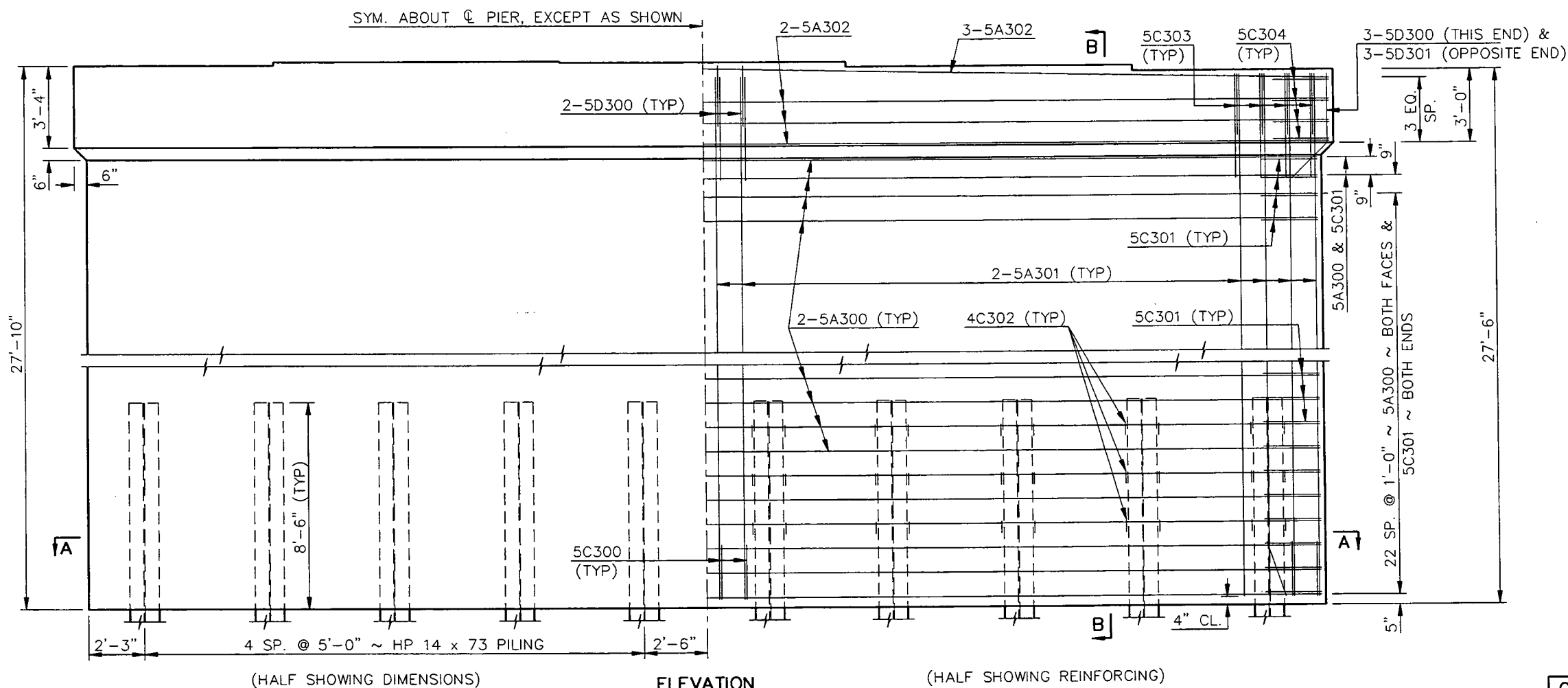
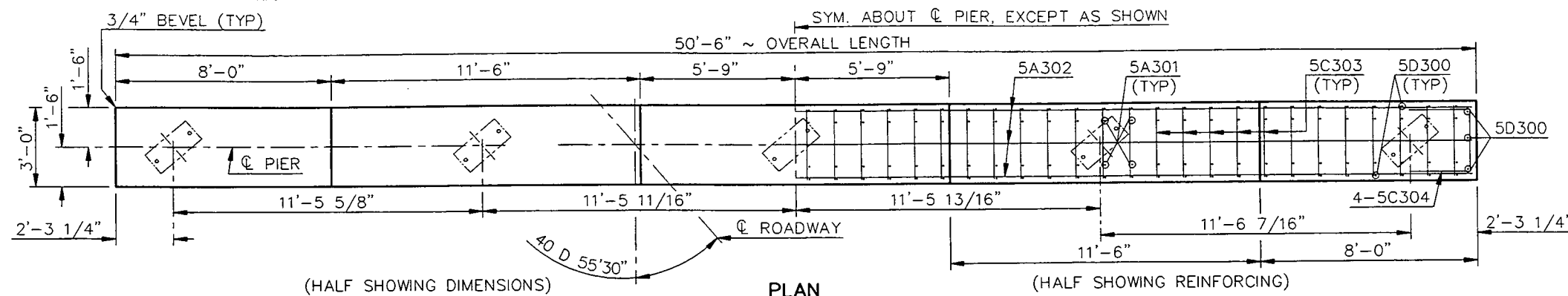
NOTE: SEE "BEARING ELEVATIONS", DWG. 2-053.731R-7 FOR STEPS IN PIER CAP.

QUANTITIES	
CLASS AE-3 CONCRETE	114 CY
REINFORCING STEEL	7820 LB
EXCAVATION (SEE LAYOUT)	

BURLINGTON NORTHERN OVERHEAD RAY

PIER 2 DETAILS

2-053.731R-13



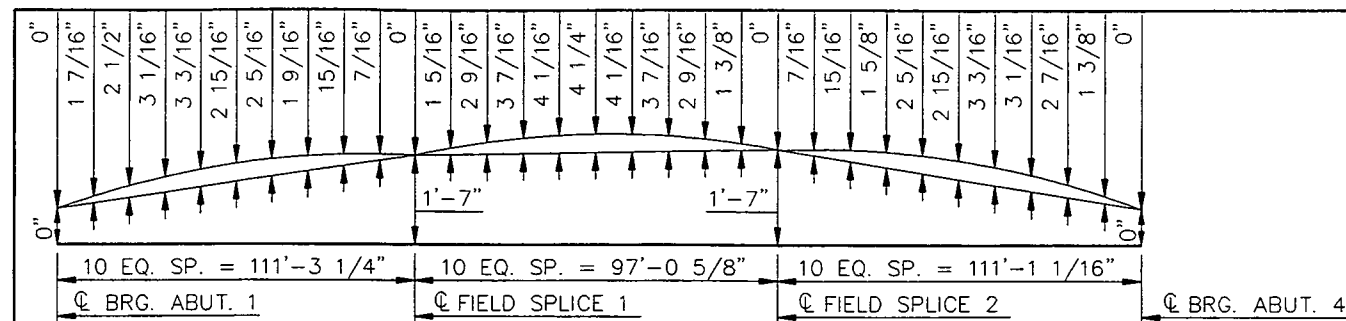
NOTE: SEE "BEARING ELEVATIONS", DWG. 2-053.731R-7

QUANTITIES	
CLASS AE-3 CONCRETE	109 CY
REINFORCING STEEL	7586 LB
EXCAVATION (SEE LAYOUT)	

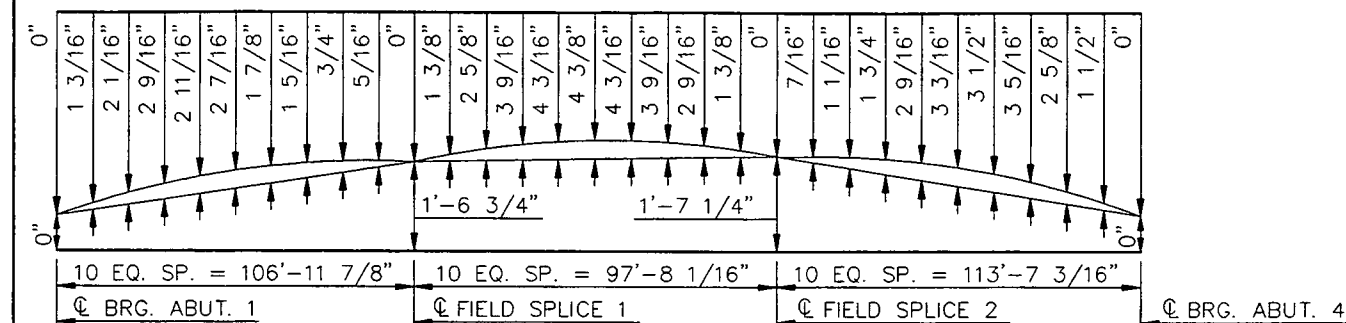
BURLINGTON NORTHERN OVERHEAD RAY

PIER 3 DETAILS

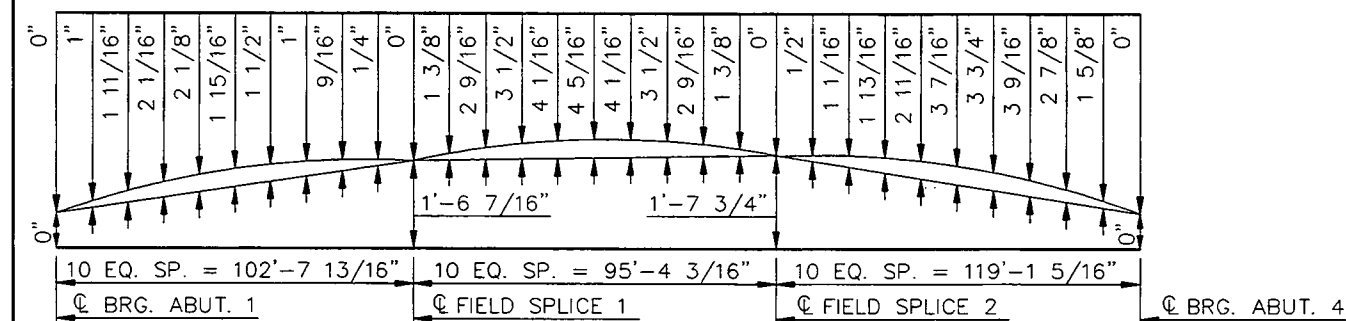
2-053.731R-14



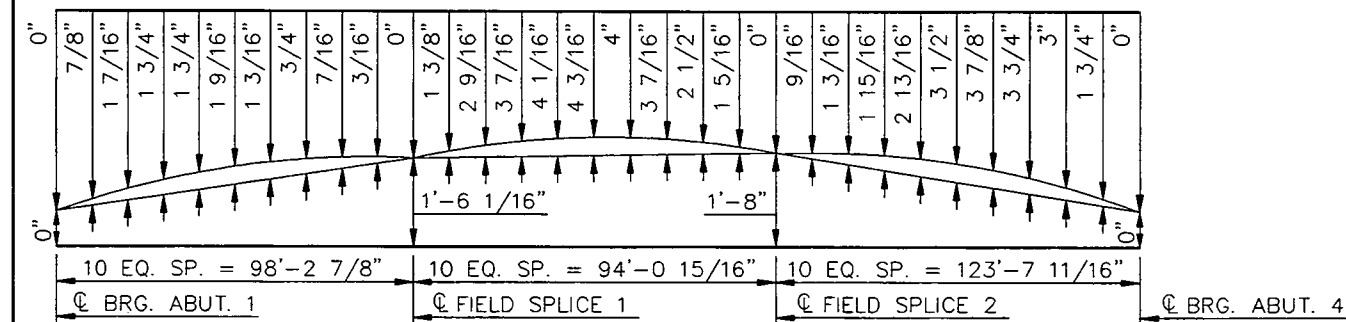
GIRDER 1 SHOP CAMBER DIAGRAM



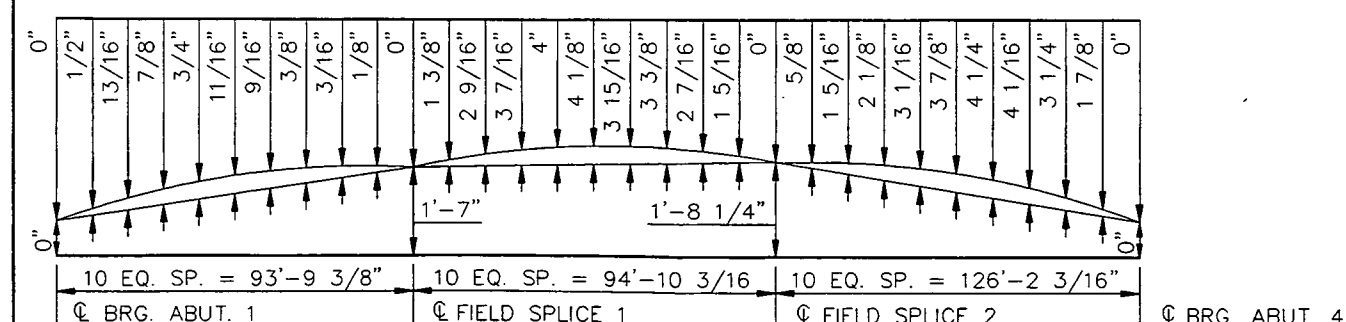
GIRDER 2 SHOP CAMBER DIAGRAM



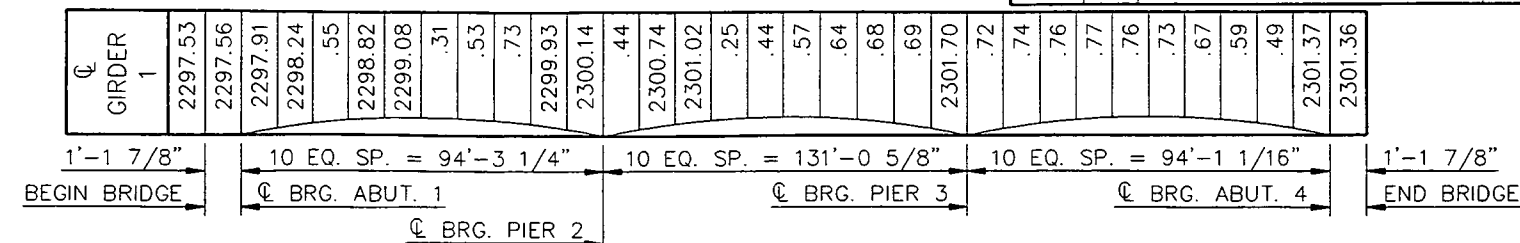
GIRDER 3 SHOP CAMBER DIAGRAM



GIRDER 4 SHOP CAMBER DIAGRAM

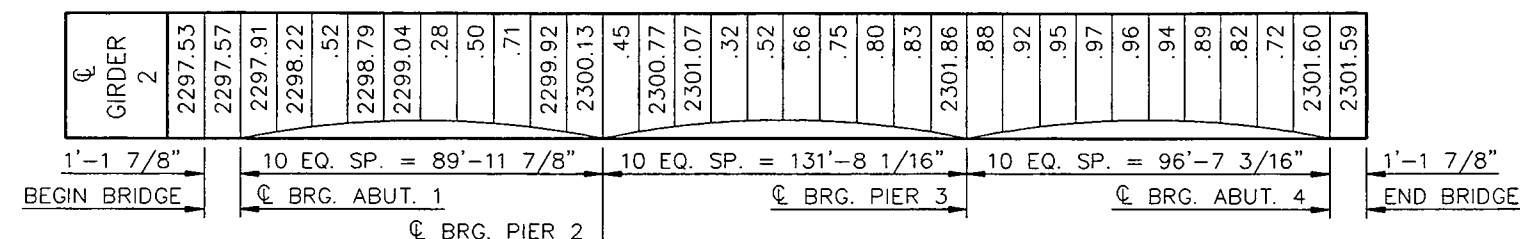


GIRDER 5 SHOP CAMBER DIAGRAM

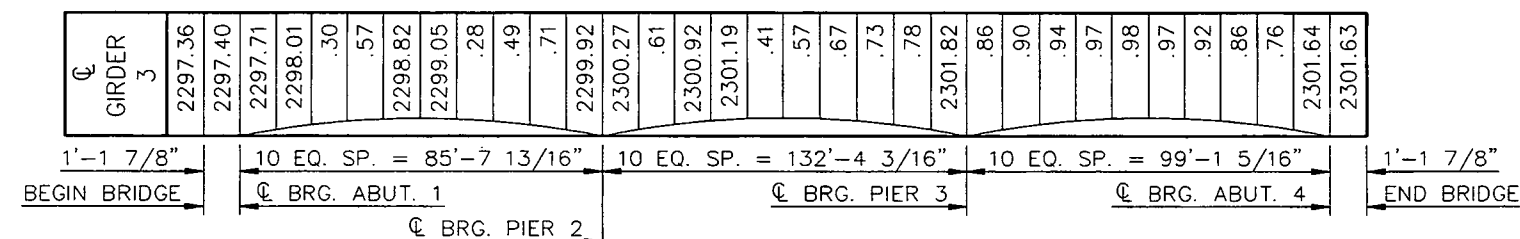


* GIRDER NO. 1 IS THE NORTH GIRDER

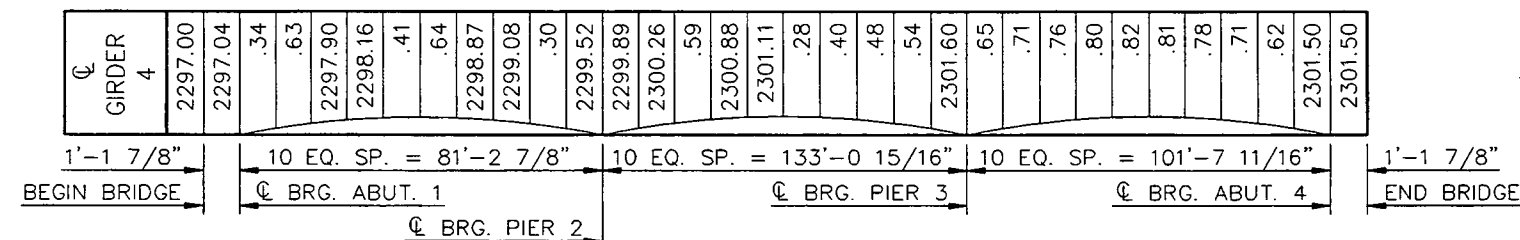
GIRDER 1 SCREED ELEVATIONS



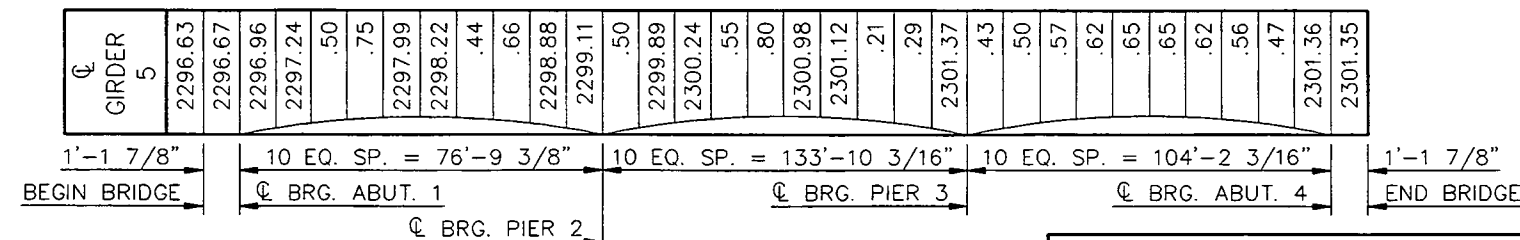
GIRDER 2 SCREED ELEVATIONS



GIRDER 3 SCREED ELEVATIONS



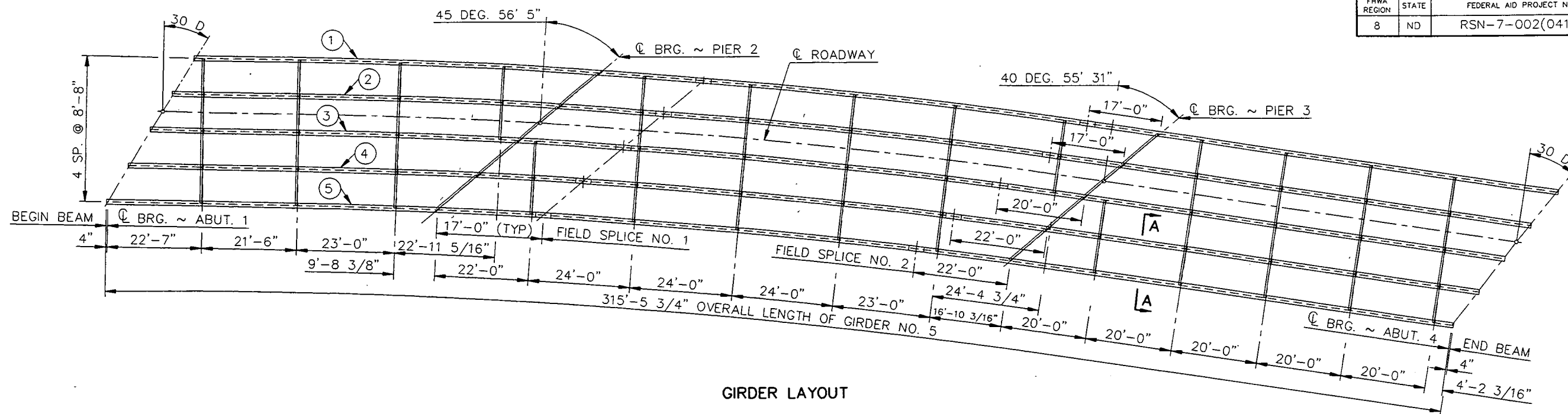
GIRDER 4 SCREED ELEVATIONS



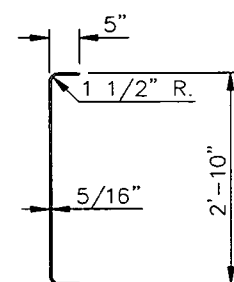
GIRDER 5 SCREED ELEVATIONS

BURLINGTON NORTHERN OVERHEAD
RAY

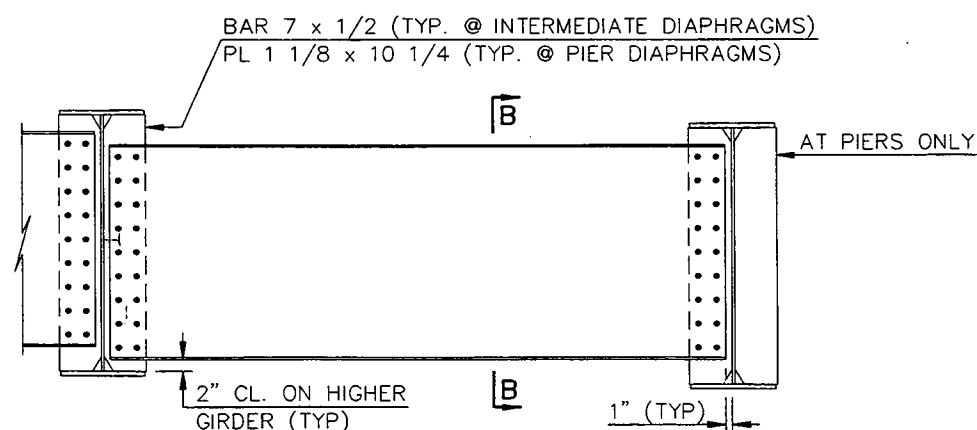
SHOP CAMBER DIAGRAMS
& SCREED ELEVATIONS



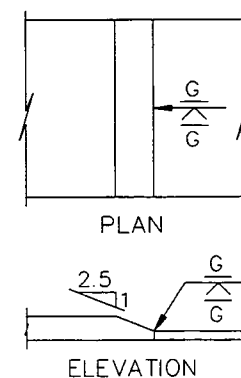
GIRDER LAYOUT



B-B



A-A



TYPICAL SHOP FLANGE SPLICE

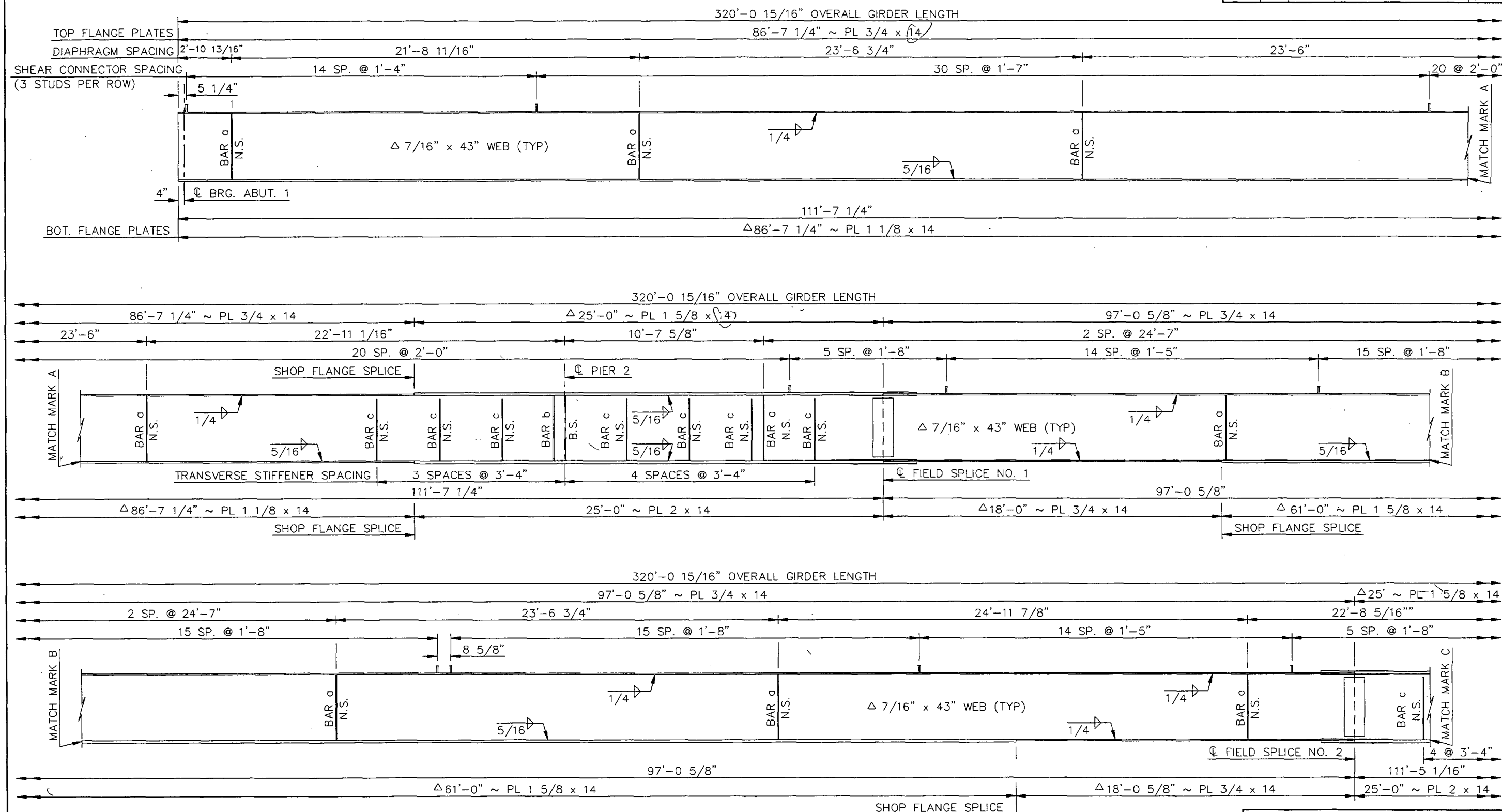
NOTE:

THE DIMENSIONS SHOWN ON THE GIRDER LAYOUT ARE MEASURED ALONG THE CENTERLINE OF THE OUTSIDE GIRDER, GIRDER NO. 5.

ALL INTERMEDIATE DIAPHRAGMS IN THE TANGENT SECTIONS SHALL BE PLACED PERPENDICULAR. ALL INTERMEDIATE DIAPHRAGMS IN THE CURVED SECTION SHALL BE PLACED ALONG THE RADIAL.

BURLINGTON NORTHERN OVERHEAD
RAY

GIRDER DETAILS



NOTES:

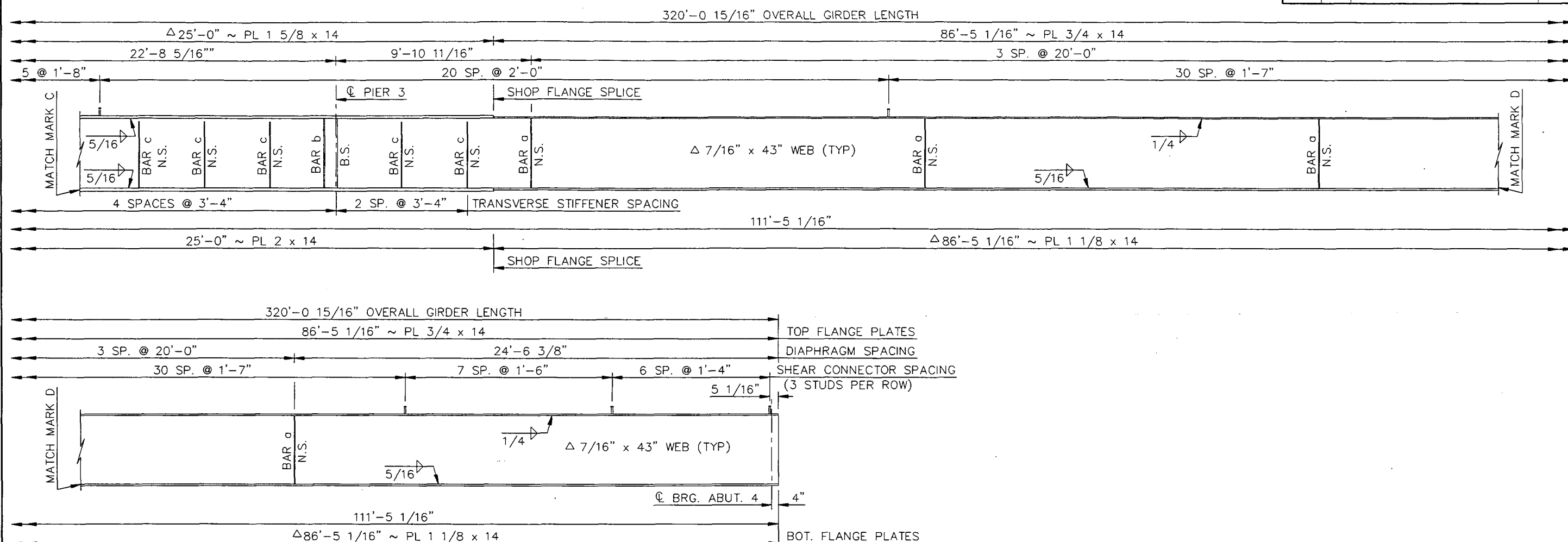
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES

SEE DWG. 2-053.731R-18 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 1 ELEVATION DETAILS



NOTES:

Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

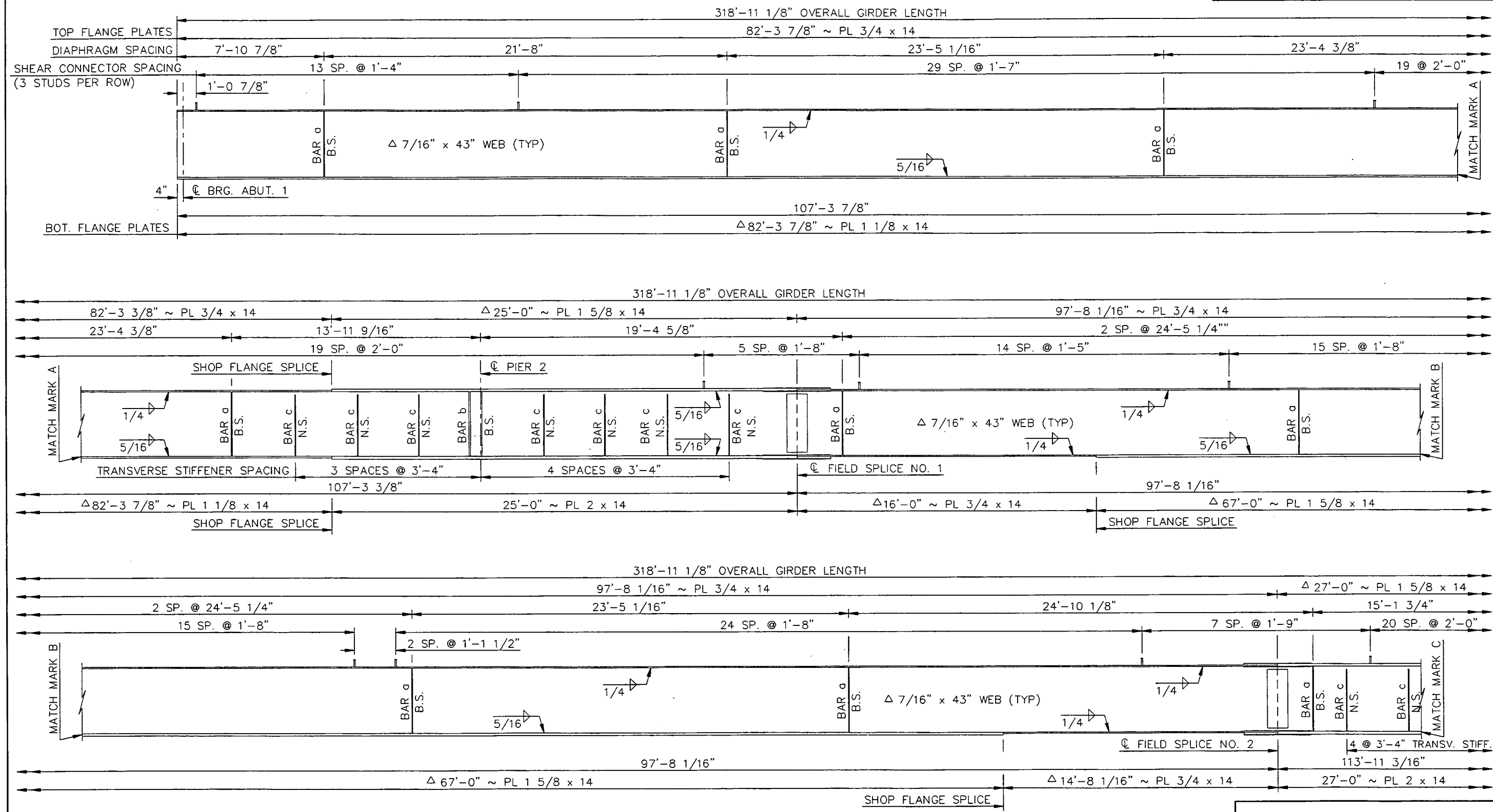
NOMENCLATURE:

N.S. = NEAR SIDE
B.S. = BOTH SIDES

SEE DWG. 2-053.731R-17 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 1 ELEVATION DETAILS



NOTES:
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

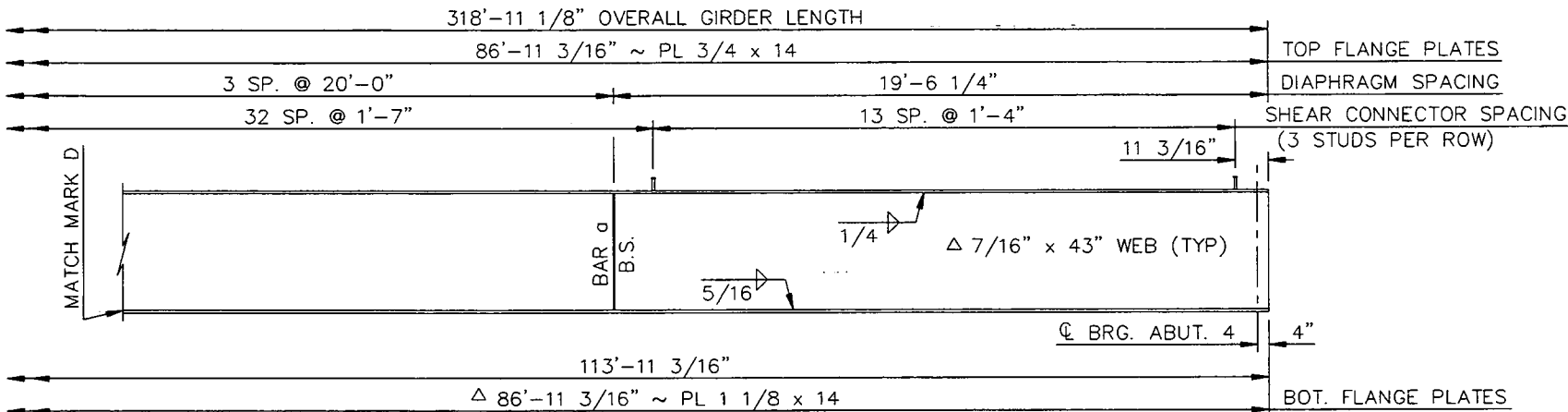
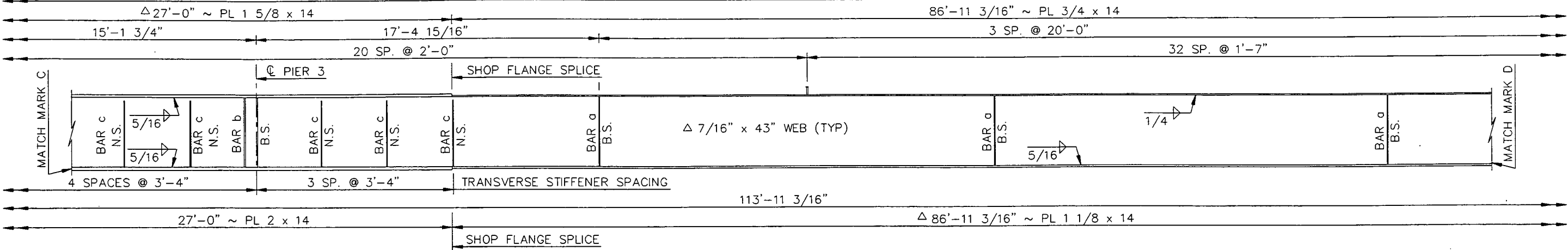
NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES

SEE DWG. 2-053.731R-20 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 2 ELEVATION DETAILS

318'-11 1/8" OVERALL GIRDER LENGTH



NOTES:

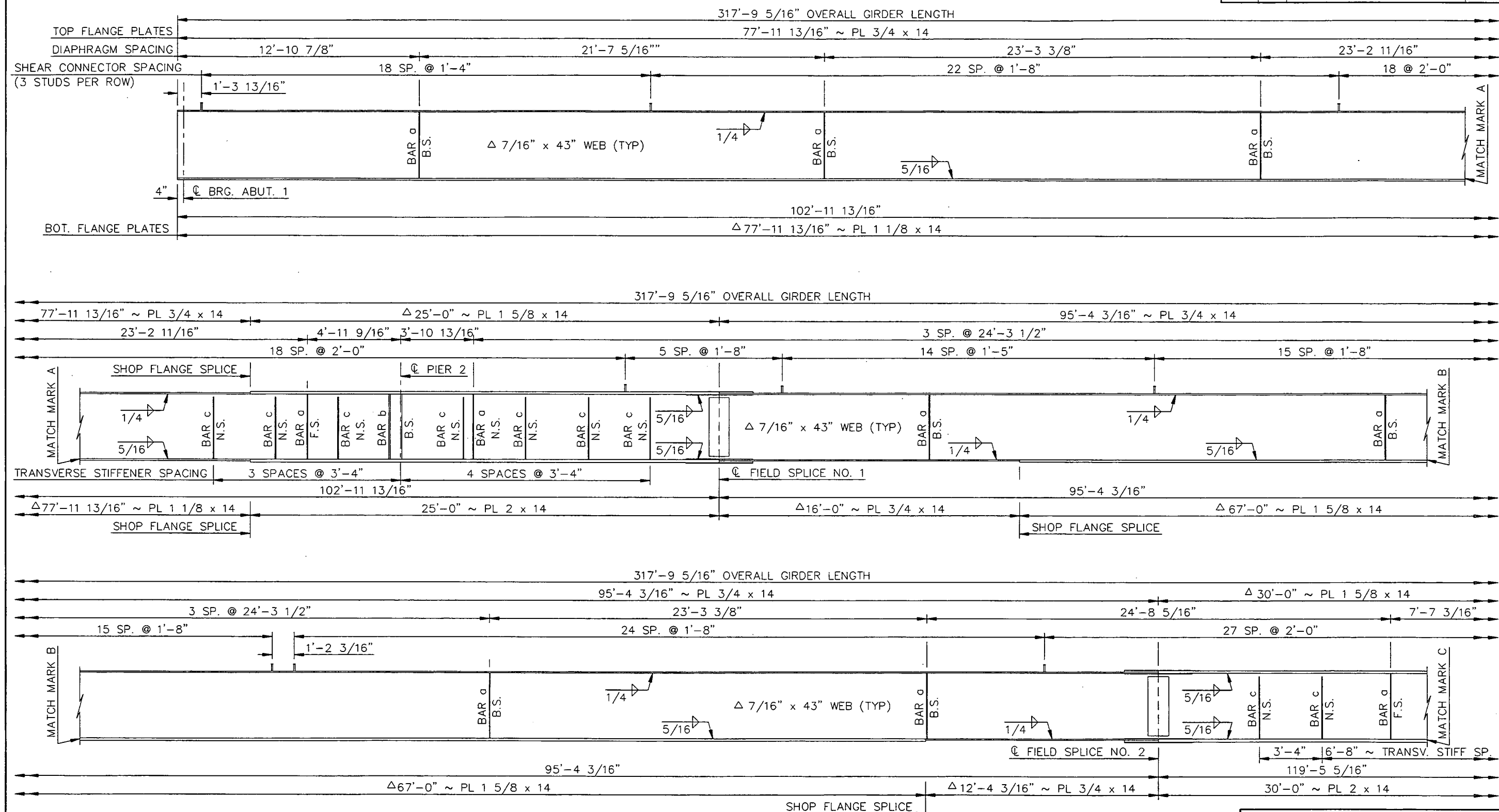
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES

SEE DWG. 2-053.731R-19 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 2 ELEVATION DETAILS



NOTES:

△ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

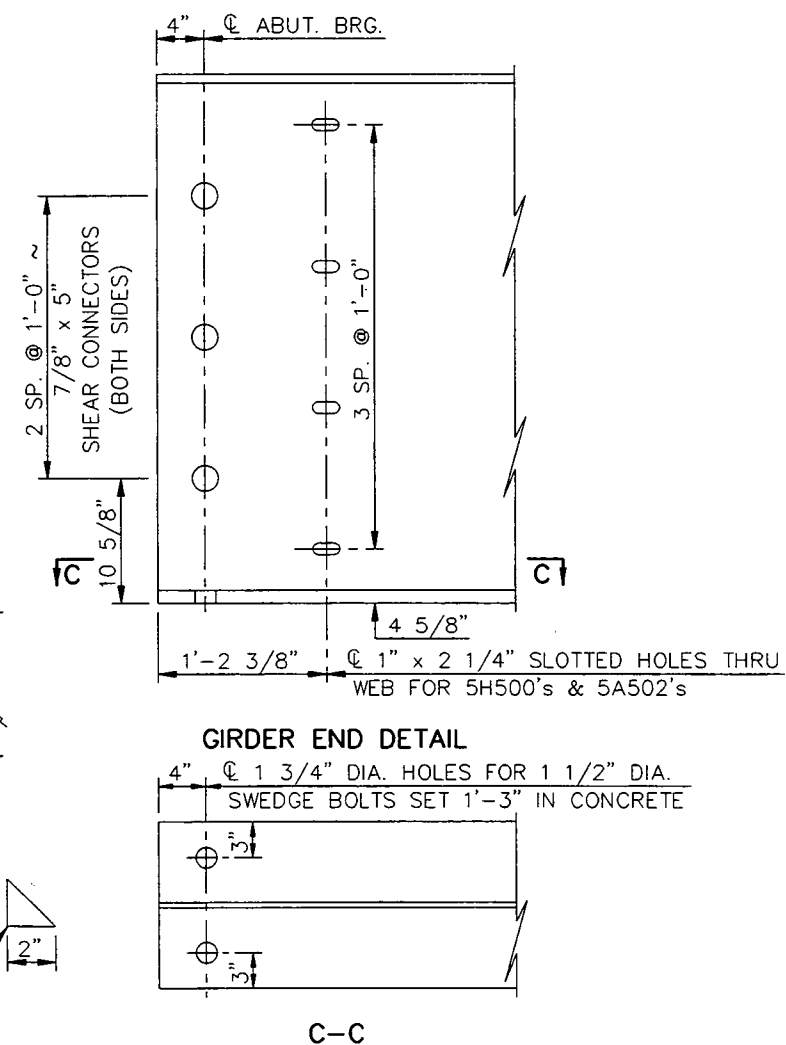
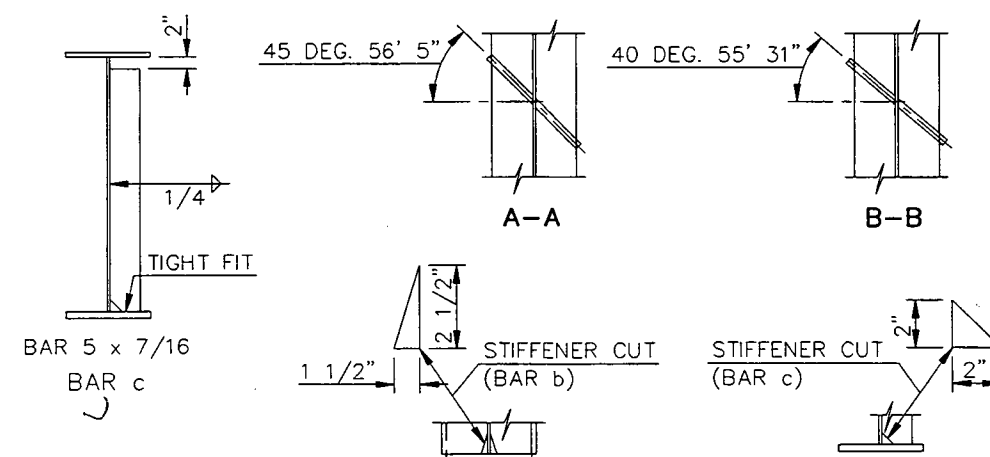
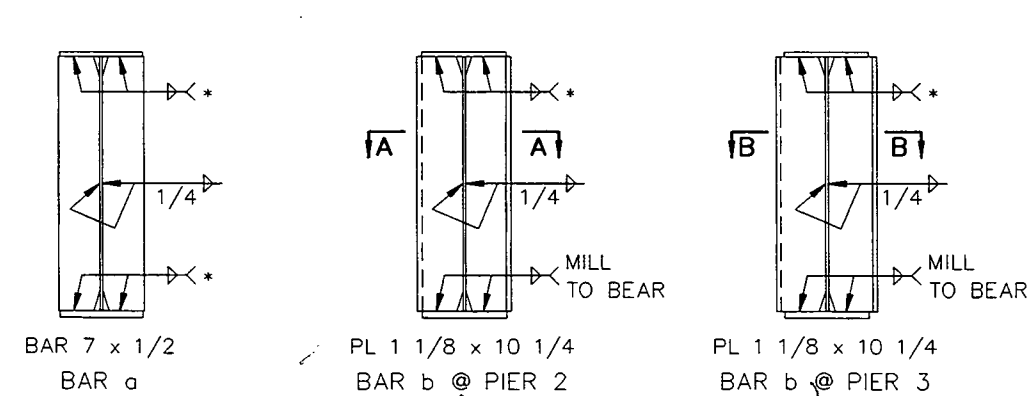
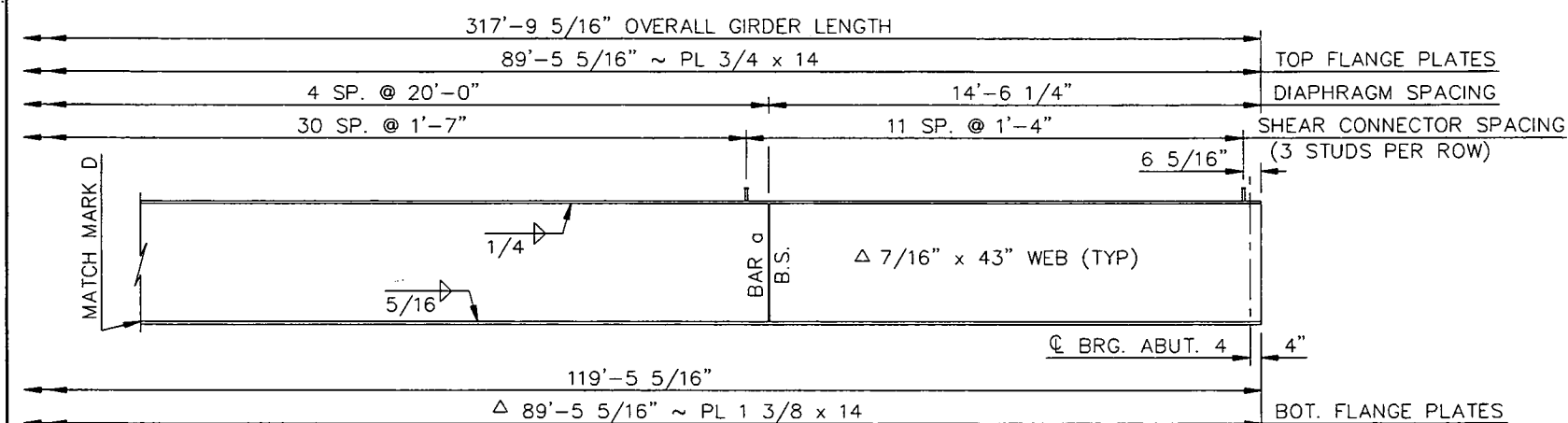
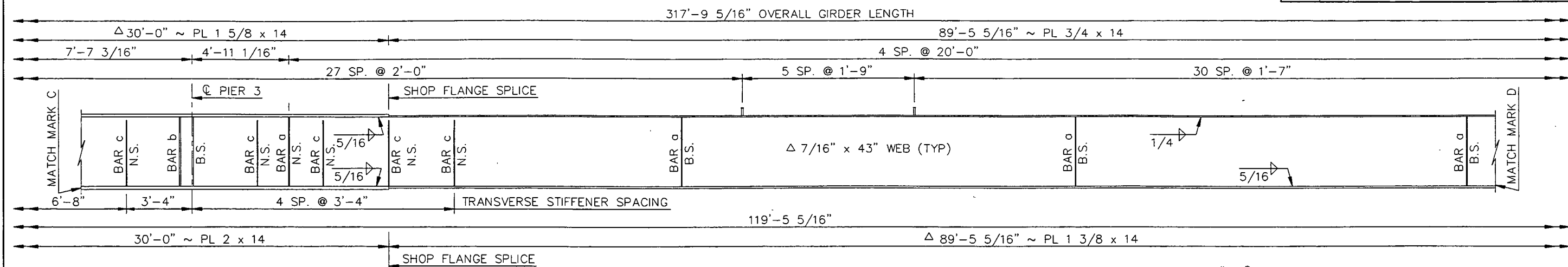
NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

SEE DWG. 2-053.731R-22 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 3 ELEVATION DETAILS

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	146



STIFFENER & DIAPHRAGM CONNECTION PLATE DETAILS

* WELDSIZE	FLANGE THICKNESS
1/4	TO 3/4
5/16	OVER 3/4

NOTES:

△ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

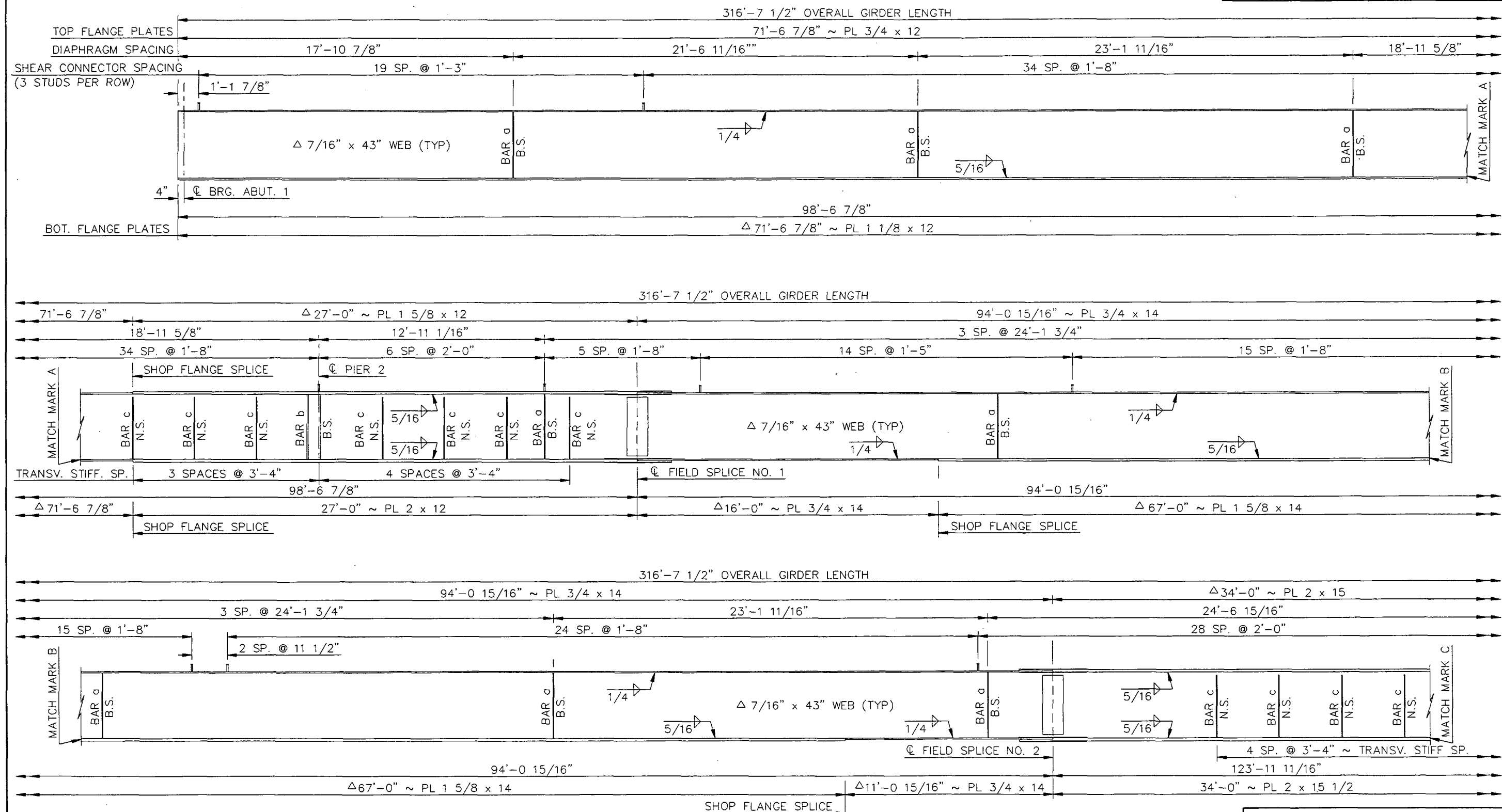
NOMENCLATURE:

N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

SEE DWG. 2-053.731R-21
FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD
RAY

GIRDER 3 ELEVATION DETAILS,
GIRDER END DETAIL, STIFFENER &
DIAPHR. CONNECTION PL DETAILS



NOTES:

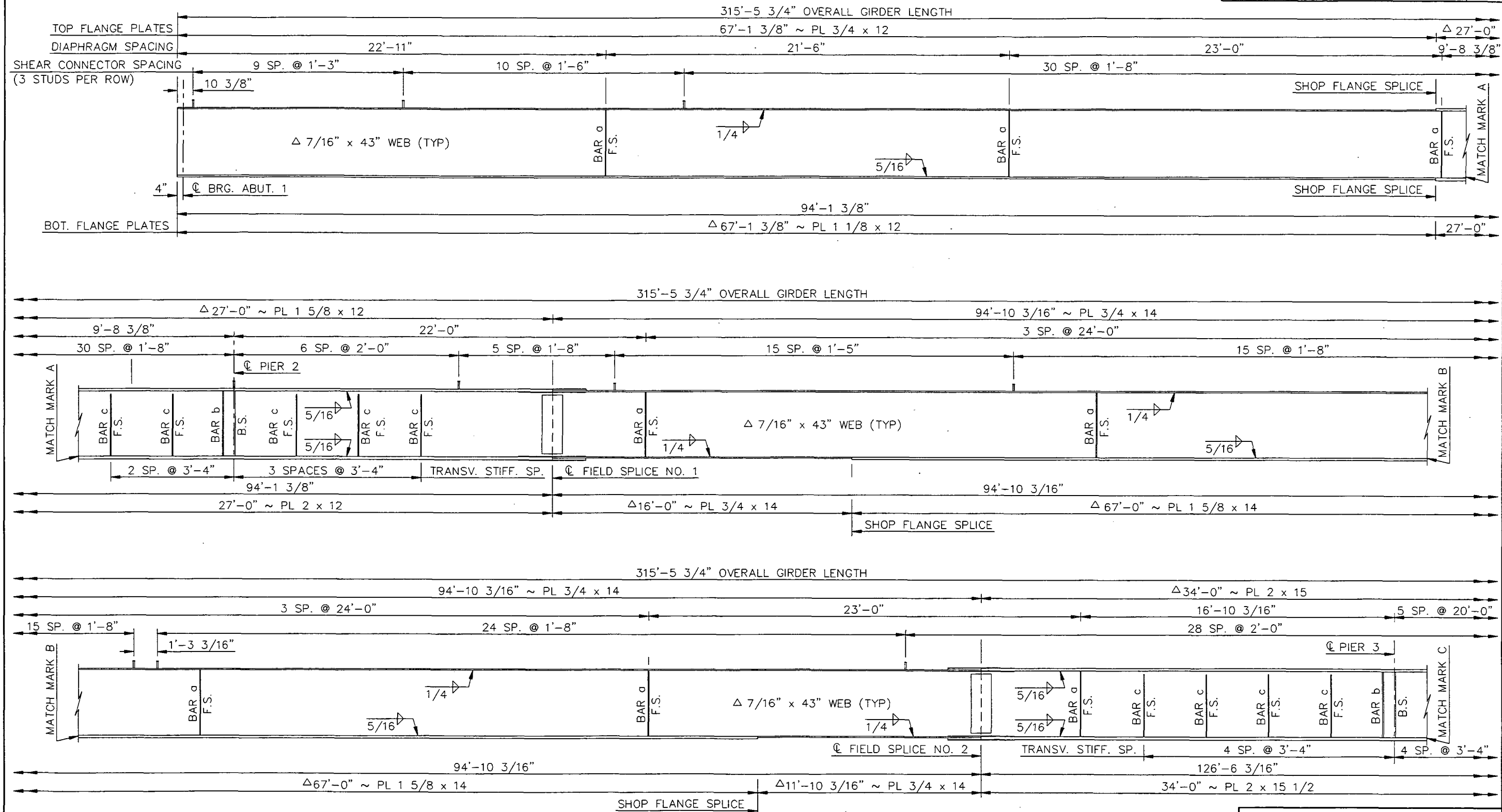
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES

SEE DWG. 2-053.731R-24 FOR MATCH MARK C.

BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 4 ELEVATION DETAILS



NOTES:

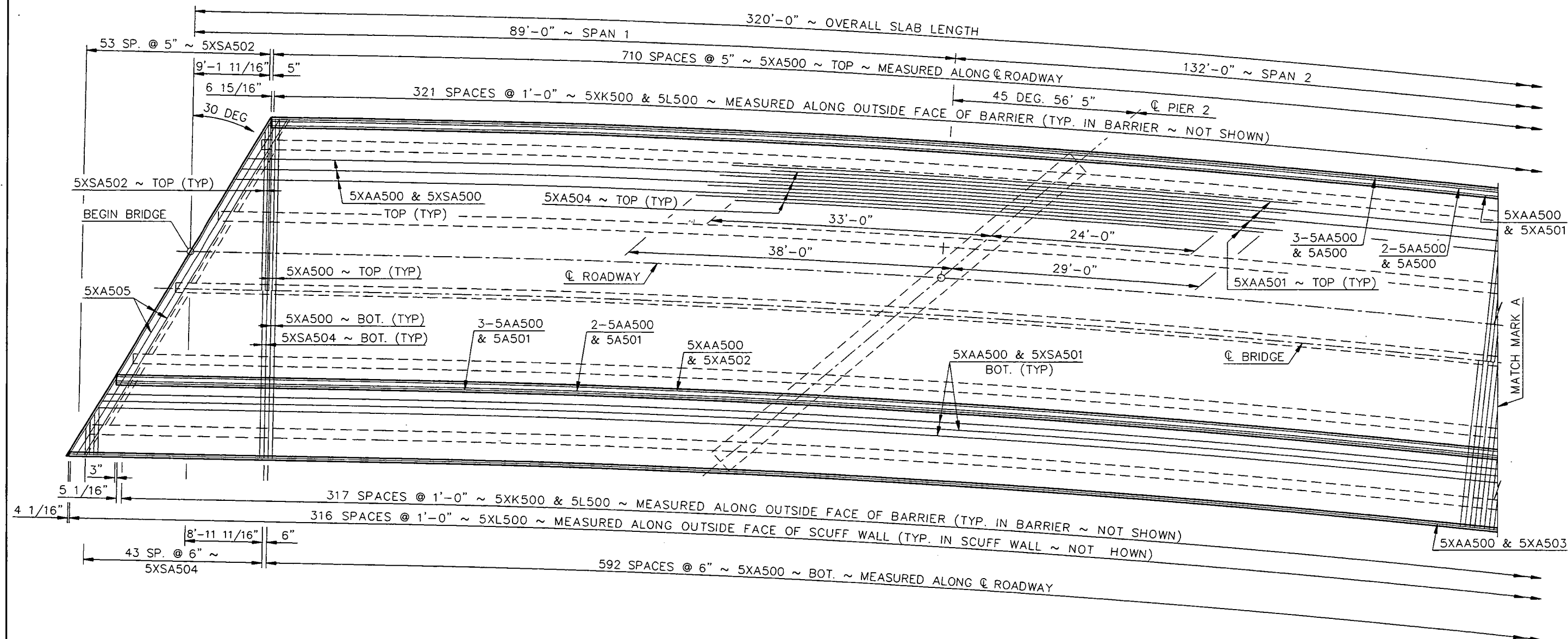
Δ ALL DESIGNATED MATERIALS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST REQUIREMENTS FOR ZONE 2.

NOMENCLATURE:
N.S. = NEAR SIDE
B.S. = BOTH SIDES
F.S. = FAR SIDE

SEE DWG. 2-053.731R-26 FOR MATCH MARK C.

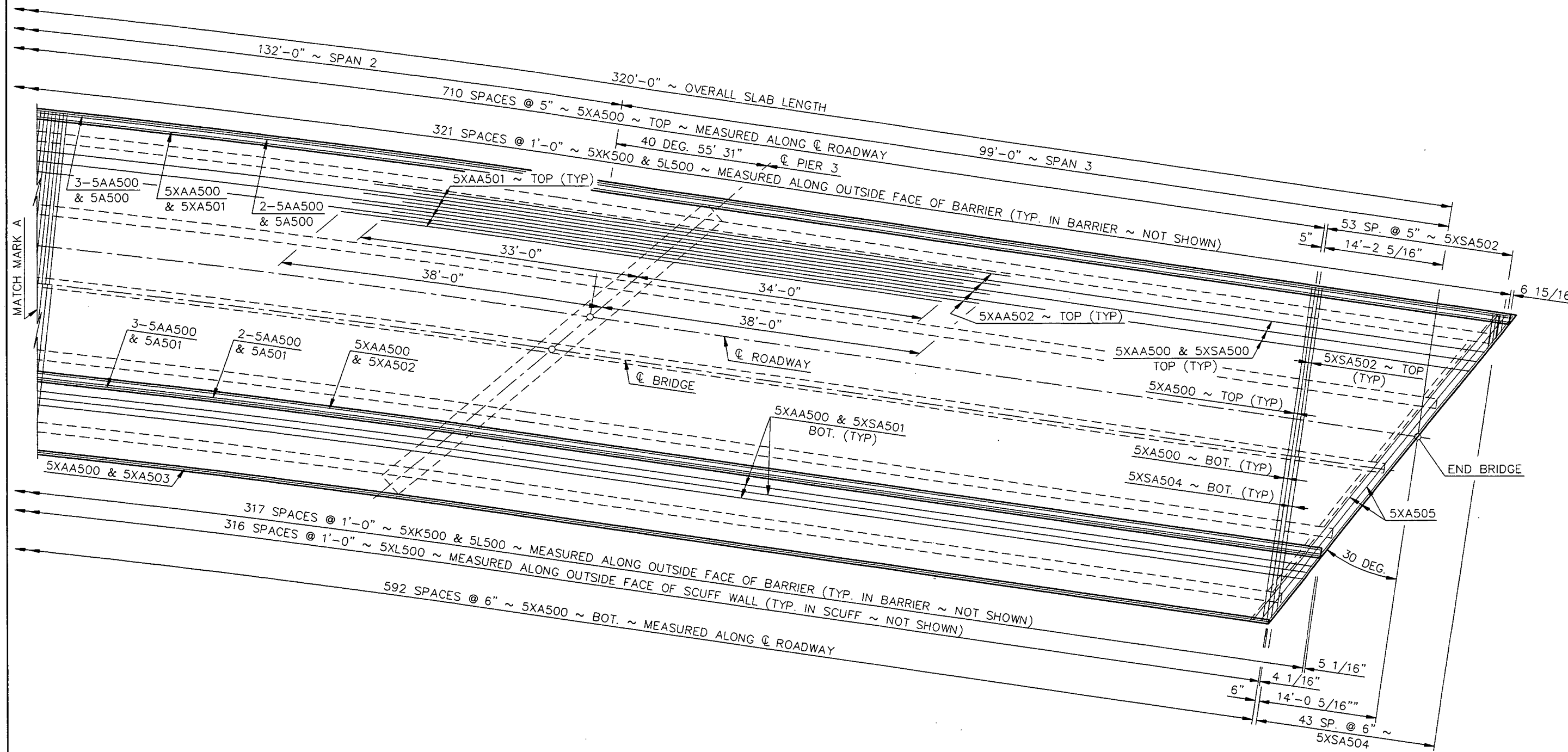
BURLINGTON NORTHERN OVERHEAD RAY

GIRDER 5 ELEVATION DETAILS



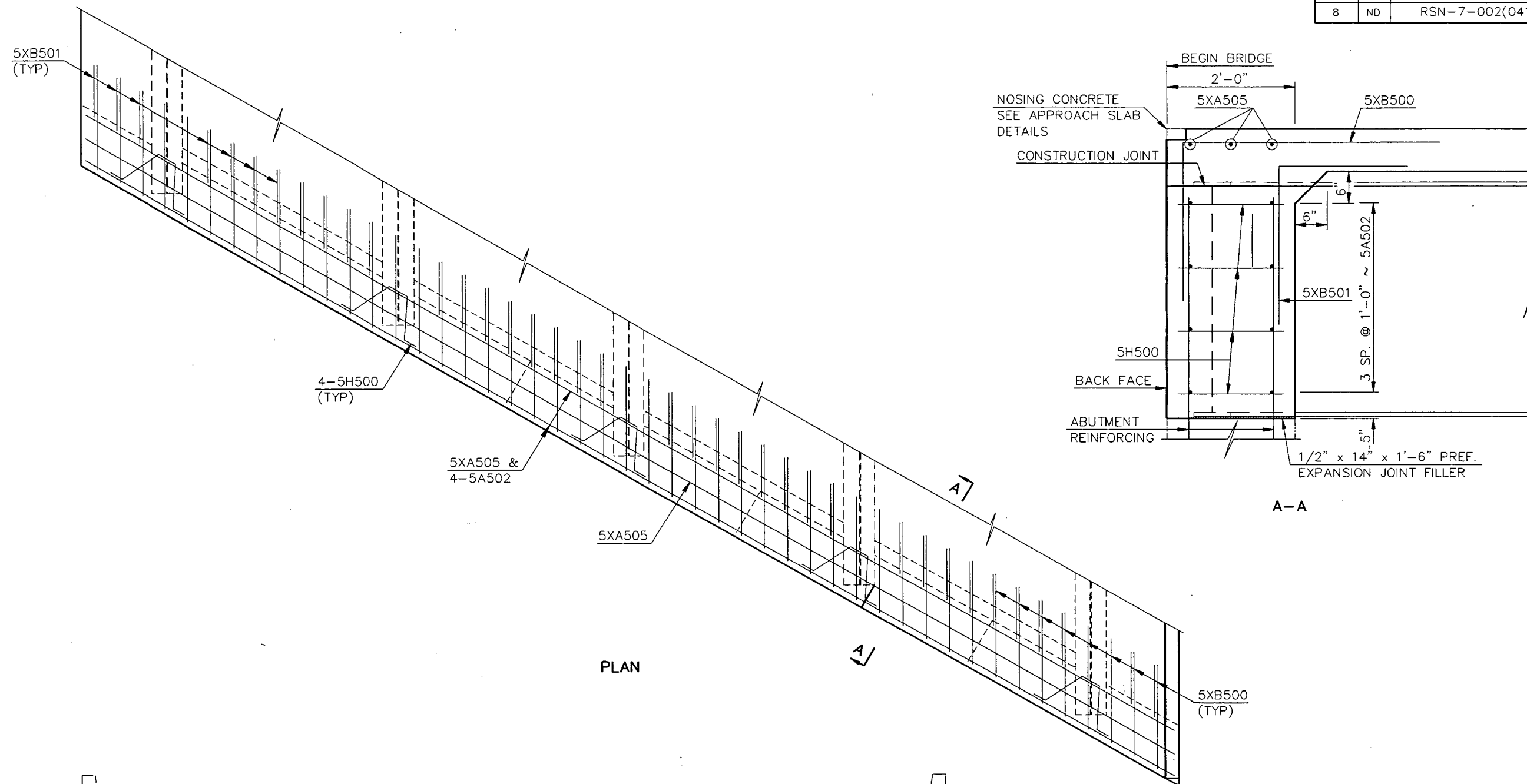
NOTE:
SEE DWG. 2-053.731R-28 FOR MATCH MARK A.

QUANTITIES
SEE DWG. 2-053.731R-30
BURLINGTON NORTHERN OVERHEAD RAY
SLAB LAYOUT

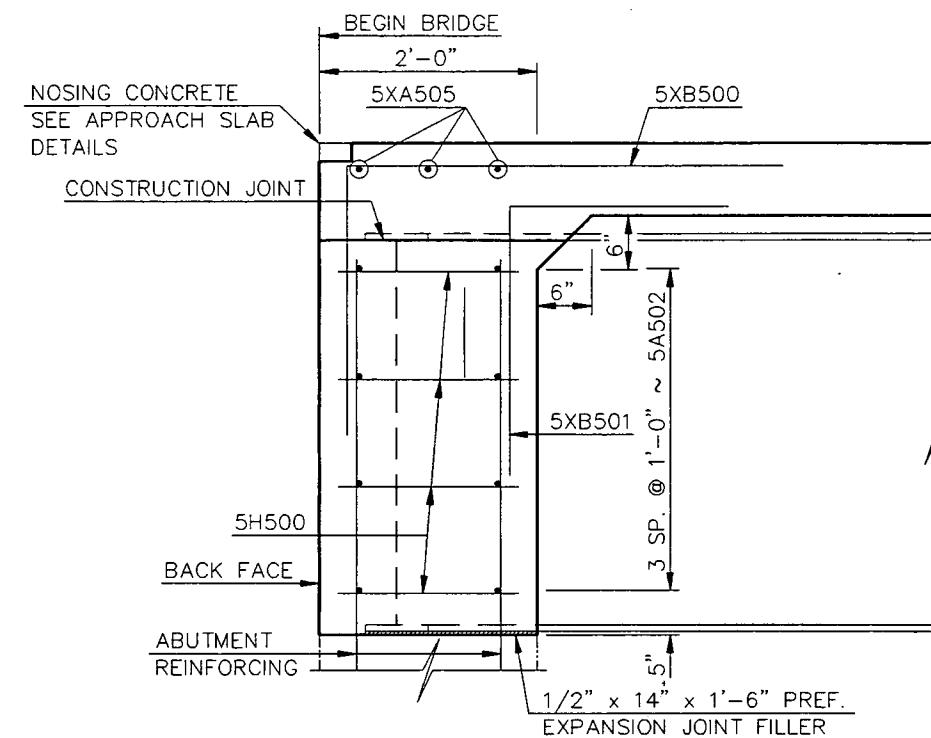


NOTE:
SEE DWG. 2-053.731R-27 FOR MATCH MARK A.

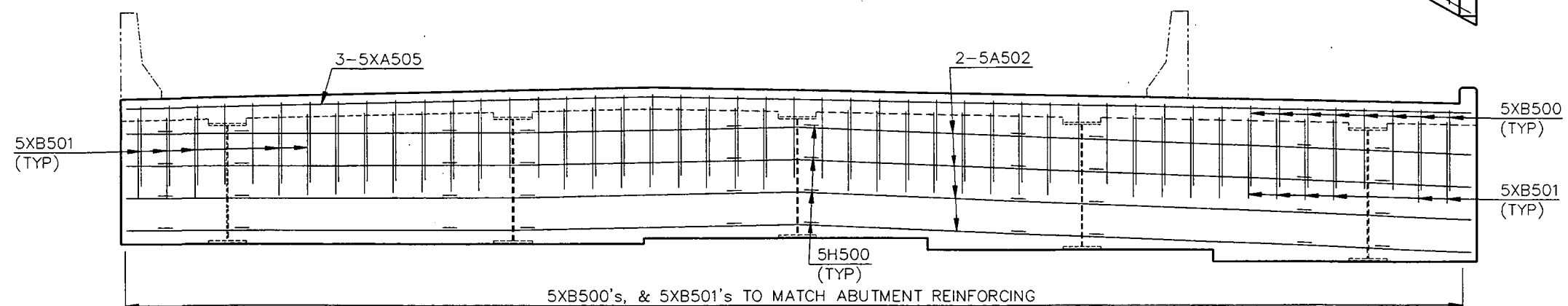
QUANTITIES	
SEE DWG. 2-053.731R-30	
BURLINGTON NORTHERN OVERHEAD RAY	
SLAB LAYOUT	



PLAN



A-A



ELEVATION

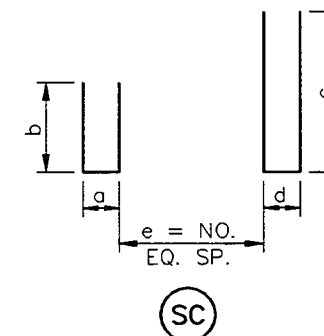
QUANTITIES
SEE DWG. 2-053.731R-30
BURLINGTON NORTHERN OVERHEAD RAY
END BEAM DETAILS

2-053.731R-29

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	155

NOTES:

1. FABRICATION AND TOLERANCES SHALL BE IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.
2. ALL DIMENSIONS ARE OUT TO OUT OF BARS.
3. NOMINAL LENGTH OF EACH BENT BAR OR CUT BAR IS THE SUM TOTAL OF THE DETAILING DIMENSIONS FOR THAT BAR, UNLESS OTHERWISE NOTED.
4. ADJACENT "AA" BARS SHALL BE TURNED END FOR END SO THAT THE SPLICE LOCATIONS ARE STAGGERED.



2-053.731R-31

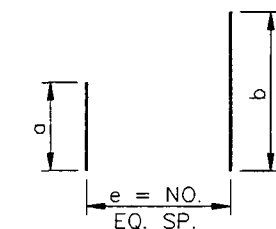
BILL OF REINFORCING STEEL, GRADE 60

LETTER PREFIX OF BAR MARK DENOTES SHAPE ~ SEE BAR DETAILS

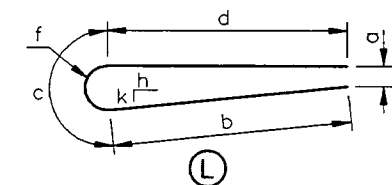
FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	156

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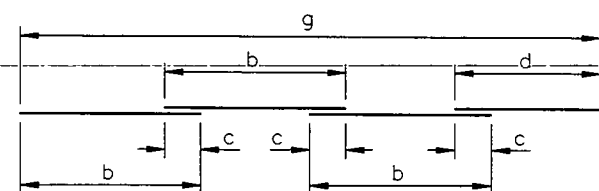
- NOTES:**
- FABRICATION AND TOLERANCES SHALL BE IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.
 - ALL DIMENSIONS ARE OUT TO OUT OF BARS.
 - NOMINAL LENGTH OF EACH BENT BAR OR CUT BAR IS THE SUM TOTAL OF THE DETAILING DIMENSIONS FOR THAT BAR, UNLESS OTHERWISE NOTED.
 - ADJACENT "AA" BARS SHALL BE TURNED END FOR END SO THAT THE SPLICE LOCATIONS ARE STAGGERED.
 - THE "f" DIMENSION INDICATES THE RADIUS.
 - AN "X" PRECEDING A BAR DESIGNATION INDICATES AN EPOXY COATED BAR.



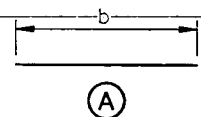
(SA)



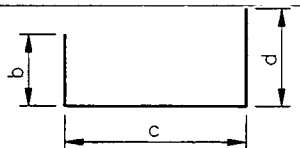
(L)



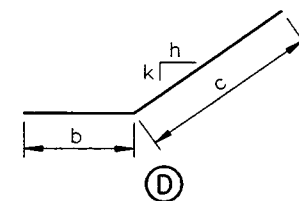
(AA)



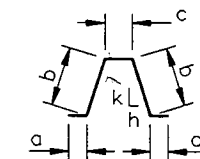
(B)



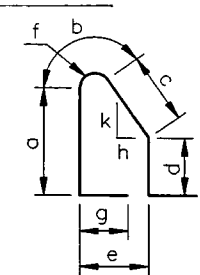
(C)



(D)



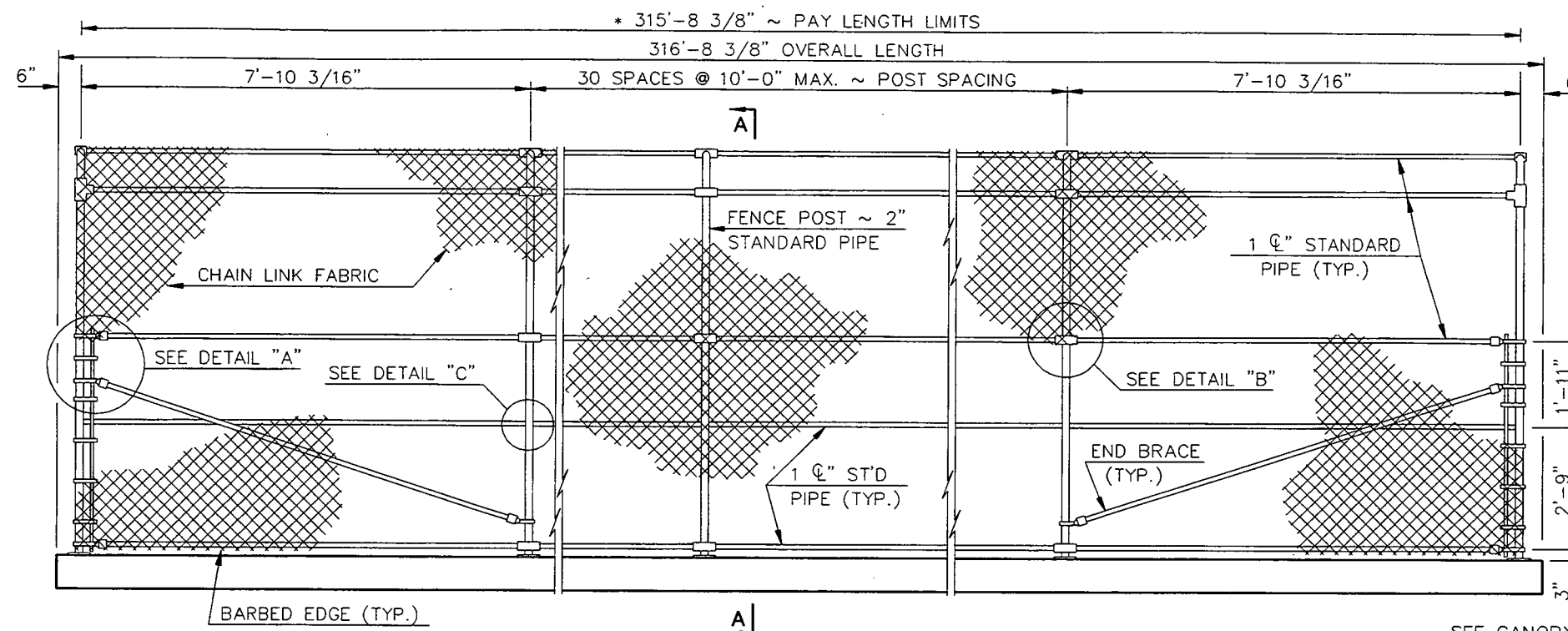
(H)



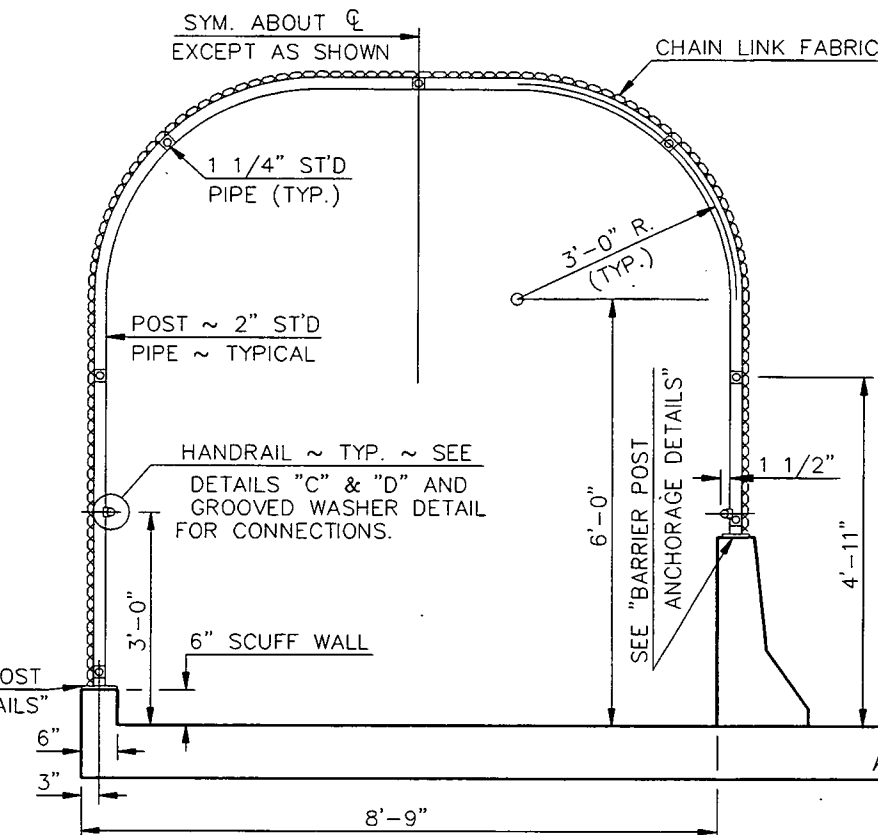
(K)

BURLINGTON NORTHERN OVERHEAD RAY

PIER & SUPERSTRUCTURE REINFORCING BAR LIST & DETAILS



CANOPY ELEVATION



NOTE: A-A

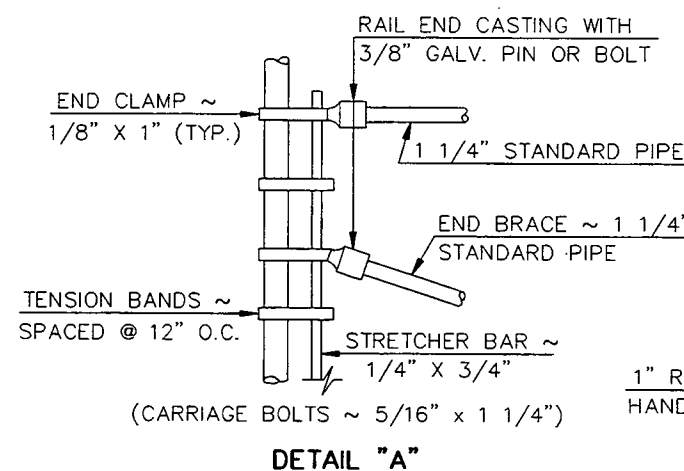
FABRIC SHALL BE AASHTO M181, TYPE 1 AND SHALL BE 9 GAGE WIRE, 2 INCH MESH, 60 INCHES HIGH, AND THE BOTTOM SELVAGE BARBED.

POSTS AND FITTINGS SHALL BE AASHTO M181, CLASS 1.

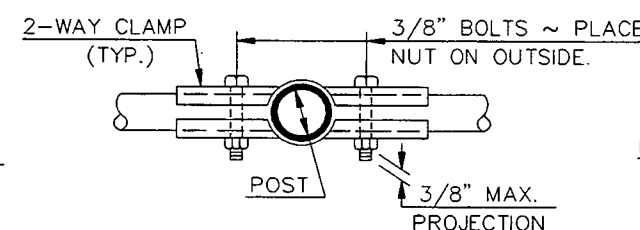
ANCHORAGE PLATES SHALL BE AASHTO M183 AND SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH AASHTO M232.

TO ANCHOR BASE PLATES, THE CONTRACTOR HAS THE OPTION TO EITHER DRILL IN 1/2" ϕ SWEDGE BOLTS OR PLACE THEM IN THE CONCRETE AT THE TIME OF POURING. IF THE CONTRACTOR ELECTS TO DRILL THE BOLT HOLES, EXTREME CAUTION SHALL BE EXERCISED NOT TO DAMAGE THE CONCRETE BARRIERS AND SHALL PLACE THE BASE PLATES LONGITUDINALLY ALONG THE BARRIERS AS SHOWN ON THE "BASE PLATE PLAN" VIEW.

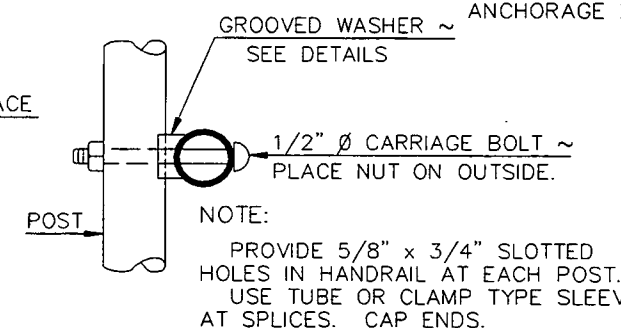
* THE PAY LENGTH IS MEASURED ALONG THE OUTSIDE FACE OF THE SCUFF WALL. POSTS LOCATED IN THE TANGENT SECTIONS SHALL BE PLACED PERPENDICULAR. POSTS LOCATED IN THE CURVE SECTION SHALL BE PLACED RADially.



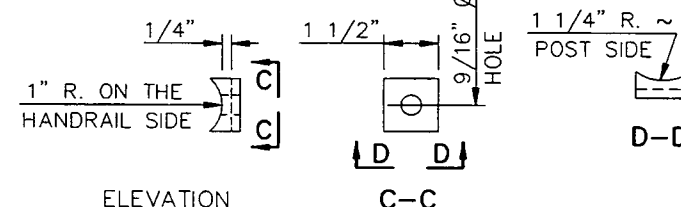
DETAIL "A"



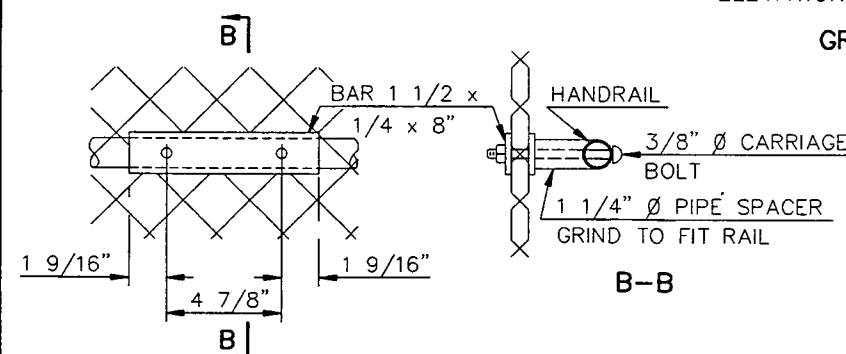
DETAIL "B"



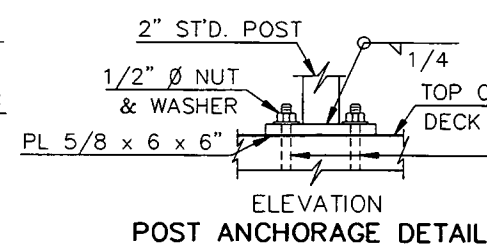
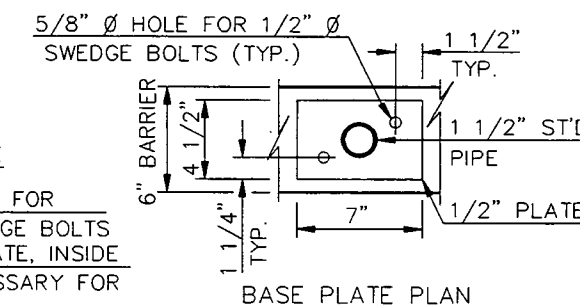
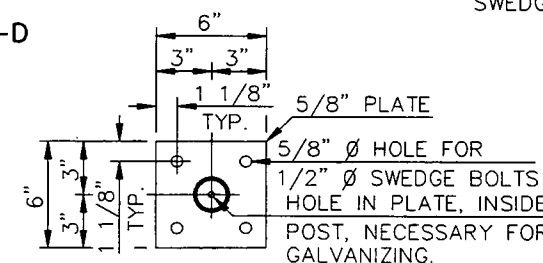
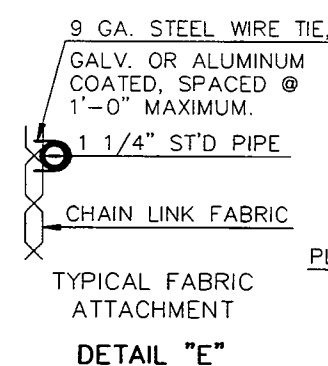
HANDRAIL CONNECTION AT POSTS
DETAIL "C"



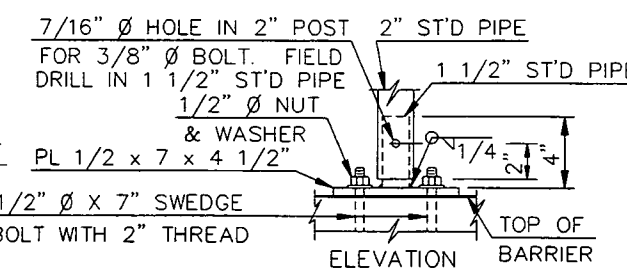
GROOVED WASHER DETAILS



HANDRAIL CONNECTION DETAILS CENTERED BETWEEN POSTS
DETAIL "D"



POST ANCHORAGE DETAILS



BARRIER POST ANCHORAGE DETAILS

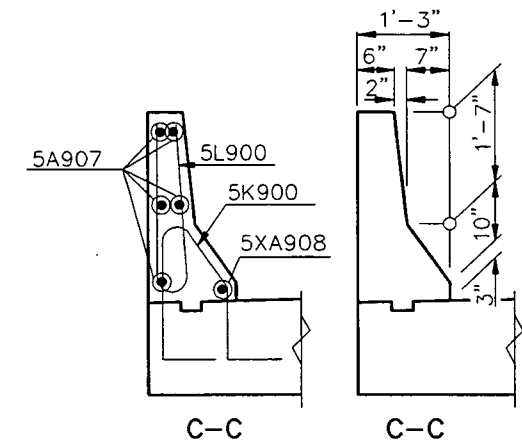
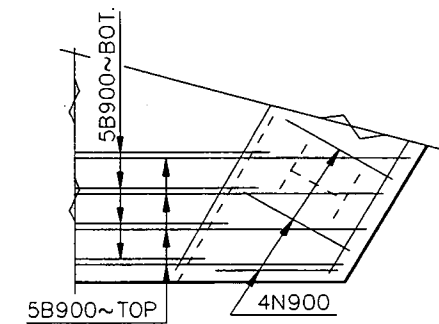
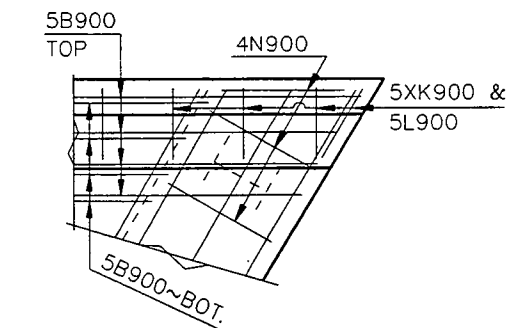
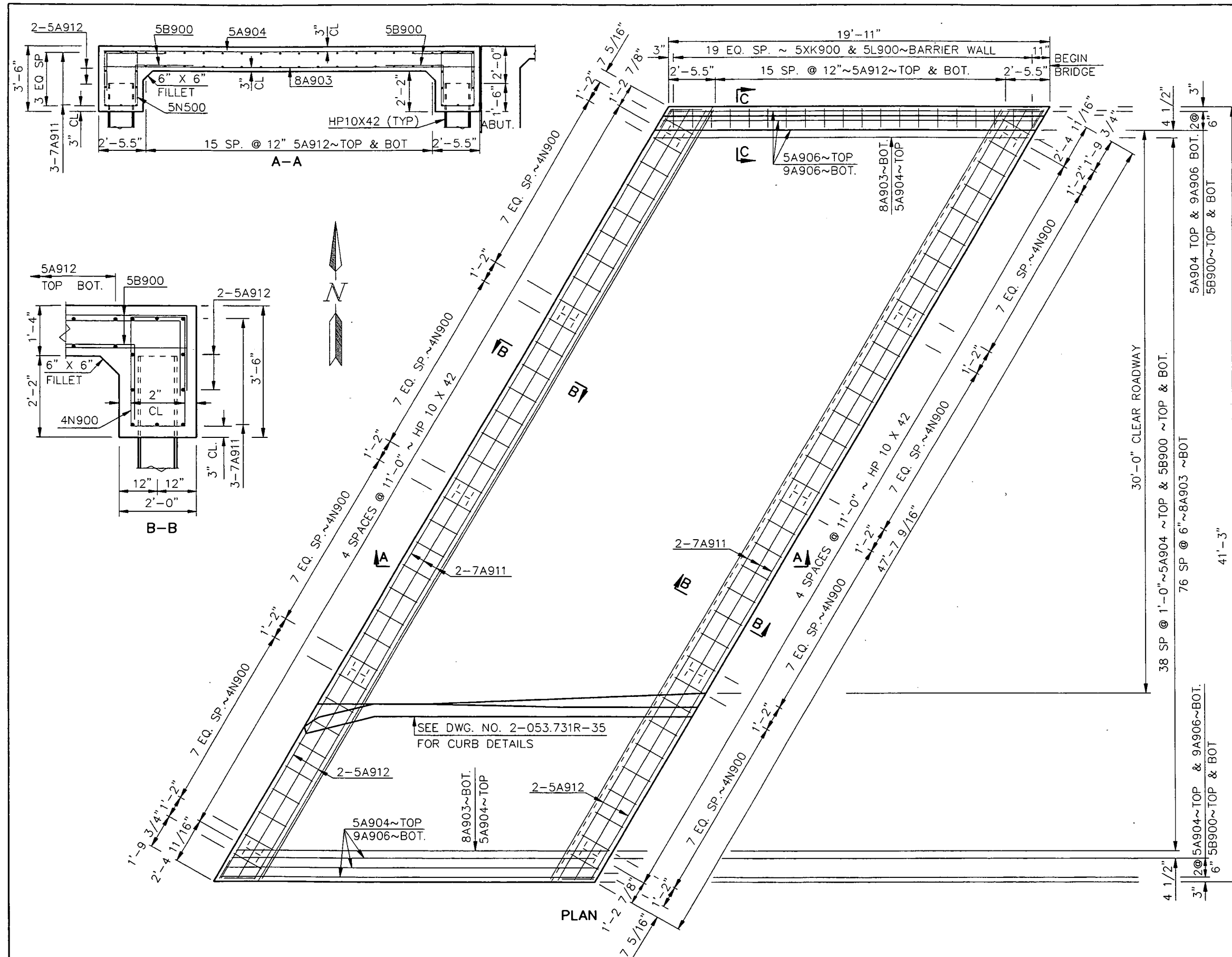
QUANTITIES

PEDESTRIAN CANOPY 316.7 LF

BURLINGTON NORTHERN OVERHEAD RAY

PEDESTRIAN CANOPY DETAILS

FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	158

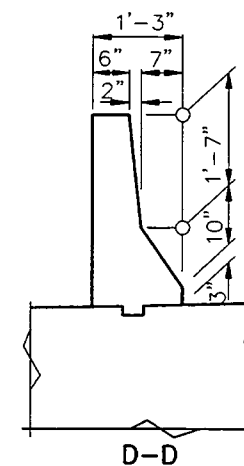
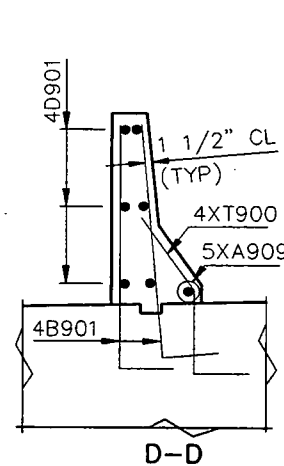
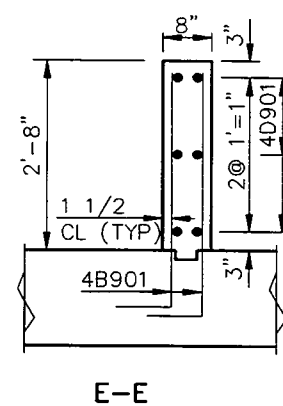
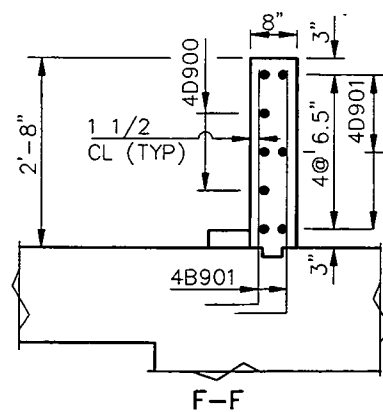
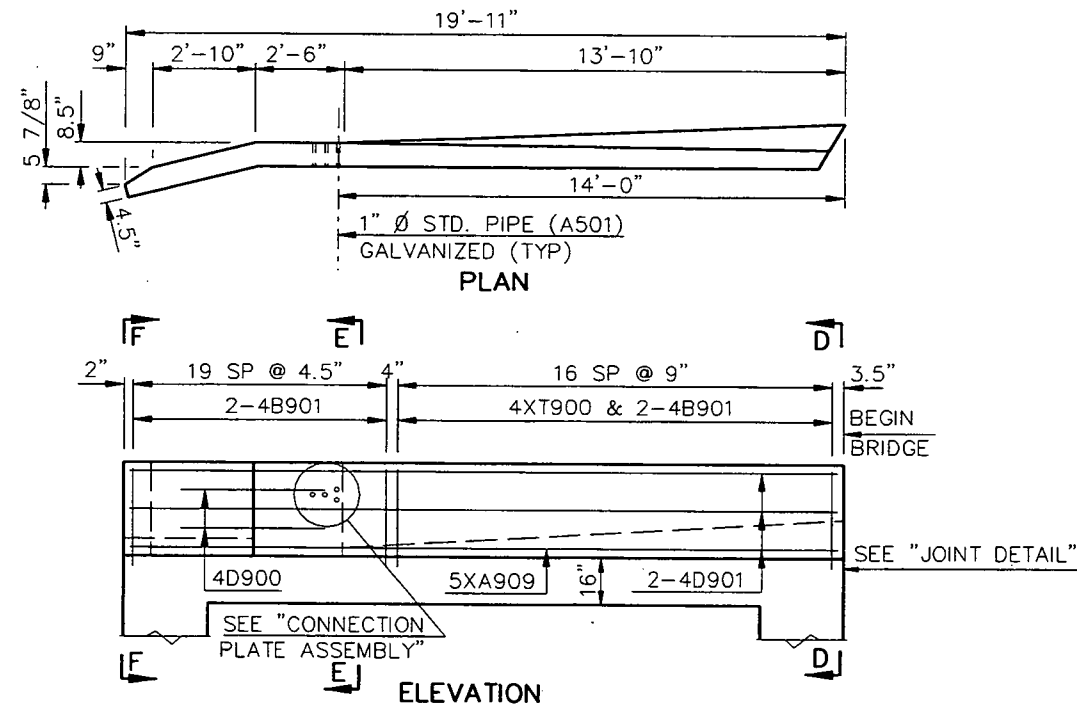
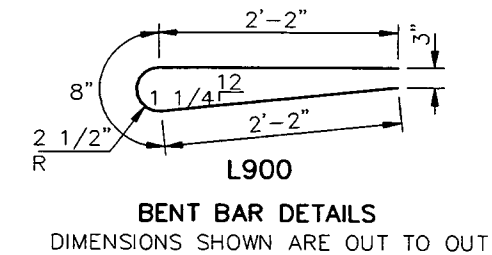
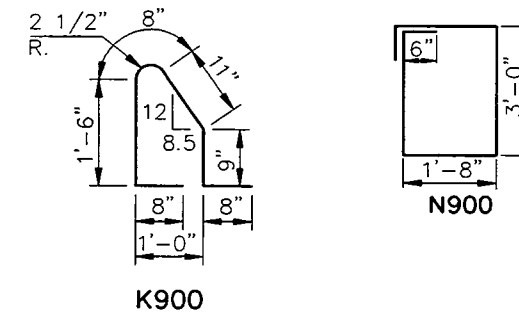
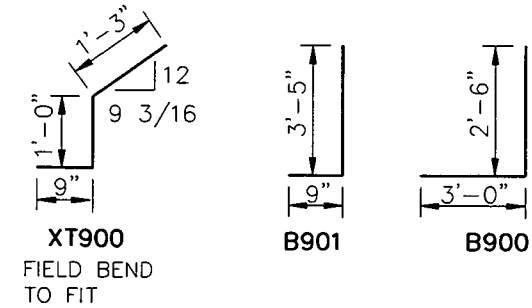
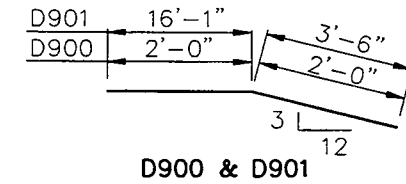
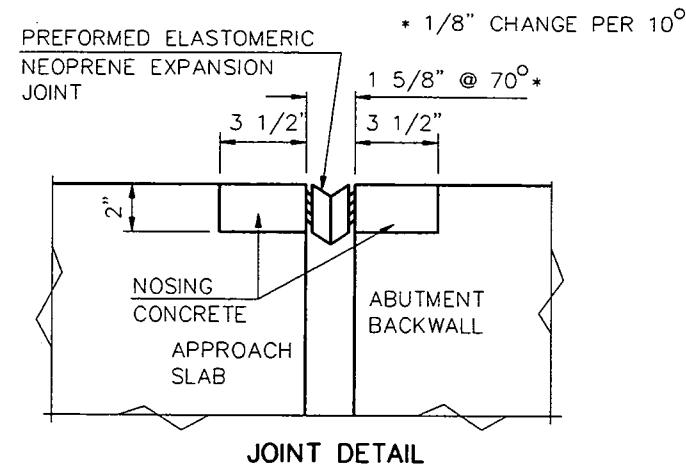
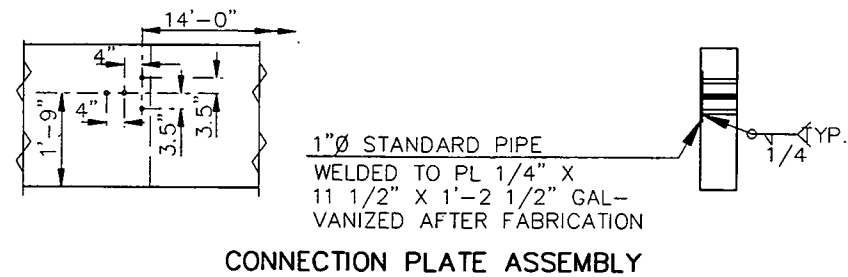


QUANTITIES

SEE DWG. NO. 2-053.731R-35

BURLINGTON NORTHERN OVERHEAD
RAY

APPROACH SLAB
BEGIN BRIDGE



WIDTH = 30'-0" CL. RDWY.

SKUE ANGLE = 30°

BAR LIST - ONE SLAB

SIZE	MARK	NO.	LENGTH
8	A903	77	19'-7"
5	A904	45	19'-7"
9	A906	6	19'-7"
5	A907	5	19'-7"
5	XA908	1	19'-7"
5	XA909	1	13'-10"
7	A911	12	47'-3"
5	A912	40	47'-3"
5	B900	180	5'-6"
4	B901	74	4'-2"
4	D900	2	4'-0"
4	D901	6	19'-7"
5	XK900	22	5'-2"
5	L900	22	5'-0"
4	N900	72	10'-4"
5	XT900	17	3'-0"

ESTIMATED MATERIAL QUANTITIES

REINFORCING STEEL LBS.	CONCRETE C.Y.
10,718	59.2

X = EPOXY COATED BAR

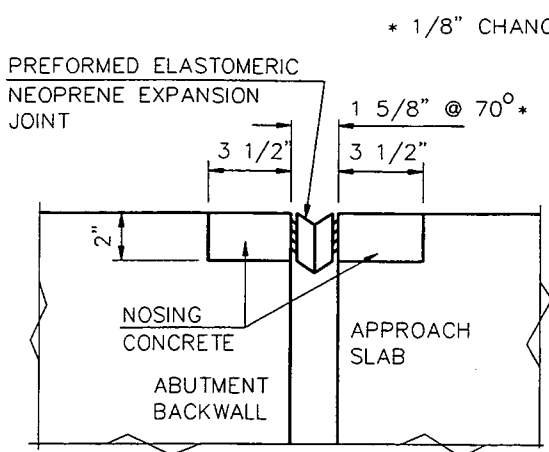
NOTES:
THE ESTIMATED MATERIAL QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. ALL MATERIALS, INCLUDING CONCRETE, REINFORCING STEEL AND LABOR REQUIRED TO BUILD THE APPROACH SLAB SHALL BE INCLUDED IN THE PAY ITEM "CONCRETE BRIDGE APPROACH SLAB". THE PILING, NOSING CONCRETE AND PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT ARE SEPARATE PAY ITEMS. THE PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT SHALL CONTINUE UP THE FRONT FACE OF THE BARRIERS FOR A DISTANCE OF 6" AND TERMINATE, EXCEPT ON THE SIDEWALK SIDE WHERE IT WILL TERMINATE AT THE EDGE OF THE SIDEWALK.

QUANTITIES		
APPROACH SLAB	91.3	SY
NOSING CONCRETE	47.6	LF
PREF. ELAST. NEOP. EXP. JT.	46.7	SY

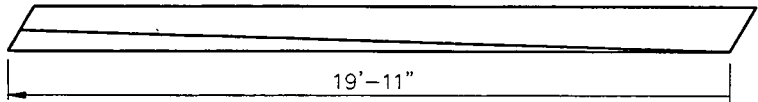
BURLINGTON NORTHERN OVERHEAD
RAY

APPROACH SLAB
BEGIN BRIDGE

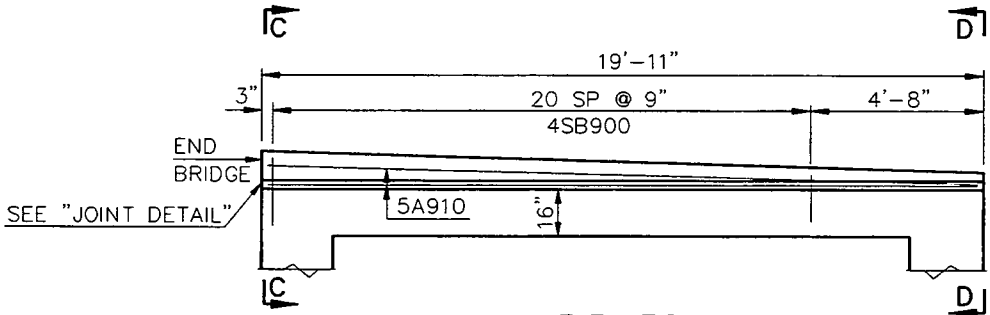
FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	RSN-7-002(041)052	161



JOINT DETAIL

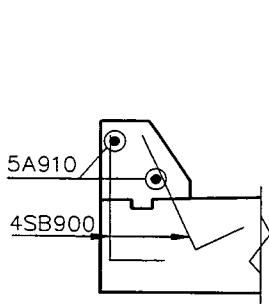


PLAN

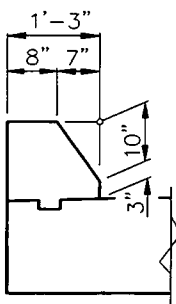


ELEVATION

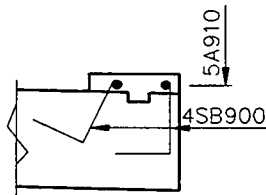
(LEFT CURB SHOWN-RIGHT CURB SIMILAR)



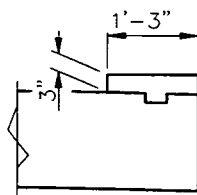
C-C



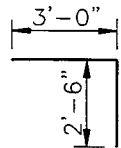
C-C



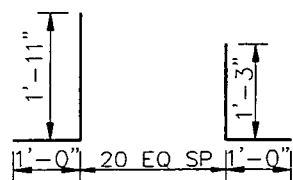
D-D



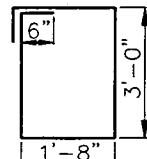
D-D



B900



SB900



N900

BENT BAR DETAILS
DIMENSIONS SHOWN ARE OUT TO OUT

WIDTH = 30'-0" CL. RDWY.
SKEW ANGLE = 30°

BAR LIST - ONE SLAB

SIZE	MARK	NO.	LENGTH
8	A903	77	19'-7"
5	A904	45	19'-7"
9	A906	6	19'-7"
5	A910	4	19'-7"
7	A911	12	47'-3"
5	A912	40	47'-3"
5	B900	180	5'-6"
4	SB900	4 SETS	54'-3"
4	N900	72	10'-4"

ESTIMATED MATERIAL QUANTITIES

REINFORCING STEEL LBS.	CONCRETE C.Y.
10,231	57.4

X = EPOXY COATED BAR

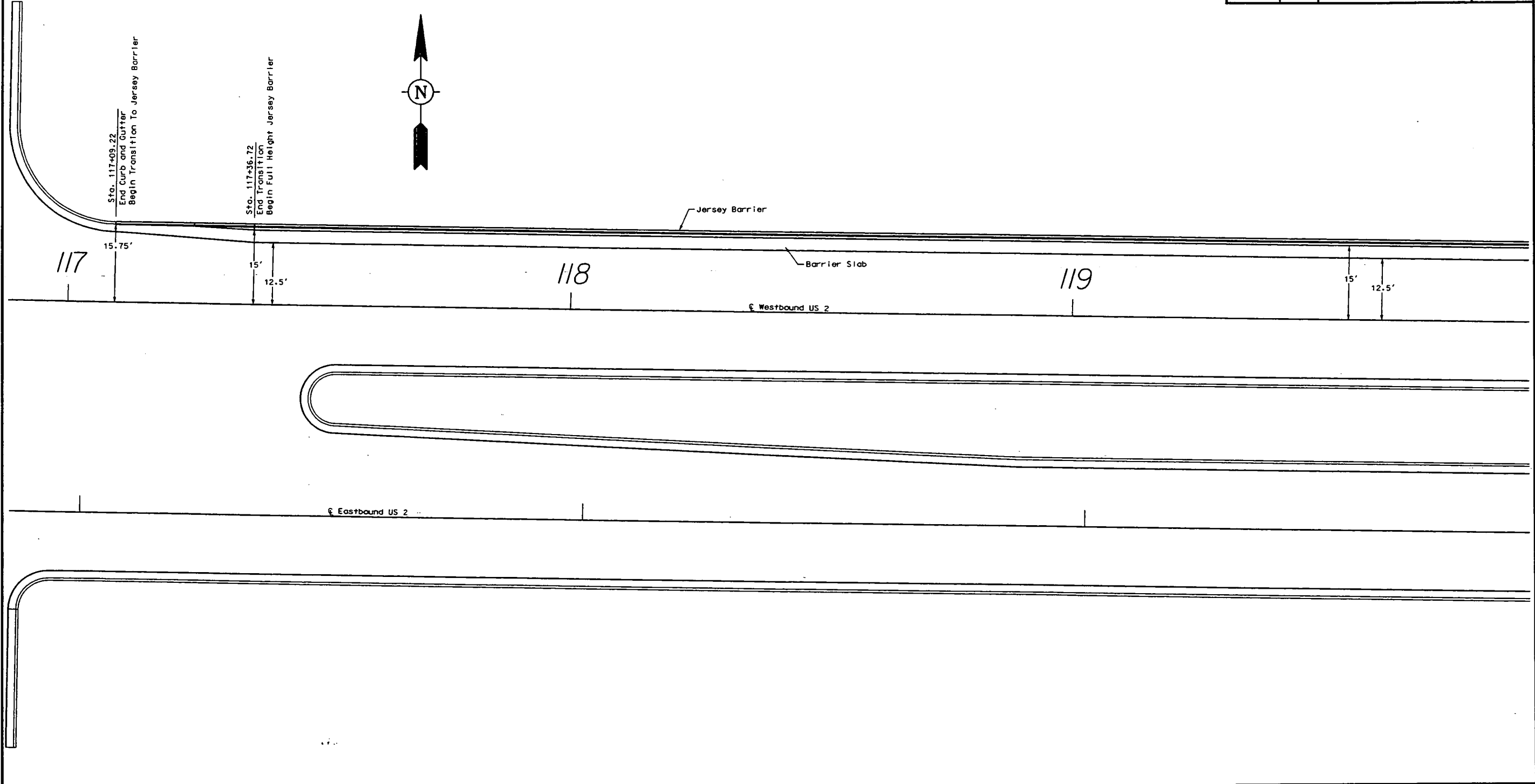
NOTES:
THE ESTIMATED MATERIAL QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. ALL MATERIALS, INCLUDING CONCRETE, REINFORCING STEEL AND LABOR REQUIRED TO BUILD THE APPROACH SLAB SHALL BE INCLUDED IN THE PAY ITEM "CONCRETE BRIDGE APPROACH SLAB". THE PILING, NOSING CONCRETE AND PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT ARE SEPARATE BID ITEMS.
THE PREFORMED ELASTOMERIC NEOPRENE EXPANSION JOINT SHALL CONTINUE UP THE FRONT FACE OF THE BARRIERS FOR A DISTANCE OF 6" AND TERMINATE, EXCEPT ON THE SIDEWALK SIDE WHERE IT WILL TERMINATE AT THE EDGE OF THE SIDEWALK.

QUANTITIES		
APPROACH SLAB	91.3	SY
NOSING CONCRETE	47.6	LF
PREF. ELAST. NEO. EXP. JT.	46.7	LF

BURLINGTON NORTHERN OVERHEAD
RAY

APPROACH SLAB
END BRIDGE

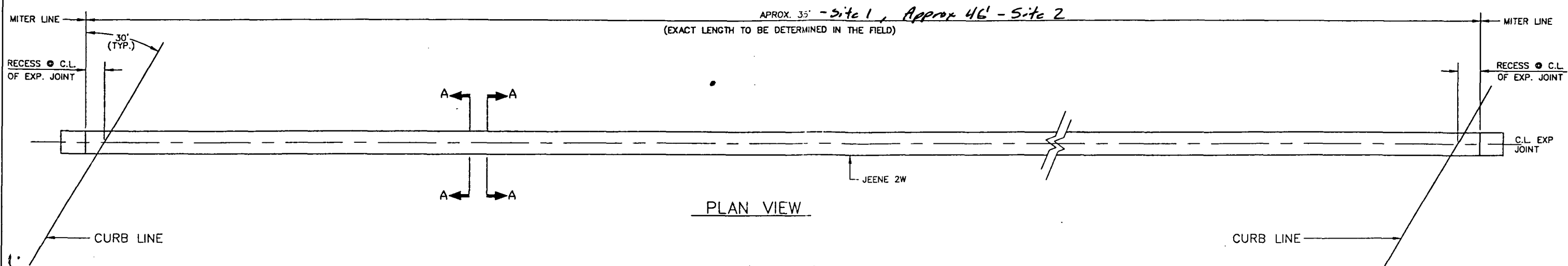
FHWA REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	RSN-7-002(041)052	162



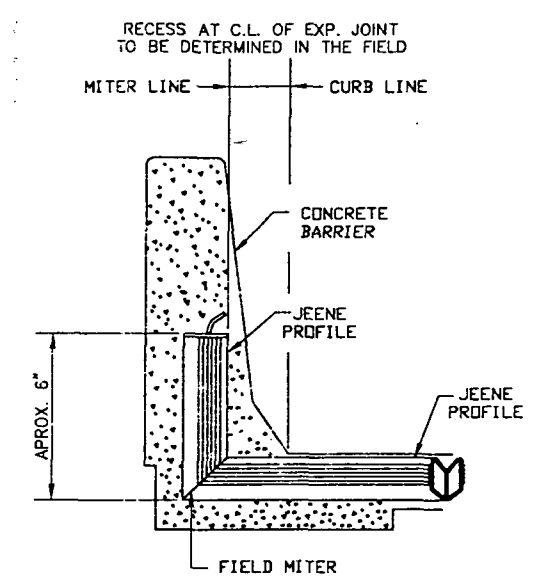
Jersey Barrier Layout

West of BN Railroad
Separation

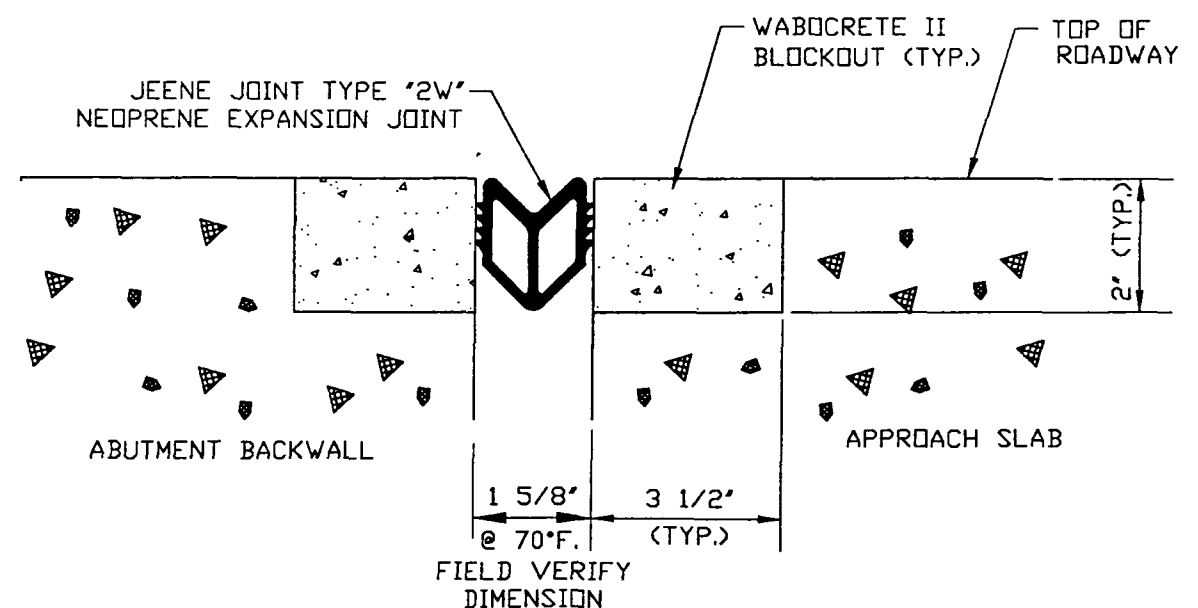
US 2
Ray, ND



PLAN VIEW



SECTION VIEW
SPLIT BARRIER



SECTION VIEW A - A

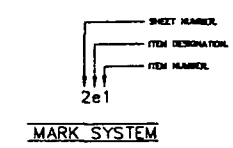
GENERAL NOTES:

1. WABOCRETE II SHALL BE INSTALLED IN ACCORDANCE WITH WATSON BOWMAN ACME INSTALLATION PROCEDURES.
2. JEENE PROFILE SHALL BE INSTALLED IN ACCORDANCE WITH WATSON BOWMAN ACME INSTALLATION PROCEDURES.

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING

8/26/97 DLR
 DATE BY

TEMPERATURE ADJUSTMENT
 THE CHANGE PER 10°F. IS MEASURED PERPENDICULAR TO THE JOINT FOR BOTH NORMAL AND SKEVED JOINTS.
 DIMENSIONS SPANNING THE OPEN JOINT WILL INCREASE OR INCREASE FOR EVERY 10°F. RISE OR FALL RESPECTIVELY.
 1/8" INCHES AT STA 122 + 80.80
 1/8" INCHES AT STA 126 + 6.31



STATE: NORTH DAKOTA
 COUNTY: WILLIAMS
 PROJECT NO.: RSN-NH-7-002(041)052
 REGION: 8 ROUTE: US 2

NO.	DESCRIPTION	NAME	DATE

Watson Bowman Acme

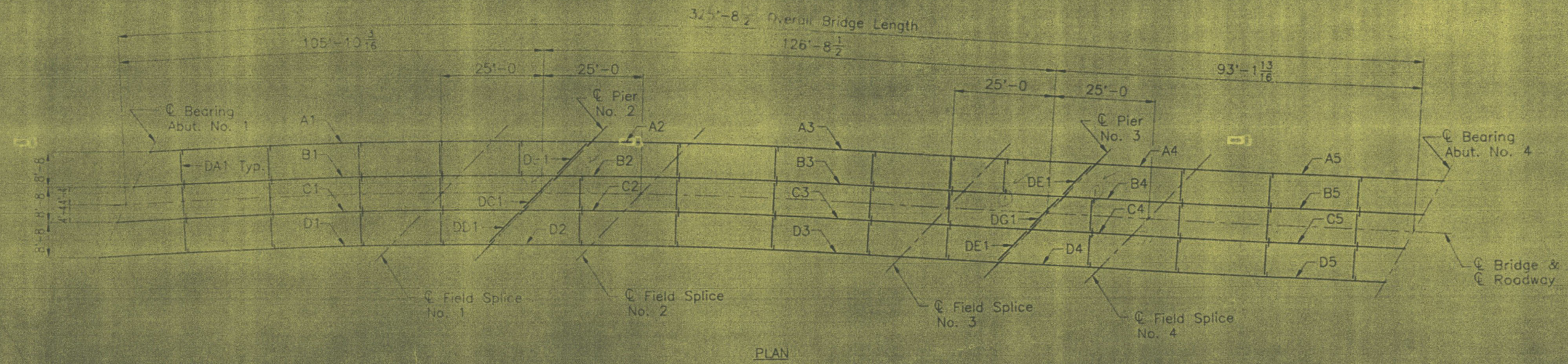
90 PINEVIEW DRIVE, ARDRETT, N.D. 58208 TEL (714) 891-7288 FAX (714) 891-9238

A DIVISION OF **HARRIS SPECIALTY CHEMICALS, INC.**

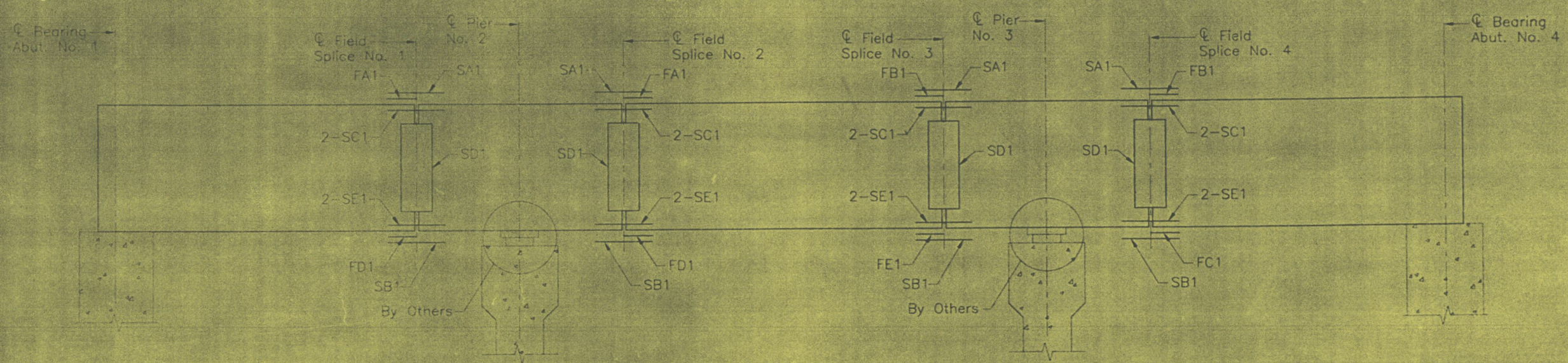
PROJECT: US 2 BURLINGTON NORTHERN RAILROAD SEPARATION IN RAY
 JEENE 2W EXPANSION JOINT DETAILS

DETAILED BY MN	DATE 8/15/97
CHECKED BY AML	DATE 8/15/97
SCALE N.T.S.	SHEET NO. 48289
SHEET NO. 1 OF 1	DRAWING NO. B-17076

#2-053.731 L & R



PLAN

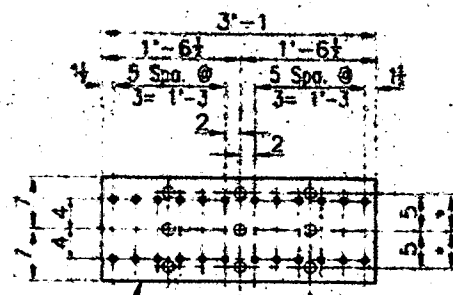


STRUCTURAL STEEL ERECTION LAYOUT

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
June 4, 1997
DATE BY DLR

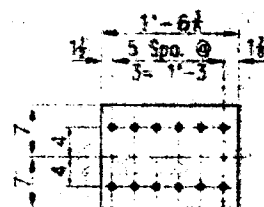
CONTRACT NO.
DESCRIPTION
SUBSTRUCTURAL STEEL ERECTION LAYOUT

CAPITAL CONTRACTORS, INC. CAPITAL STEEL DIVISION, LINCOLN, NE				
1001 N. 9TH STREET, BOX 80080, ZIP 68501, PH. 402-476-1081				
APPROVAL DATE	CONTRACTOR	INDUSTRIAL BUILDERS		
NO. H. 10 OR NOTED	STRUCTURE	3-SPAN W.P.C. BRIDGE (SITE 1)		
DESIGN NO.	LOCATION	BURLINGTON NORTHERN OVERHEAD RAY		
FIELDS	AS NOTED	CUSTOMER	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION	
PAINT	SEE NOTES	PROJECT	7-002(DA1)052	COUNTY WILLIAMS
DRAWN	5-5-97	STATION	124 + 43.6	ROADWAY 20 FT.
CHECKED	5-5-97	LOADING	H225	SKW VARIES
BRIDGE NO.	2-053.251L	SHEET NO.	E1 OF 1	ORDER NO. 4213W

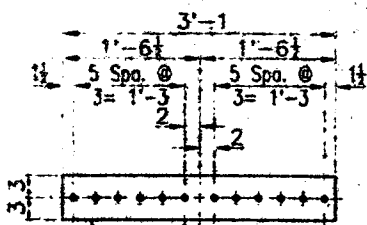


● $\frac{1}{2}$ x 14 x 3'-1 SA1
16-SPLICE PLATES: SA1

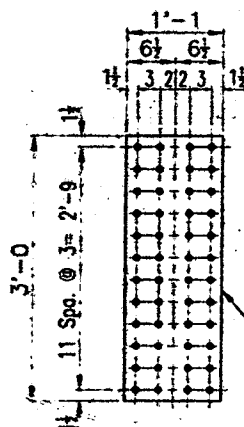
* Space studs to clear bolt holes.



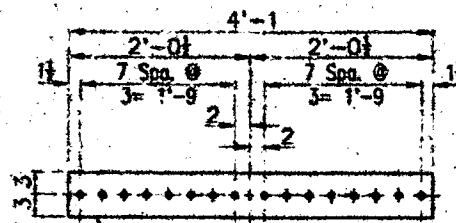
● $\frac{1}{2}$ x 14 x 1'-6 $\frac{1}{2}$ FA1
● $\frac{1}{2}$ x 14 x 1'-6 $\frac{1}{2}$ FB1
16-FILL PLATES: 8-FA1, 8-FB1



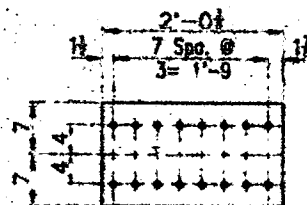
● $\frac{1}{2}$ x 6 x 3'-1 SC1
32-SPLICE PLATES: SC1



32-SPLICE PLATES: SD1

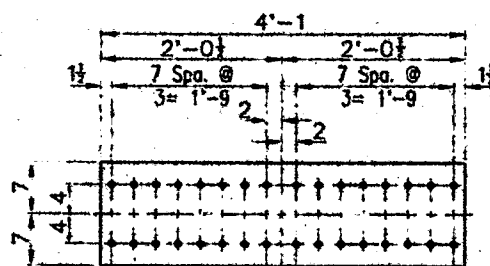


● $\frac{3}{4}$ x 6 x 4'-1 SE1
32-SPLICE PLATES: SE1



● $\frac{1}{2}$ x 14 x 2'-0 $\frac{1}{2}$ FC1
● $\frac{1}{2}$ x 14 x 2'-0 $\frac{1}{2}$ FD1
● $\frac{1}{2}$ x 14 x 2'-0 $\frac{1}{2}$ FE1

16-FILL PLATES: 4-FC1, 8-FD1, 4-FE1



● $\frac{5}{8}$ x 14 x 4'-1 SF1
16-SPLICE PLATES: SB1

SHOP BILL OF MATERIALS									
QTY	NO. PLATES	SHAPE	DIMENSIONS		MARK	REMARKS	CALCULATED	WEIGHT	EACH
			FEET	INCHES					
1	12	8	SPLICE PLATES		16-SAL, 16-SBT, 12-SC1, 12-SD1, 32-SE1				
2	1	6	P x 14		3	1	SA1	M270 G. 502	
3	1	6	P x 14		4	1	SB1	M270 G. 502	
4	3	2	P x 6		3	1	SC1	M270 G. 502	
5	3	2	P x 13		3	0	SD1	M270 G. 502	
6	3	2	P x 6		4	1	SE1	M270 G. 502	
7	1	4	SHEAR STUD		4	5	ma1		
8	3	2	FILL PLATES		8-FA1, 8-FB1, 4-FD1, 4-FE1				
9	8	P x 14		1	6	1	FA1	A709 G. 36	
10	8	P x 14		1	6	1	FB1	A709 G. 36	
11	4	P x 14		2	0	1	FC1	A709 G. 36	
12	8	P x 14		2	0	1	FD1	A709 G. 36	
13	4	P x 14		2	0	1	FE1	A709 G. 36	
14	4	5	DIAPHRAGMS		1-DB1, 1-DC1, 1-DD1, 2-DE1, 1-DG1				
15	3	9	P x 42		8	5	DA1	A709 G. 36	
16	1	1	P x 42		1	1	DB1	A709 G. 36	
17	1	1	P x 42		1	1	DC1	A709 G. 36	
18	1	1	P x 42		1	1	DD1	A709 G. 36	
19	2	1	P x 42		1	1	DE1	A709 G. 36	
20	1	1	P x 42		1	1	DG1	A709 G. 36	

NOTES

All Steel marked thus Δ shall be A.S.T.M. A709 Grade 50. Steel marked thus $\bullet$ shall conform to the requirements of A.A.S.H.T.O. M270 Grade 50T2 and shall meet the longitudinal Charpy V-notch test of 15 ft. lbs. @ 40° F. All other steel shall be ASTM A709 Grade 36.

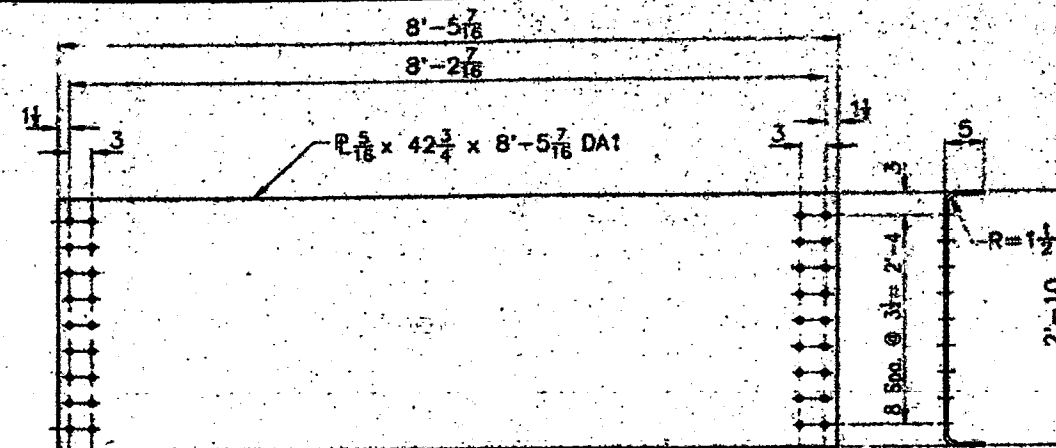
Web plates shall be cut to camber using the "Automatic Torch" with a mechanical guide. See shop drawing details.

Flange plate to web plate welds shall be by the "Automatic Submerged Arc Welder". All other welds shall be by the "Submerged Arc Process," "Flux-Cored Arc Process," or by the Shielded Metal Arc Process, using E7018, or E7028 electrodes.

All Welding shall be done in accordance with the Standard Specifications and shall conform to the American Welding Society and A.A.S.H.T.O. joint publication D1.5-95 and approved Capital Steel Welding procedures.

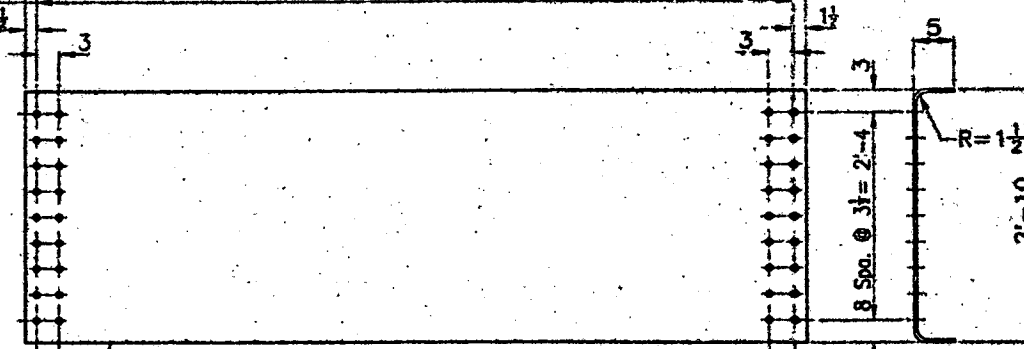
All surfaces to be painted shall be blast-cleaned in accordance with Steel Structures Painting Council Specification SSPC-SP10. Apply one coat of Inorganic Zinc-Silicate primer with a dry film thickness a minimum of 3.0 mils and a finish coat of Aliphatic Polyurethane (See Specifications).

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE BY DLR



39-DIAPHRAGMS: DA1

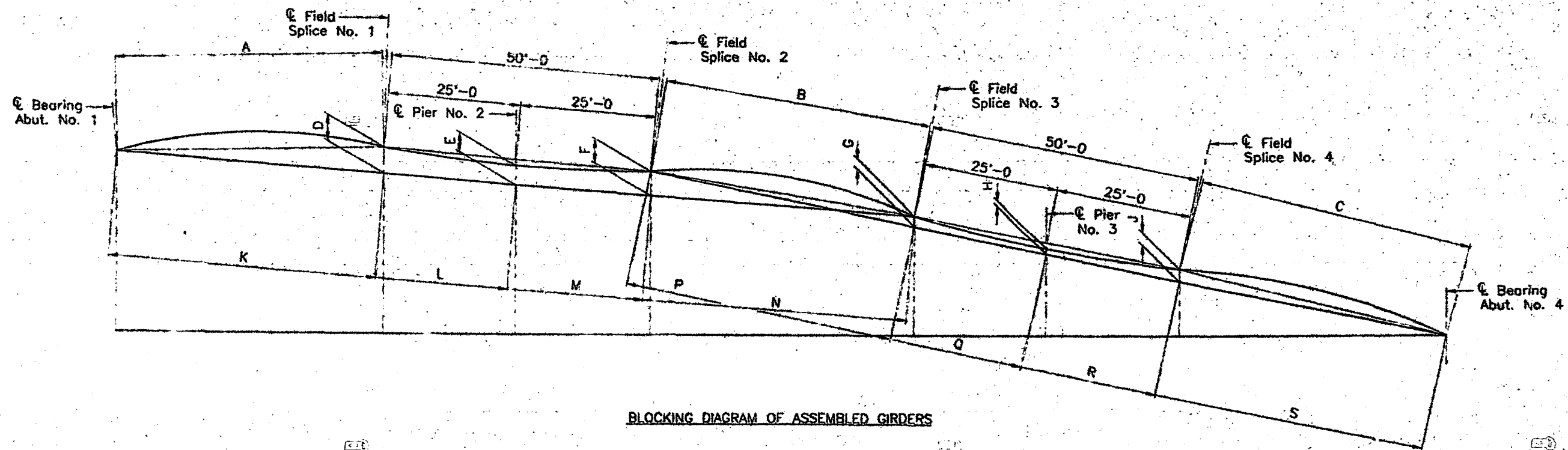
DG1	11'-0 $\frac{15}{16}$
DG1	10'-9 $\frac{15}{16}$
DE1	11'-0 $\frac{7}{8}$
DE1	10'-9 $\frac{7}{8}$
DD1	11'-7 $\frac{11}{16}$
DD1	11'-4 $\frac{11}{16}$
DC1	11'-6 $\frac{13}{16}$
DC1	11'-3 $\frac{13}{16}$
DB1	11'-6
DB1	11'-3



6-DIAPHRAGMS: 1-DB1, 1-DC1,
1-DD1, 2-DE1, 1-DG1

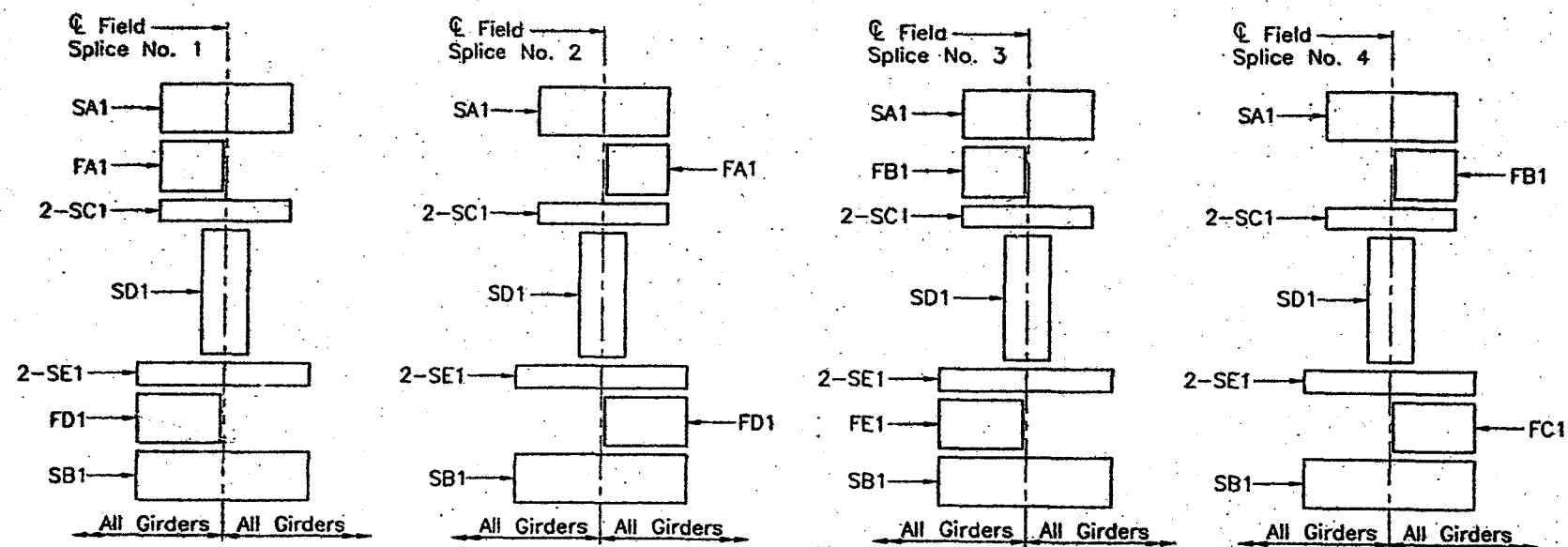
● $\frac{5}{16}$ x 42 $\frac{3}{4}$ x 11'-6 DB1
● $\frac{5}{16}$ x 42 $\frac{3}{4}$ x 11'-6 $\frac{13}{16}$ DC1
● $\frac{5}{16}$ x 42 $\frac{3}{4}$ x 11'-7 $\frac{11}{16}$ DD1
● $\frac{5}{16}$ x 42 $\frac{3}{4}$ x 11'-0 $\frac{7}{8}$ DE1
● $\frac{5}{16}$ x 42 $\frac{3}{4}$ x 11'-0 $\frac{15}{16}$ DG1

CAPITAL CONTRACTORS, INC.
CAPITAL STEEL DIVISION, LINCOLN, NE
1001 N. 9TH STREET, BOX 80000, ZIP 68501, TEL 402-476-1000
APPROVAL DATE CONTRACTOR INDUSTRIAL BUILDERS
DESIGN NO. LOCATION
STRUCTURE 3-SPAN W.P.O. BRIDGE (SITE 1)
WELDS AS NOTED
PAINT SEE NOTES
PROJECT RSN-NH-7-002(04)1002 COUNTY WILLIAMS
DRAWN 5-30-97 STATION 124 + 43.6 ROADWAY 30 FT.
CHECKED 5-30-97 LOADING HS25 SKEW VARIOUS
BRIDGE NO. 2-053.731L SHEET NO. 1 OF 21 ORDER NO. 4213W



BLOCKING DIAGRAM OF ASSEMBLED GIRDERS

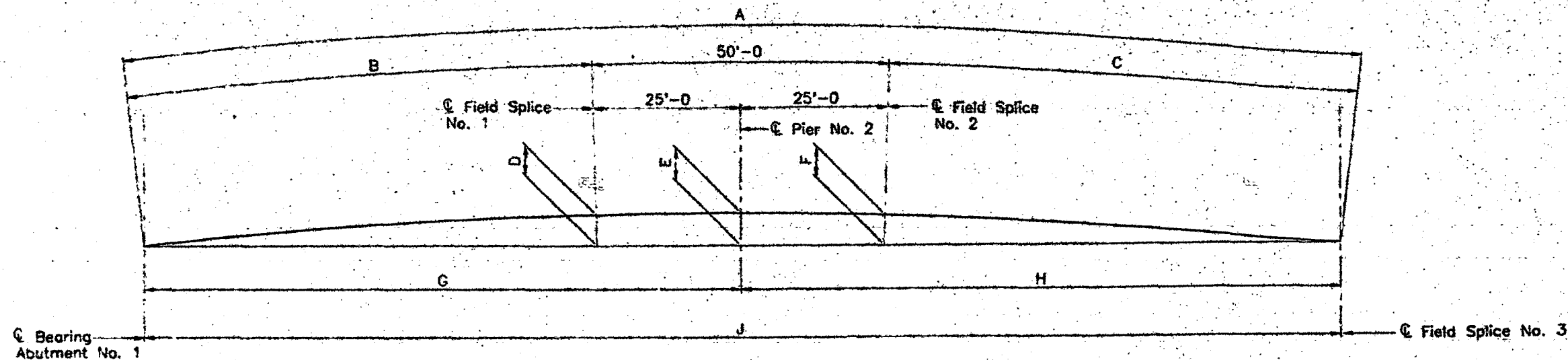
Girder Line	A	B	C	D	E	F	G	H	J	K	L	M	N	P	Q	R	S
Line A	85'-10 $\frac{1}{2}$	76'-1 $\frac{1}{2}$	63'-2 $\frac{1}{2}$	8 $\frac{1}{2}$	8 $\frac{1}{2}$	8 $\frac{1}{2}$	7 $\frac{1}{2}$	6 $\frac{1}{2}$	6 $\frac{1}{2}$	85'-9 $\frac{1}{2}$	110'-9 $\frac{1}{2}$	135'-9 $\frac{1}{2}$	211'-11 $\frac{1}{2}$	76'-1 $\frac{1}{2}$	101'-1 $\frac{1}{2}$	126'-1 $\frac{1}{2}$	189'-3 $\frac{1}{2}$
Line B	81'-9 $\frac{1}{2}$	76'-5 $\frac{1}{2}$	65'-8 $\frac{1}{2}$	8 $\frac{1}{2}$	8 $\frac{1}{2}$	8 $\frac{1}{2}$	7 $\frac{1}{2}$	7	7 $\frac{1}{2}$	81'-9 $\frac{1}{2}$	108'-9 $\frac{1}{2}$	131'-9 $\frac{1}{2}$	208'-3 $\frac{1}{2}$	76'-5 $\frac{1}{2}$	101'-5 $\frac{1}{2}$	126'-5 $\frac{1}{2}$	192'-2 $\frac{1}{2}$
Line C	77'-8	76'-11 $\frac{1}{2}$	68'-3	8 $\frac{1}{2}$	8 $\frac{1}{2}$	8 $\frac{1}{2}$	7 $\frac{1}{2}$	7 $\frac{1}{2}$	7 $\frac{1}{2}$	77'-7 $\frac{1}{2}$	102'-7 $\frac{1}{2}$	127'-7 $\frac{1}{2}$	204'-7 $\frac{1}{2}$	76'-11 $\frac{1}{2}$	101'-11 $\frac{1}{2}$	126'-11 $\frac{1}{2}$	195'-2 $\frac{1}{2}$
Line D	73'-6 $\frac{1}{2}$	77'-5 $\frac{1}{2}$	70'-9	7 $\frac{1}{2}$	7 $\frac{1}{2}$	8 $\frac{1}{2}$	7 $\frac{1}{2}$	7 $\frac{1}{2}$	7 $\frac{1}{2}$	73'-5 $\frac{1}{2}$	98'-5 $\frac{1}{2}$	123'-6	200'-11 $\frac{1}{2}$	77'-5 $\frac{1}{2}$	102'-5 $\frac{1}{2}$	127'-5 $\frac{1}{2}$	198'-2 $\frac{1}{2}$



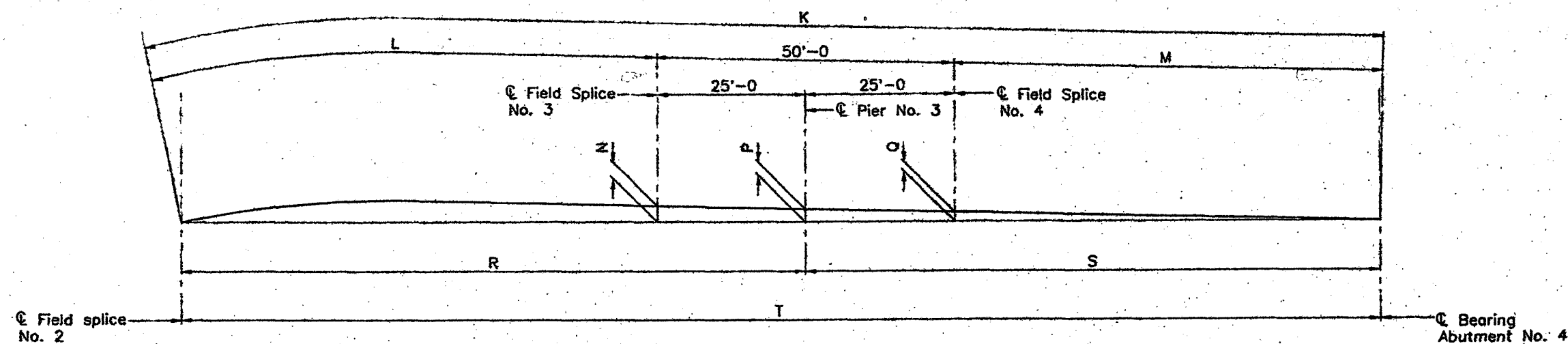
MATCH MARK ILLUSTRATION

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE BY DLR

CAPITAL CONTRACTORS, INC. CAPITAL STEEL DIVISION, LINCOLN, NE			
1001 N. 22ND STREET, BOX 80080, ZIP 68501, TEL 402-478-1081			
APPROVAL DATE	CONTRACTOR	INDUSTRIAL BUILDERS	
30 E. 10 N. 10 E.	STRUCTURE	3-SPAN W.P.O. BRIDGE (SITE 1)	
DESIGN NO.	LOCATION	BURLINGTON NORTHERN OVERHEAD RAY	
VELAS	AS NOTED	CUSTOMER	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
PAINT	SEE NOTES	PROJECT	201-11-1-002(04)1053
DRAWN	5-5-97	STATION	124 + 43.6
CHECKED	5.9.97	LOADING	HS25
BRIDGE NO.	2-053.731L	SHEET NO.	2 OF 21
DESCRIPTION		BLOCKING & MATCH MARK ILLUSTRATION	
		ORDER NO.	42134



HORIZONTAL CURVATURE DETAIL OF ASSEMBLED GIRDERS (SITE 1)



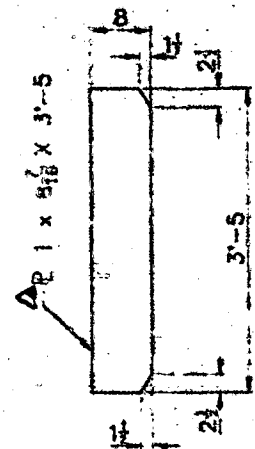
HORIZONTAL CURVATURE DETAIL OF ASSEMBLED GIRDERS (SITE 1)

Girder Line	A	B	C	D	E	F	G	H	J	K	L	M	N	P	Q	R	S	T
Line A	211'-11 $\frac{1}{8}$	85'-10 $\frac{1}{8}$	76'-1	3'-8 $\frac{3}{8}$	3'-10 $\frac{1}{8}$	3'-8 $\frac{3}{8}$	110'-8 $\frac{3}{8}$	100'-11 $\frac{1}{8}$	211'-8 $\frac{1}{2}$	189'-3 $\frac{3}{8}$	76'-1 $\frac{1}{8}$	63'-2 $\frac{1}{2}$	6 $\frac{1}{2}$	4 $\frac{1}{8}$	3 $\frac{1}{8}$	101'-4 $\frac{1}{8}$	84'-9 $\frac{1}{2}$	186'-2 $\frac{1}{2}$
Line B	208'-3 $\frac{1}{8}$	81'-9 $\frac{3}{8}$	76'-5 $\frac{1}{2}$	3'-6 $\frac{1}{2}$	3'-8 $\frac{1}{2}$	3'-5 $\frac{1}{2}$	106'-8 $\frac{1}{2}$	101'-4 $\frac{1}{2}$	208'-1 $\frac{1}{8}$	192'-2 $\frac{1}{2}$	76'-3 $\frac{1}{2}$	65'-8 $\frac{1}{2}$	8 $\frac{1}{2}$	6 $\frac{1}{8}$	4 $\frac{1}{8}$	101'-7 $\frac{1}{2}$	89'-9 $\frac{3}{8}$	191'-5 $\frac{1}{2}$
Line C	204'-7 $\frac{1}{2}$	77'-8	76'-11 $\frac{1}{2}$	3'-5 $\frac{1}{8}$	3'-7 $\frac{1}{2}$	3'-4 $\frac{1}{8}$	102'-8 $\frac{1}{8}$	101'-10 $\frac{1}{8}$	204'-5 $\frac{3}{8}$	195'-2 $\frac{1}{2}$	76'-11 $\frac{1}{2}$	68'-3 $\frac{1}{2}$	11 $\frac{1}{8}$	8 $\frac{1}{8}$	6 $\frac{1}{2}$	101'-10 $\frac{1}{2}$	94'-9 $\frac{3}{8}$	198'-8 $\frac{1}{2}$
Line D	200'-11 $\frac{1}{8}$	73'-6 $\frac{1}{8}$	77'-5 $\frac{1}{2}$	3'-3 $\frac{1}{2}$	3'-6 $\frac{1}{8}$	3'-4 $\frac{1}{2}$	98'-5 $\frac{1}{8}$	102'-4 $\frac{1}{2}$	200'-9 $\frac{3}{8}$	198'-2 $\frac{1}{2}$	77'-5 $\frac{1}{2}$	70'-9 $\frac{1}{2}$	1'-2 $\frac{1}{2}$	11 $\frac{1}{2}$	8 $\frac{3}{8}$	102'-2 $\frac{1}{8}$	99'-10	202'-0 $\frac{1}{8}$

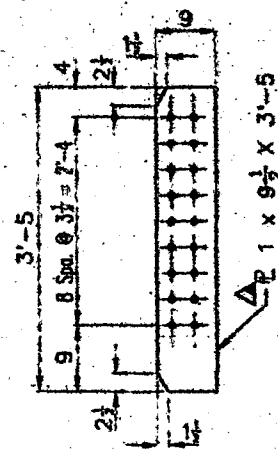
NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE BY DLR

CONTRACT NO.
DESCRIPTION
HORIZONTAL CURVATURE DETAIL
OF ASSEMBLED GIRDERS (SITE 1)

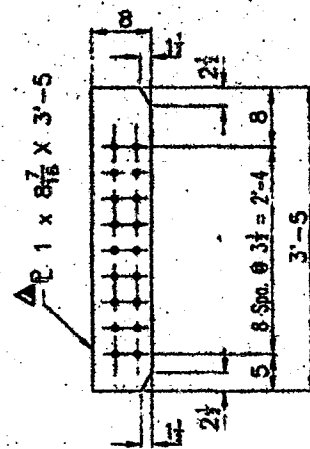
CAPITAL CONTRACTORS, INC. CAPITAL STEEL DIVISION, LINCOLN, NE 1001 N. 8TH STREET, BOX 80386, ZIP 68501, PH. 402-476-1021			
APPROVAL DATE	CONTRACTOR	INDUSTRIAL BUILDERS	
DESIGN NO.	STRUCTURE	3-SPAN W.P.C. BRIDGE (SITE 1)	
LOCATION	BURLINGTON NORTH-DEM OVERHEAD RY		
WELDER	AS NOTED	CUSTOMER	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
PAINT	SEE NOTES	PROJECT	NSA-04-7-002(041)053
DRAWN	5-13-97 G. CLARK	STATION	124 + 43.6
CHECKED	5-12-97	LOADING	HESS
BRIDGE NO.	2-053.7311	SHEET NO.	3 OF 21
		ORDER NO.	42134



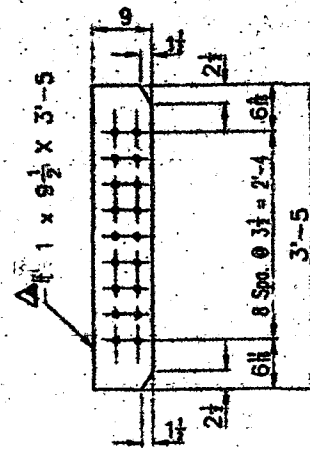
2-STIFFENERS: KA4



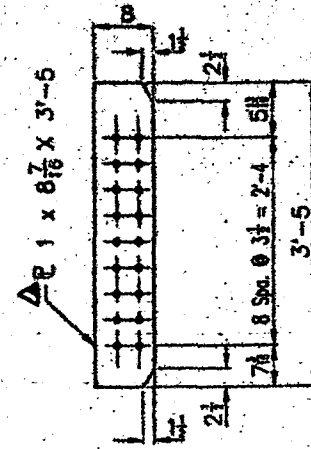
1-STIFFENER: KB4



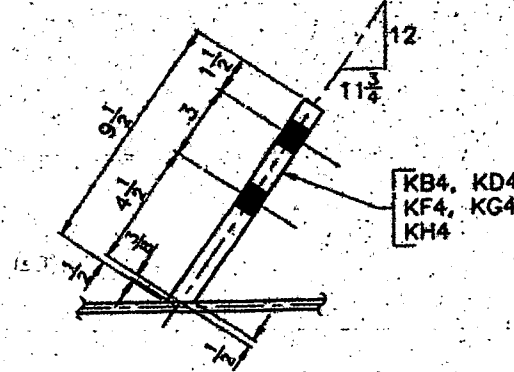
4-STIFFENERS: KC4



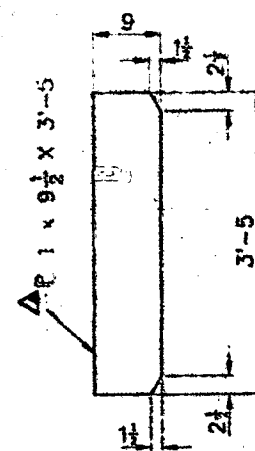
1-STIFFENER: KD4



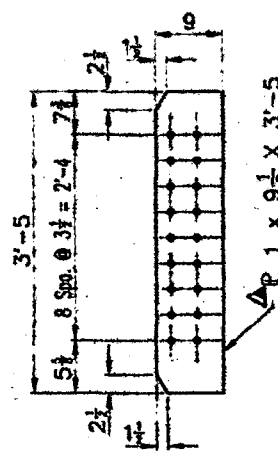
1-STIFFENER: KE4



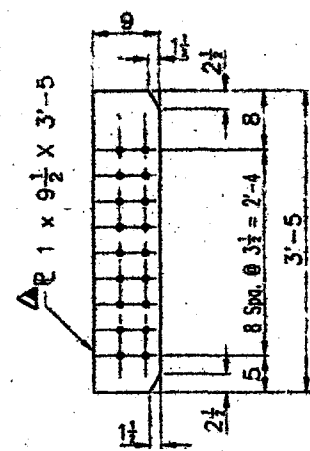
SKewed STIFFENER DETAIL



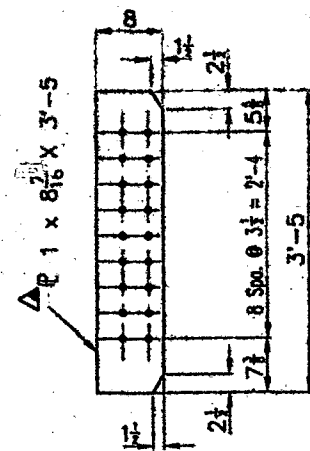
2-STIFFENERS: KF4



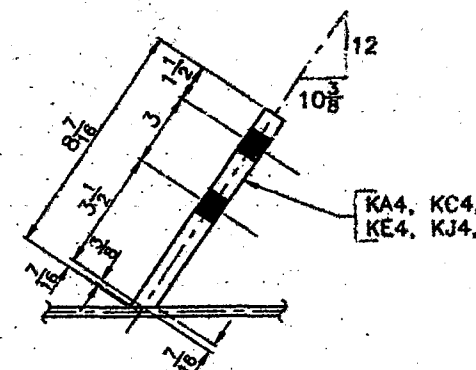
1-STIFFENER: KG4



3-STIFFENERS: KH4

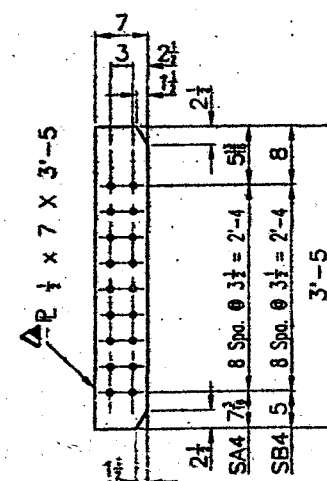


1-STIFFENER: KJ4



SKewed STIFFENER DETAIL

SHOP BILL OF MATERIALS									
LINE NO.	NO. OF PIECES	SHAPE	LENGTH	ADD.	MARK	REMARKS	QUANTITY	WEIGHT	REMARKS
1	9	4 STIFFENERS	2 SA, 1 SB, 3 SC, 1 SD, 1 SE, 2 SF						
2	1	1 SA	4 SA						
3	2	1 x 8 7/8	3	5		KA4	1709	0.50	
4	1	1 x 8 7/8	3	5		KE4	1709	0.50	
5	4	1 x 8 7/8	3	5		KC4	1709	0.50	
6	1	1 x 8 7/8	3	5		KJ4	1709	0.50	
7	1	1 x 9 1/2	3	5		KB4	1709	0.50	
8	2	1 x 9 1/2	3	5		KF4	1709	0.50	
9	1	1 x 9 1/2	3	5		KG4	1709	0.50	
10	3	1 x 9 1/2	3	5		KH4	1709	0.50	
11	1	1 x 9 1/2	3	5		KD4	1709	0.50	
12	2	6 x 7	3	5		SA4	1709	0.50	
13	5	2 x 7	3	5		SB4	1709	0.50	
14									
15									
16									
17									
18									
19									
20									

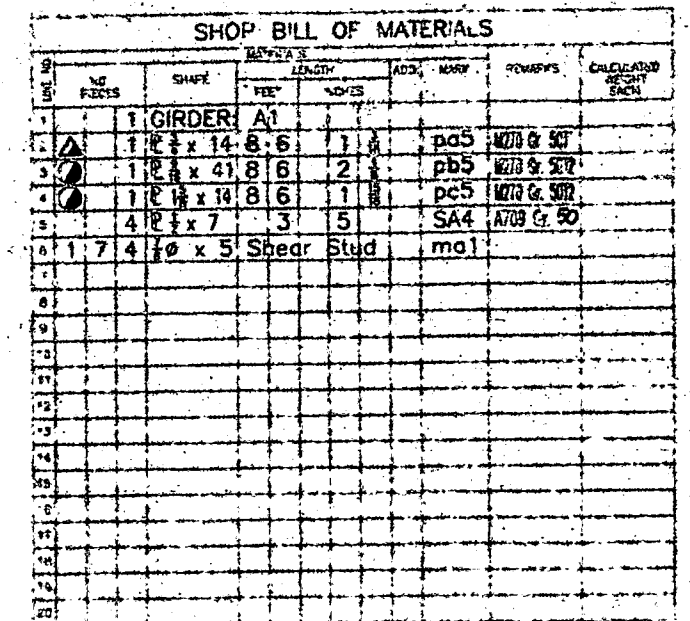


78-STIFFENERS: 26-SA4, 52-SB4 ~ 73 CWT

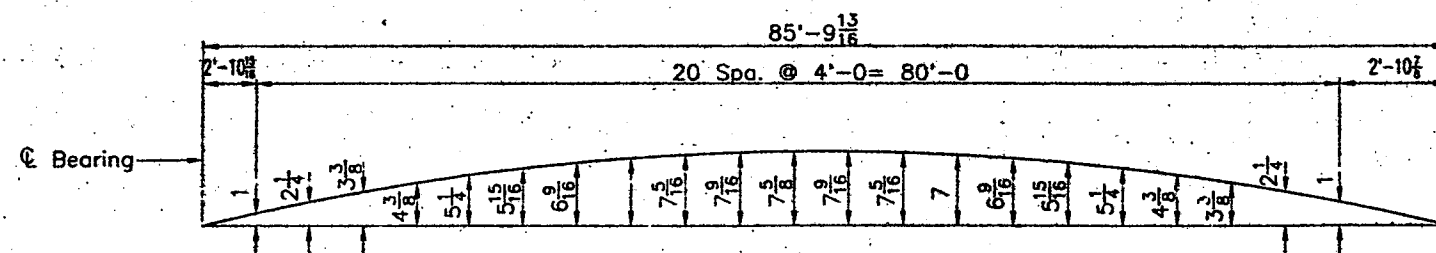
FIELD BOLT LIST						
NUMBER	DIA.	DESCRIPTION	LGTH.	CONNECTION	ADD.	SHIP
1,404	1/2	A325-1 Hvy. Hex. Hd.	2 1/2	Intermediate Stiffeners to Diaphragm	71	1,475
216	1/2	A325-1 Hvy. Hex. Hd.	2 1/2	Pier Stiffeners to Diaphragm	14	230
192	1/2	A325-1 Hvy. Hex. Hd.	4 1/2	Top Flange Girder Field Splices No. 1 & 2	13	205
192	1/2	A325-1 Hvy. Hex. Hd.	3 1/2	Top Flange Girder Field Splices No. 3 & 4	13	205
768	1/2	A325-1 Hvy. Hex. Hd.	3	Girder Web Field Splice	42	810
256	1/2	A325-1 Hvy. Hex. Hd.	5	Bottom Flange Girder Field Splices No. 1 & 2	14	270
256	1/2	A325-1 Hvy. Hex. Hd.	4 1/2	Bottom Flange Girder Field Splices No. 3 & 4	14	270
3,284	1/2	A563-DH Hvy. Hex. Nut			181	3,465
3,284	1/2	F436-1 Structural Washers			181	3,465
Total Weight -						lbs.

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE BY DLR

CAPITAL CONTRACTORS, INC.			
CAPITAL STEEL DIVISION, LINCOLN, NE			
1001 N. 9TH STREET, BOX 80080, SEP 68001, TEL 402-478-1037			
APPROVAL DATE		CONTRACTOR	
INDUSTRIAL BUILDERS			
NO. 10	NOTED	STRUCTURE	
DESIGN NO.		3-SPAN W.P.D. BRIDGE (SITE 1)	
LOCATION		BURLINGTON NORTHERN OVERHEAD	
FIELD	AS NOTED	CUSTOMER	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
PAINT	SEE NOTES	PROJECT	RSN-01-7-002(041)002
DRAWN	5-28-87	STATUS	124 + 43.6
CHECKED	5-30-97	LOADING	HS25
BRIDGE NO.	2-053.731L	SHEET NO.	4 OF 21
		ORDER NO.	45138



FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPLICES
SEE SHEET 5.

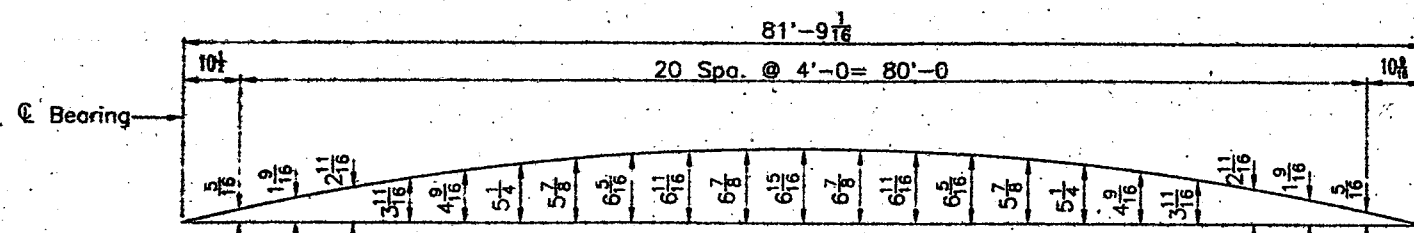


HORIZONTAL CURVE - GIRDER A1

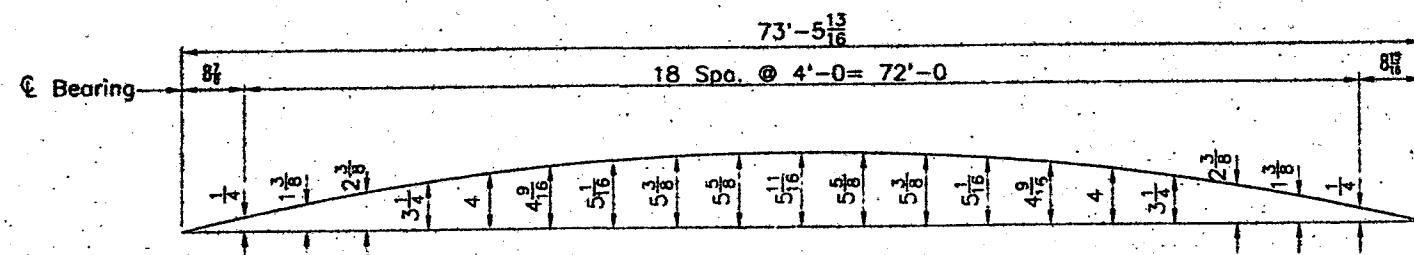
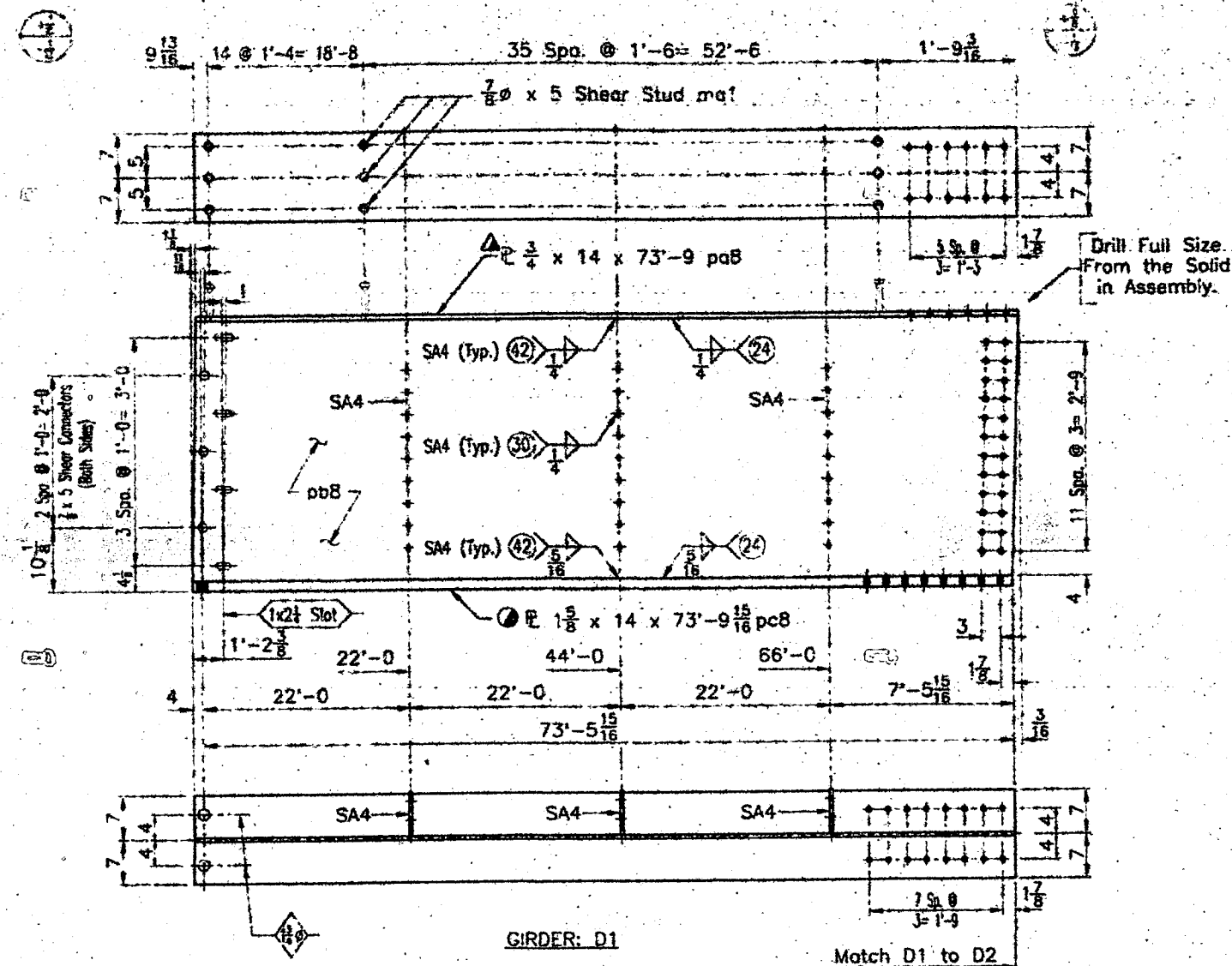
NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97 DATE BY DLR

CONTRACT NO	
DESCRIPTION	ORDER A1 DETAILS

CAPITAL CONTRACTORS, INC.					
CAPITAL STEEL DIVISION, LINCOLN, NE					
1001 N. 9TH STREET, BOX 80098, STP 68501, PH. 402-478-1081					
APPROVAL DATE		CONTRACTOR		INDUSTRIAL BUILDERS	
5-8 N OR NOTED		STRUCTURE		3-SPAN W.P.O. BRIDGE (SITE 1)	
DESIGN NO.		LOCATION		BURLINGTON NORTHERN OVERHEAD RAY	
WELDS		AS NOTED		CUSTOMER	
PAINT		SEE NOTES		NORTH DAKOTA DEPARTMENT OF TRANSPORTATION	
DRAWN		5-29-87 C. CLARK		PROJECT RD 7-002 (A41)052	
STATION		124 + 43.6		COUNTY WILLIAMS	
CHECKED		5-30-87		LOADING	
BRIDGE NO.		5-053.734		SHEET NO. 6 OF 21	
				ORDER NO. 4813W	

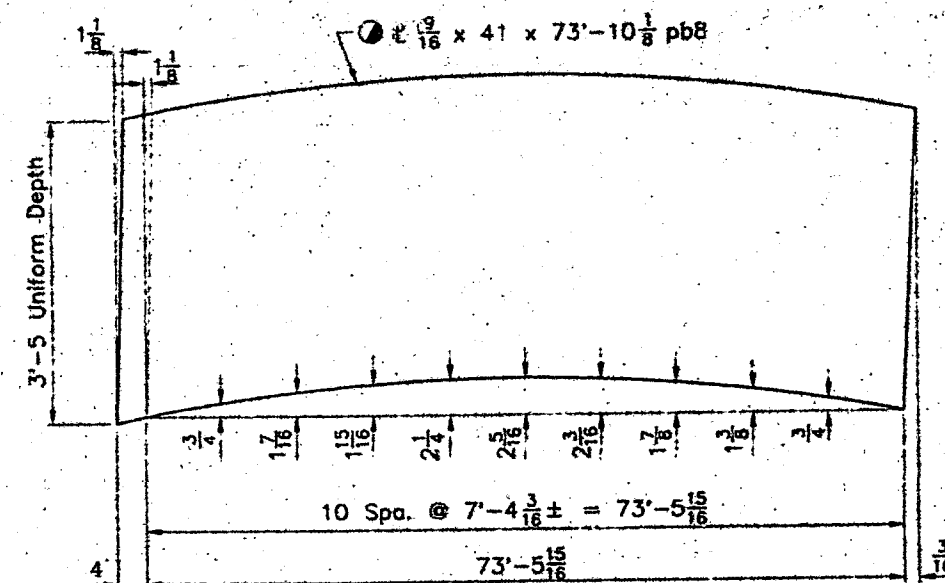


BRIDGE PC 3-05473H SECRET NO. 7 OF 21 ORDER NO. 4213W



HORIZONTAL CURVE - GIRDER D1

SHOP BILL OF MATERIALS									
QTY	NO. PAGES	SHAPE	SIZE	FEET	INCHES	ADD. MARK	REMARKS	CALCULATED	DATE
GIRDER: D1									
1	1	1/2" x 14	73	9		pg8	WEB OF 501		
1	1	1/2" x 41	73	10		pb8	WEB OF 501		
1	1	1/2" x 14	73	9		pc8	WEB OF 501		
1	1	3/4" x 7	3	5	14	SR2	ATD G. 50		
1	1	5/8" x 5	Shear Stud	ma1					



WEB PLATE LAYOUT

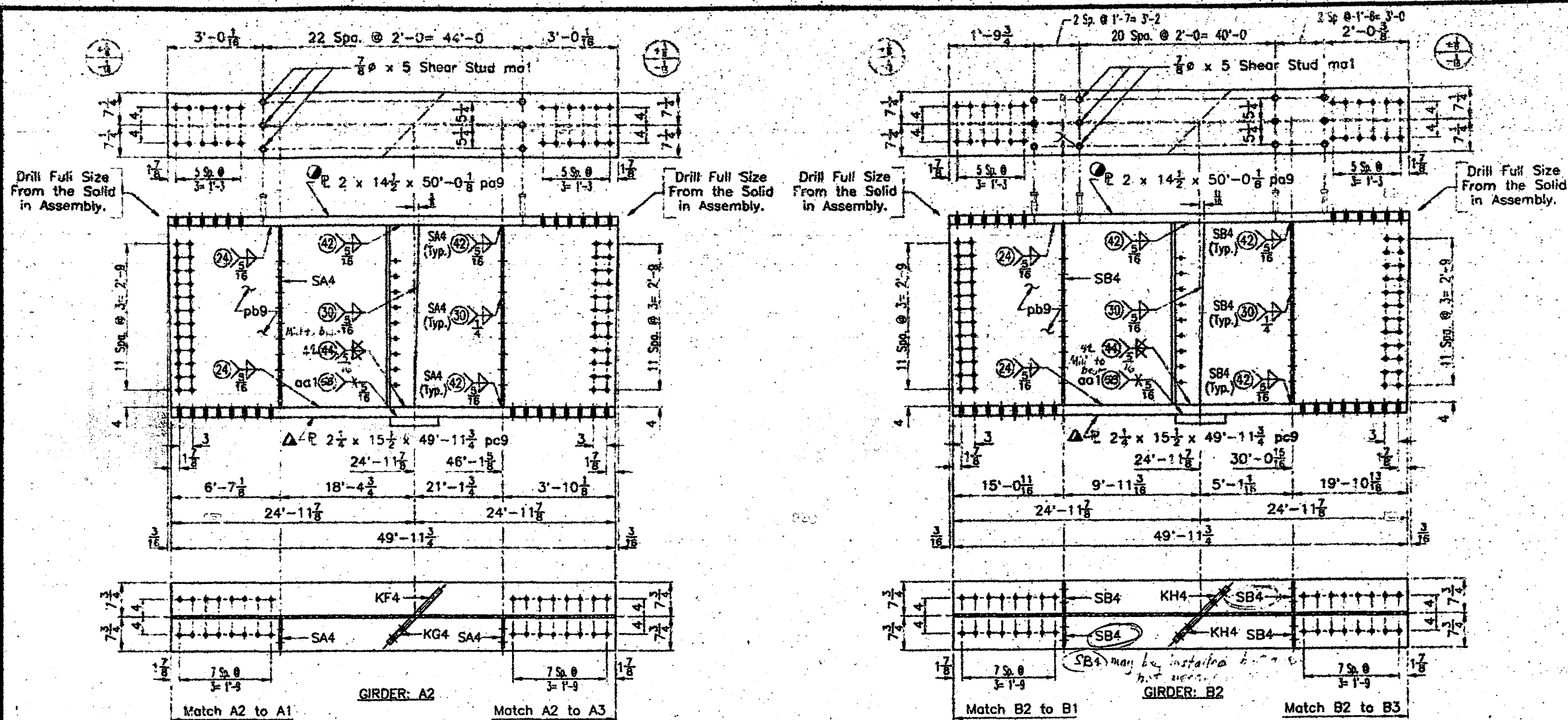
FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPACING
SEE SHEET 5.

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
6/4/97
DATE BY DLR

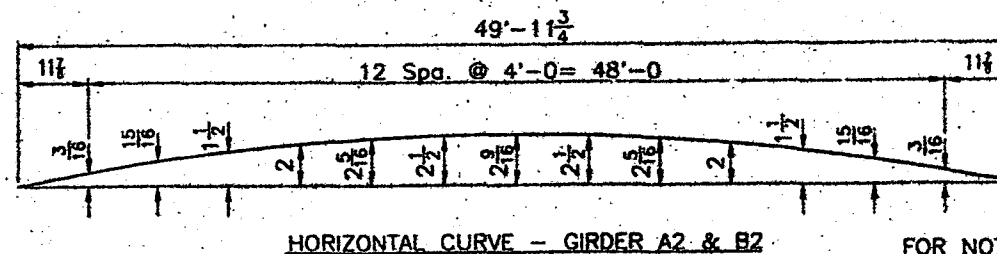
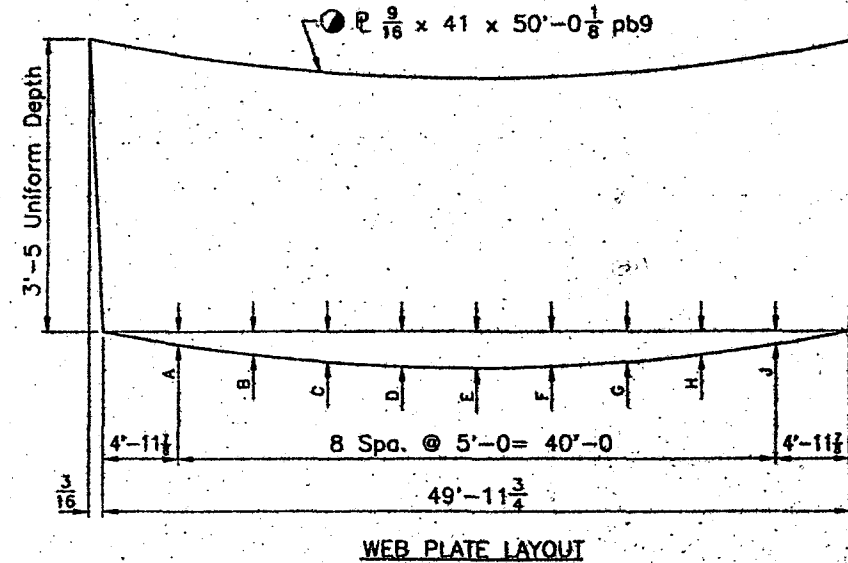
CONTRACT NO.
DESCRIPTION
GIRDER D1 DETAILS

CAPITAL CONTRACTORS, INC.
CAPITAL STEEL DIVISION, LINCOLN, NE

APPROVAL DATE	CONTRACTOR	INDUSTRIAL BUILDERS
DATE OR NOTED	STRUCTURE	3-SPAN W.P.G. BRIDGE (SITE 1)
DESIGN NO.	LOCATION	BURLINGTON NORTHERN OVERHEAD RAY
FIELDS	AS NOTED	CUSTOMER
PAINT	SEE NOTES	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
DRAWN	5-28-97	PROJECT
DATE	5-30-97	STATION
CHIEF	5-30-97	LOADING
BRIDGE NO.	2-053.731L	SHEET NO. 9 OF 21
		ORDER NO. 42138



SHOP BILL OF MATERIALS							
LINE NO.	QTY	SHAPE	LENGTH	ADD.	MARK	REMARKS	CALCULATED WEIGHT EACH
1	2	GIRDERS: A2 & B2					
2	2	2 x 14 1/2	50	0	pa9	W20 x 50	
3	2	2 x 15 1/2	49	1	pb9	W20 x 50	
4	2	2 x 15 1/2	49	1	pc9	W20 x 50	
5	1	1 x 9	3	5	KF4	A709 Gr. 50	
6	1	1 x 9	3	5	KG4	A709 Gr. 50	
7	2	1 x 9	3	5	KH4	A709 Gr. 50	
8	2	1 x 7	3	5	SA4	A709 Gr. 50	
9	2	2 x 11	3	5	SB4	A709 Gr. 50	
10	2	2 x 11	3	5	aa1	By Others	
11	1	4 x 5 Shear Stud			mal		



Girder	A	B	C	D	E	F	G	H	J
A2	-1/16	-1/8	-3/16	-1/4	-1/4	-1/4	-3/16	-1/8	-1/16
B2	-1/16	-1/8	-1/8	-3/16	-3/16	-3/16	-1/8	-1/8	-1/16

CONTRACT NO.

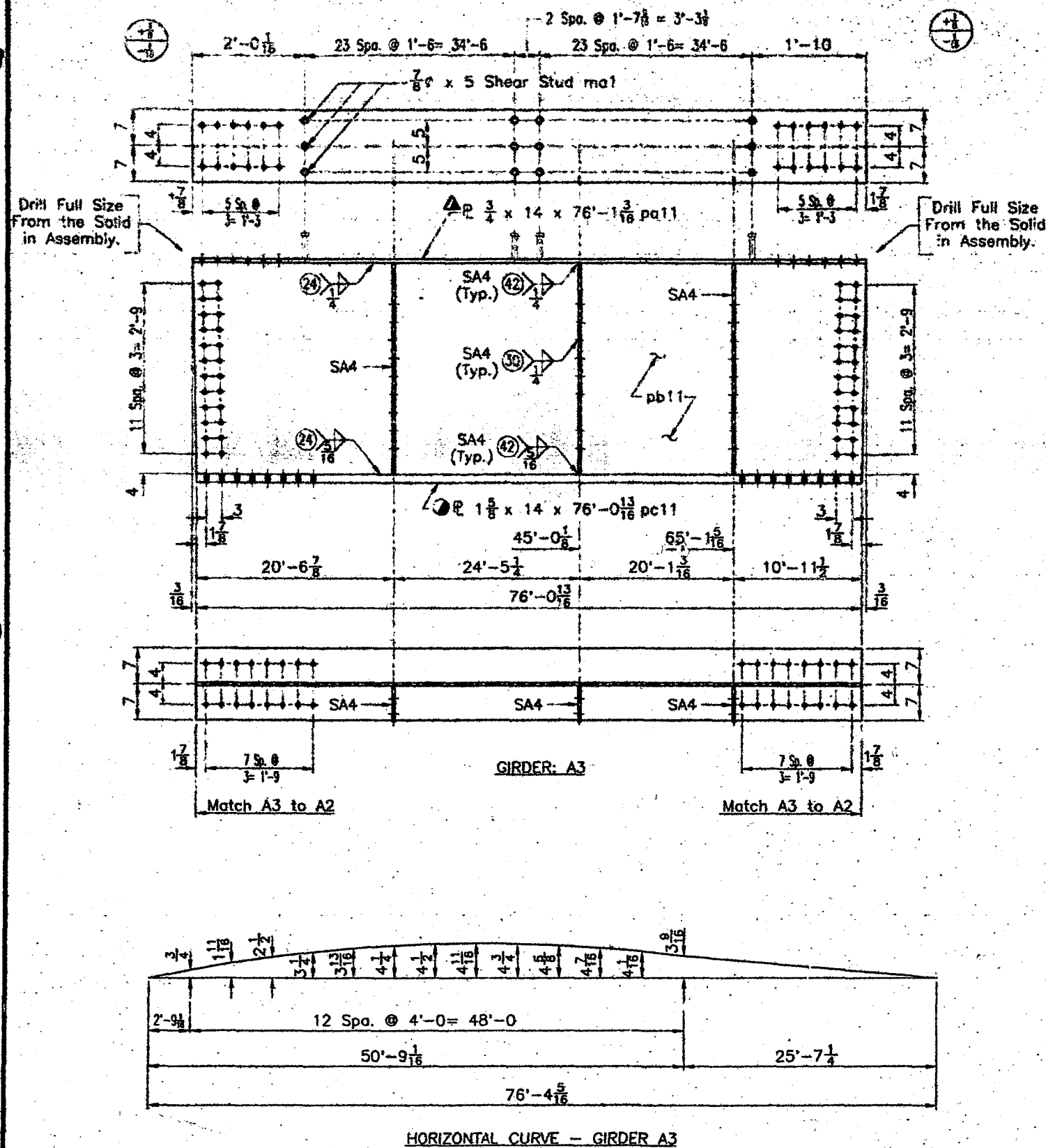
DESCRIPTION

ORDER A2 & B2 DETAILS

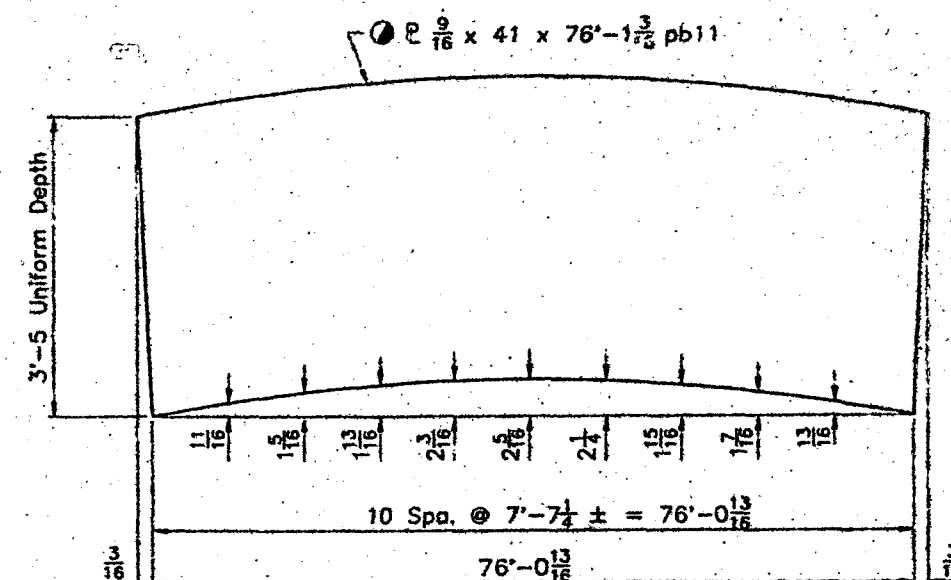
NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
APPROVED AS NOTED
6/4/97
DATE BY DLR

FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPLICES
SEE SHEET 5.

CAPITAL CONTRACTORS, INC. CAPITAL STEEL DIVISION, LINCOLN, NE	
1001 N. 5TH STREET, BOX 80801, LINCOLN, NE 68501-8081	
APPROVAL DATE	CONTRACTOR
STRUCTURE	3-SPAN W.P.S. BRIDGE (TYPE 1)
LOCATION	BURLINGTON NORTHERN OVER-CROSSING
CLIENT	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
PROJECT	NSM-404-7-002(041)052
DESIGNER	WILLIAMS
DATE	5-25-97
STATION	124 + 43.6
ROADWAY	30 FT.
CHECKED	5-30-97
LOADING	MS23
KEY	VARIES
BRIDGE NO.	2-053.731L
SHEET NO.	10 OF 21
ORDER NO.	43138



SHOP BILL OF MATERIALS							
LINE	NO. OF PIECES	SHAPE	LENGTH	WEIGHT	MARK	QUANTITY	REMARKS
1	1	GIRDER	A3				
2	1	1 5/8" x 14	76	1	pa11	1	W70 x 58
3	1	1 5/8" x 14	76	1	pb11	1	W70 x 58
4	1	1 5/8" x 14	76	1	pc11	1	W70 x 58
5	3	1 5/8" x 7	3	5	SA4	15	W70 x 58
6	1	1 5/8" x 5 Shear Stud	mat				



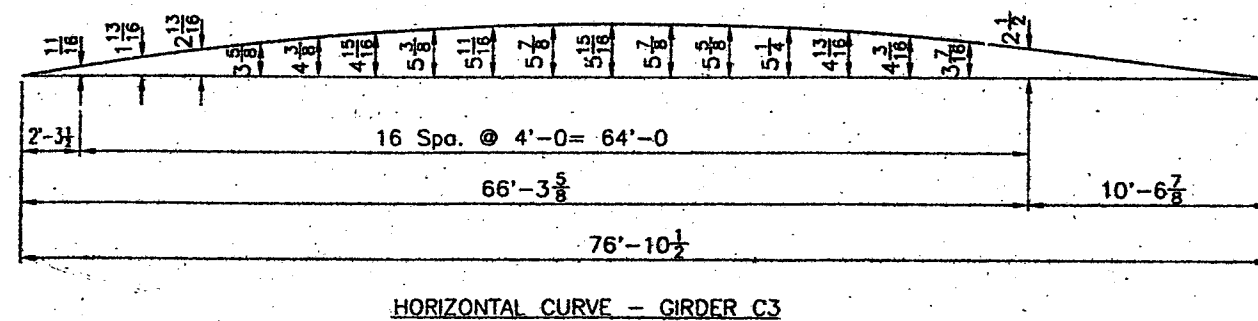
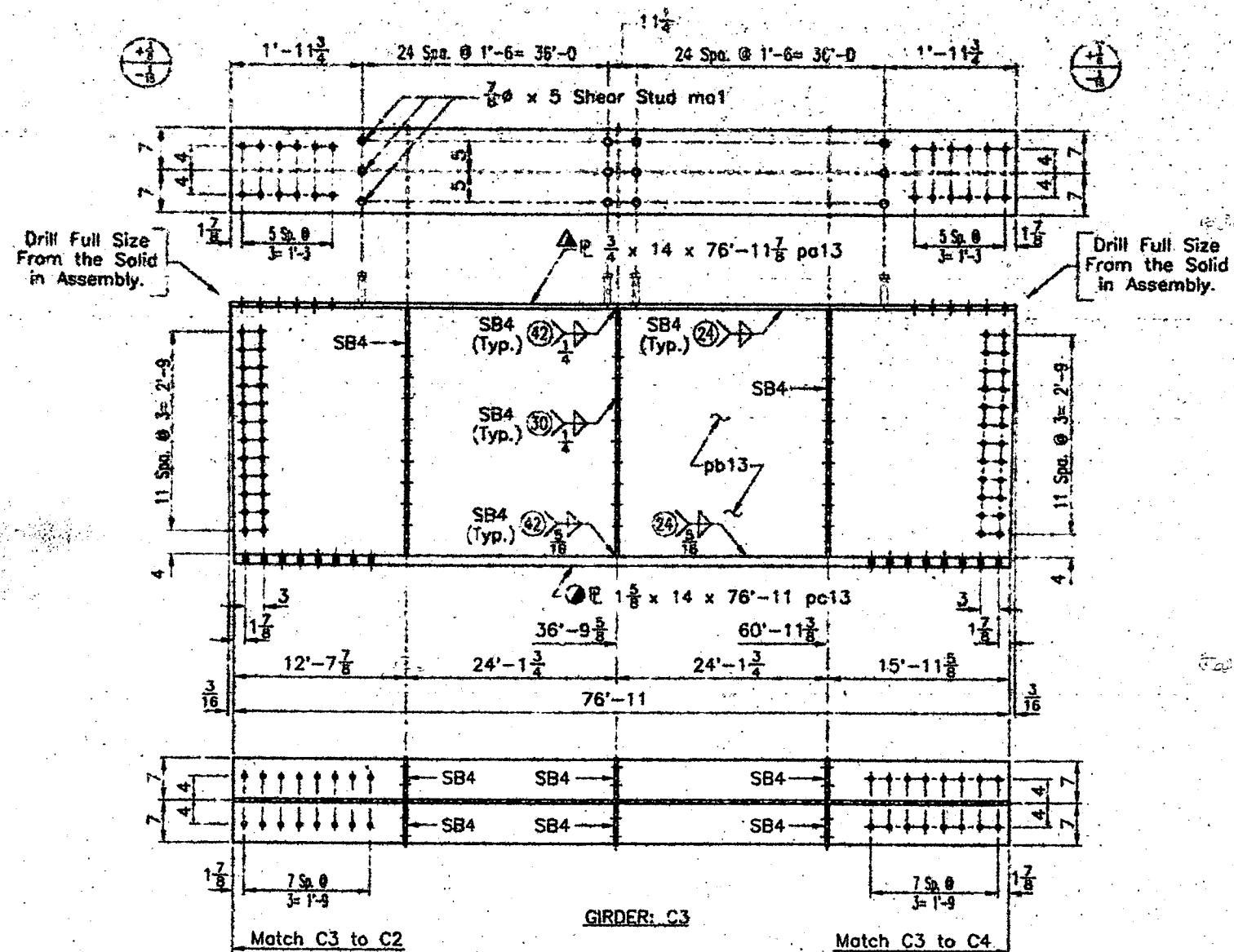
WEB PLATE LAYOUT

FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPLICES
SEE SHEET 5.

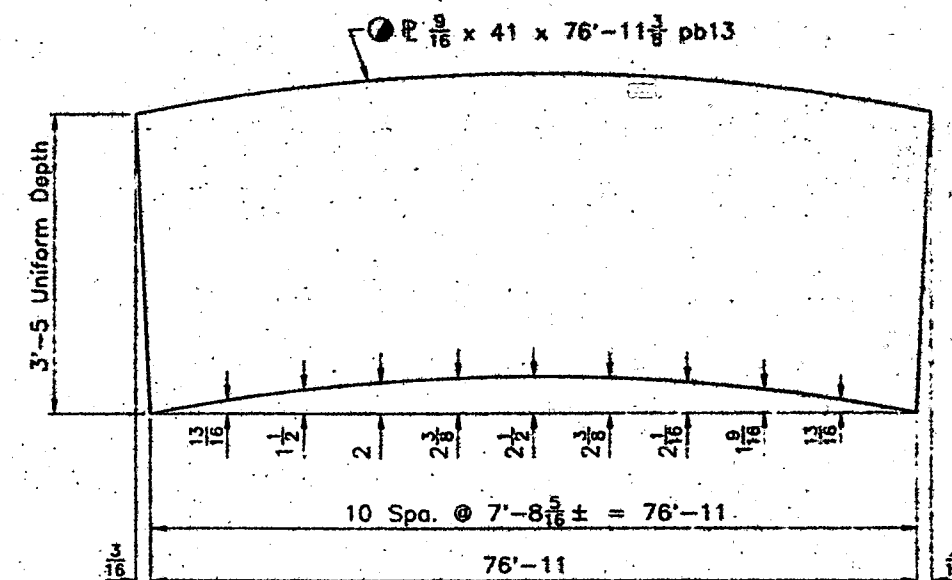
NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE BY DLR

CAPITAL CONTRACTORS, INC.			
CAPITAL STEEL DIVISION, LINCOLN, NE			
1001 N. 5TH STREET, BOX 8008, LINCOLN, NE 68501, PH. 402-476-1021			
APPROVAL DATE	CONTRACTOR	INDUSTRIAL BUILDERS	
NO. 1	STRUCTURE	3-SPAN W.P.A. BRIDGE (SITE 1)	
DESIGN NO.	LOCATION	BURLINGTON NORTHERN OVERHEAD RAY	
FIELD	AS NOTED	CUSTOMER NORTH DAKOTA DEPARTMENT OF TRANSPORTATION	
PAINT	SEE NOTES	PROJECT	WILLIAMS
DRAWN	5-9-97	STATION	124 + 43.0
CHECKED	5-30-97	LOADED	MS25
BRIDGE NO.	2-053.731L	SHEET NO.	12 OF 21

CONTRACT NO.
DESCRIPTION
GIRDER A3 DETAILS



SHOP BILL OF MATERIALS									
LINE NO.	QTY	DESCRIPTION	UNIT	REMARKS	REMARKS	REMARKS	REMARKS	REMARKS	REMARKS
1	1	GIRDER: C3							
2	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
3	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
4	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
5	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
6	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
7	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
8	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
9	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
10	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
11	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
12	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
13	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
14	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
15	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
16	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
17	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
18	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
19	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
20	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
21	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
22	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
23	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
24	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
25	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
26	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
27	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
28	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
29	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
30	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
31	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
32	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
33	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
34	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
35	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
36	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
37	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
38	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
39	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
40	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
41	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
42	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
43	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
44	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
45	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
46	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
47	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
48	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
49	1	1/2\" x 14\" x 76'-11 3/8\" pc13							
50	1	1/2\" x 14\" x 76'-11 3/8\" pc13							



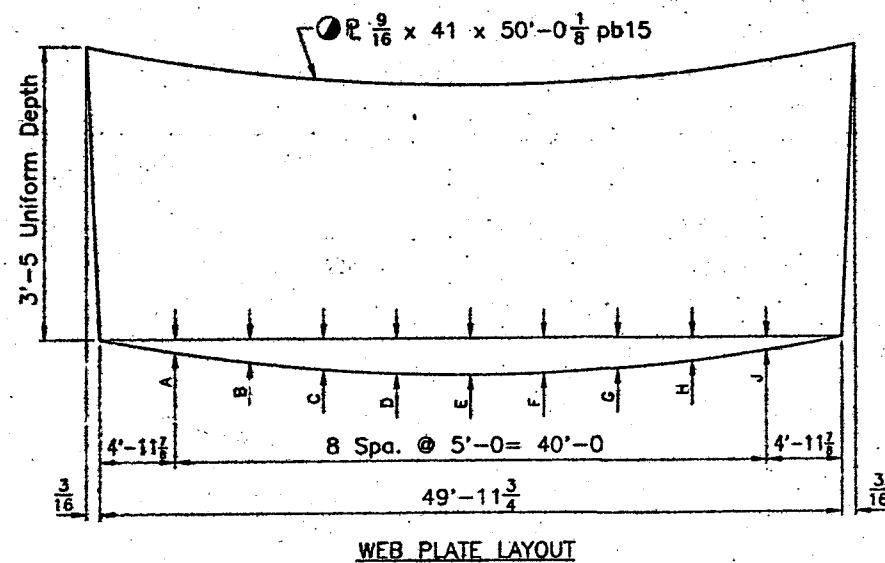
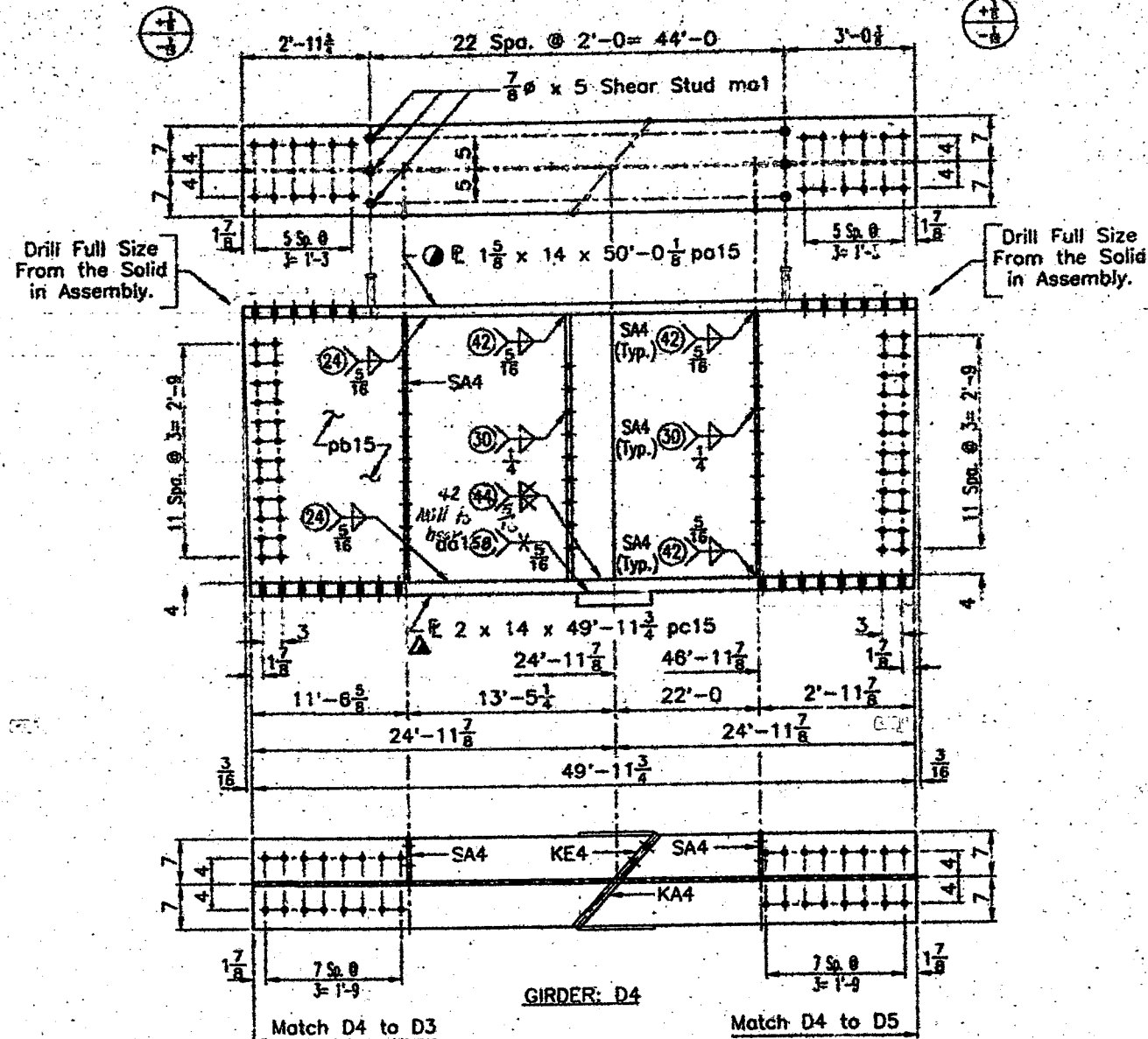
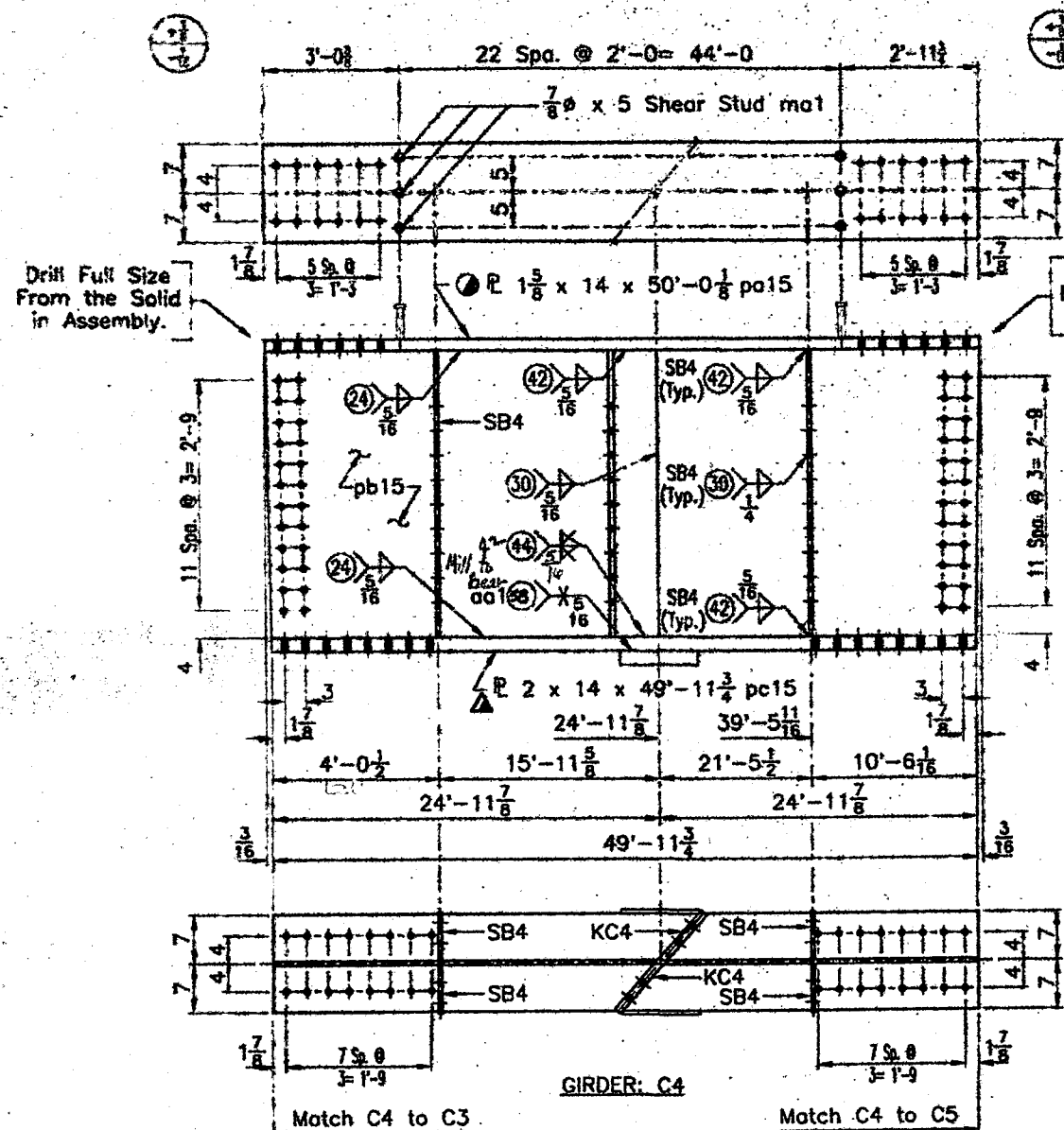
WEB PLATE LAYOUT

FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPICES
SEE SHEET 5.

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE

CONTRACT NO.
DESCRIPTION
GIRDER C3 DETAILS

CAPITAL CONTRACTORS, INC.
CAPITAL STEEL DIVISION, LINCOLN, NE
1001 N. 3TH STREET, BOX 8008, ZIP 68501, PH. 402-476-1021
APPROVAL DATE
CONTRACTOR
INDUSTRIAL BUILDERS
FIELD NO. NOTED
STRUCTURE
3-SPAN W.P.C. BRIDGE (SEE 1)
DESIGN NO.
LOCATION
BURLINGTON NORTHERN OVERHEAD R.
FIELD NO. NOTED
CUSTOMER
NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
PROJECT
RSD-104-7-002(041)052
COUNTY
WILLIAMS
DRAWN
5-9-97
G. CLARK
STATION
124 + 43.6
ROADWAY
30 FT.
CHECKED
5-3-97
LOADING
MS05
JEEV
VARIES
BRIDGE NO.
2-053731L
SHEET NO. 14 OF 21
ORDER NO.
4213W



Girder	A	B	C	D	E	F	G	H	J
A2	0	-1/16	-1/8	-1/8	-3/16	-1/8	-1/8	-1/16	0
B2	0	-1/16	-1/8	-3/16	-3/16	-3/16	-1/8	-1/16	-1/16

SHOP BILL OF MATERIALS									
LINE NO.	NO. OF PIECES	SHAPE	LENGTH	FEET	INCHES	ADD.	MARK	REMARKS	CALCULATED WEIGHT EACH
1	2	GIRDERS: C4 & D4							
2	2	1/2" x 41 x 50'-0 1/8" pb15	50	0	0		pb15	W70 x 502	
3	2	1/2" x 41 x 50'-0 1/8" pb15	50	0	0		pb15	W70 x 502	
4	2	2 x 14 x 49'-11 3/4" pc15	49	1	1		pc15	W70 x 502	
5	2	1 x 8 x 3 x 5	3	5	1		KC4	A709 G. 50	
6	1	1 x 8 x 3 x 5	3	5	1		KE4	A709 G. 50	
7	1	1 x 8 x 3 x 5	3	5	1		KA4	A709 G. 50	
8	2	1 x 7 x 3 x 5	3	5	1		SA4	A709 G. 50	
9	4	1 x 7 x 3 x 5	3	5	1		SB4	A709 G. 50	
10	2	2 x 11 x 1 x 3	1	3	1		aa1	By Others	
11	13	7/8" x 5 Shear Stud					mq1		

CONTRACT NO.

DESCRIPTION

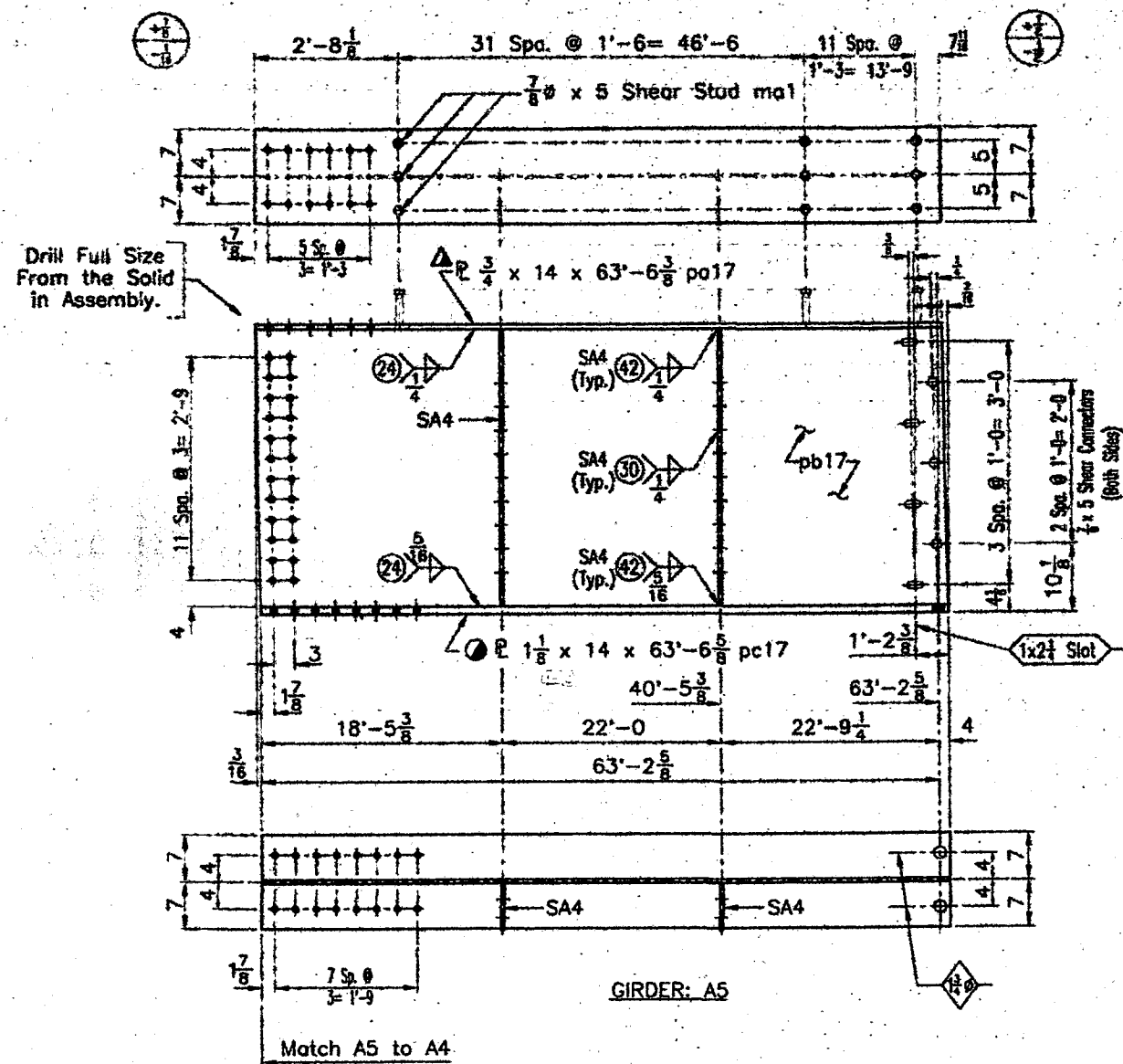
ORDER C4 & D4 DETAILS

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
APPROVED AS NOTED
6/4/97
DATE BY DLR

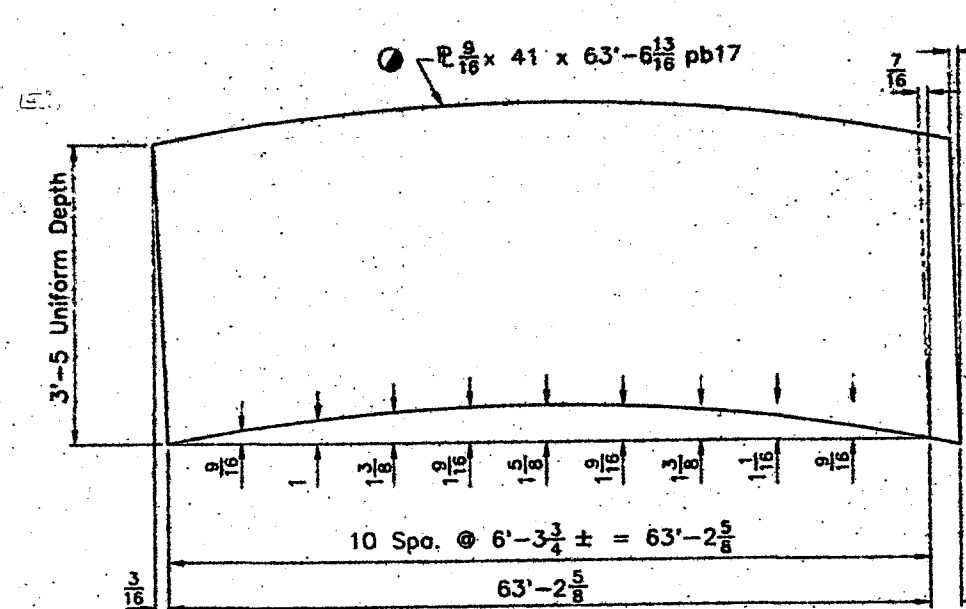
FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPLICES
SEE SHEET 5.

CAPITAL CONTRACTORS, INC.
CAPITAL STEEL DIVISION, LINCOLN, NE
1801 N. 9TH STREET, BOX 60080, ZIP 68501, TEL 402-476-1081

APPROVAL DATE	CONTRACTOR	INDUSTRIAL BUILDERS
DATE	BY	
STRUCTURE	3-SPAN W.P.C. BRIDGE (SITE 1)	
DESIGN NO.	LOCATION	BURLINGTON NORTHERN OVERHEAD RAY
RELAY	AS NOTED	CUSTOMER
PAINT	SEE NOTES	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
DRAWN	5-30-97 G. CLARK	PROJECT
CHECKED	5-30-97	STATION
BRIDGE NO.	2-053.731L	LOADING
		HS25
		SEW
		VARIES
		ORDER NO.
		4213W



SHOP BILL OF MATERIALS									
QTY	NO. PLATES	SHAPE	LENGTH FEET	THICKNESS INCHES	ADD. MARK	QTY	QTY	QTY	QTY
1	1	GIRDER: A5							
2	1	1 1/8 x 14	63	3/8	pc17	2	2	2	2
3	1	1 1/8 x 14	63	3/8	pb17	2	2	2	2
4	1	1 1/8 x 14	63	3/8	pc17	2	2	2	2
5	2	1 1/8 x 7	3	5/8	SA4	2	2	2	2
6	1	3/8 x 5	Shear Stud	ma1					
7									
8									
9									
10									
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14									
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17									
18									
19									
20									



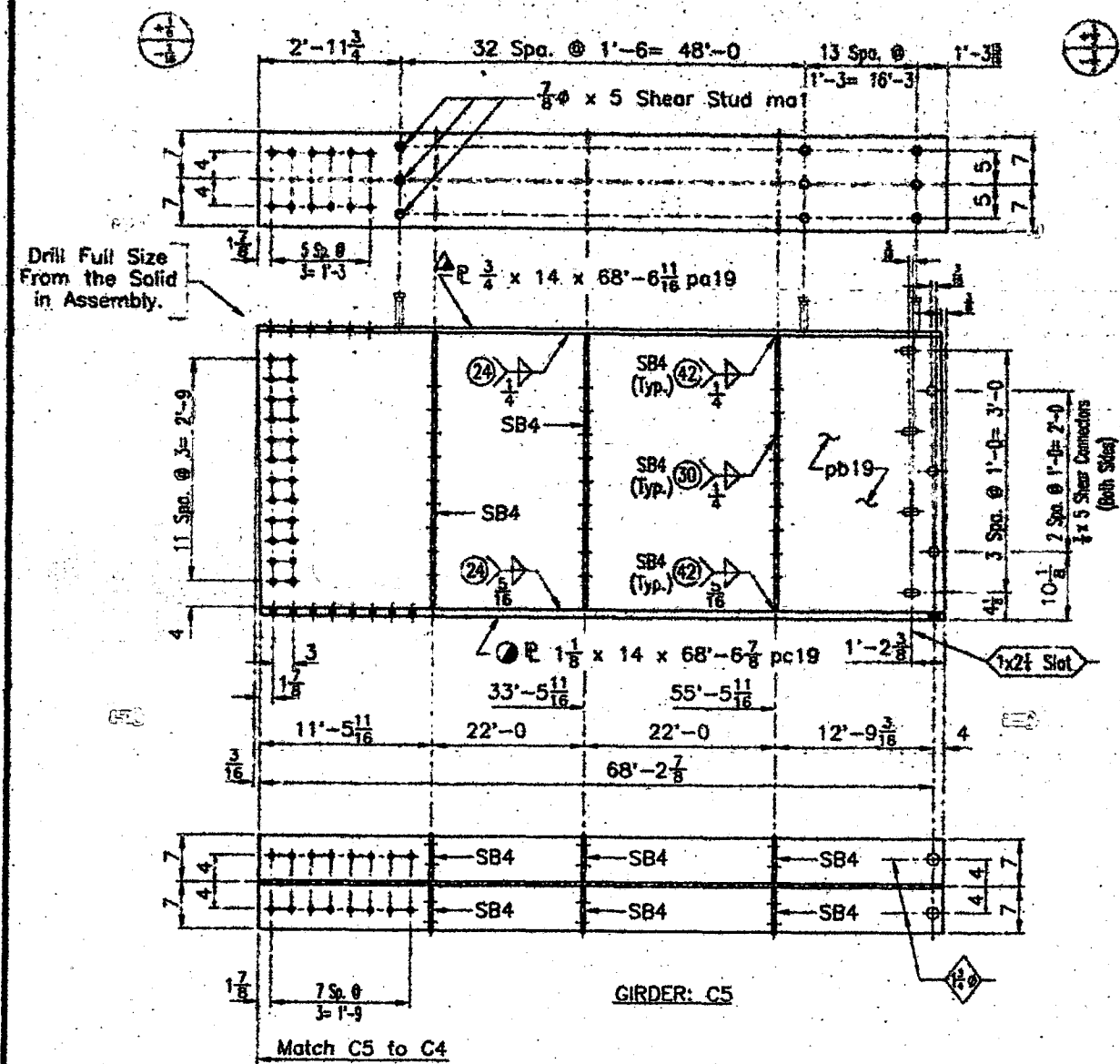
WEB PLATE LAYOUT

FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPLICES
SEE SHEET 5.

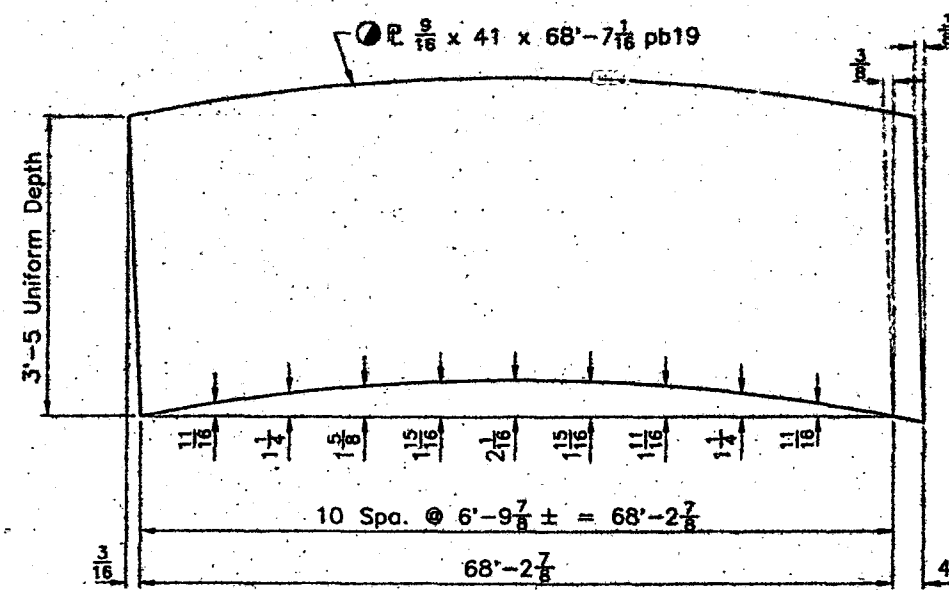
NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE BY DLR

CONTRACT NO.
DESCRIPTION
ORDER AS DETAILS

CAPITAL CONTRACTORS, INC.			
CAPITAL STEEL DIVISION, LINCOLN, NE			
1001 N. 5TH STREET, BOX 60000, LINCOLN, NE 68501, TEL. 402-476-1000			
APPROVAL DATE	CONTRACTOR	INDUSTRIAL BUILDERS	
NO. H. OR NOTED	STRUCTURE	3-SPAN W.P.D. BRIDGE (SITE 1)	
DESIGN NO.	LOCATION	BURLINGTON NORTHERN OVERHEAD RAY	
FIELD	AS NOTED	CUSTOMER NORTH DAKOTA DEPARTMENT OF TRANSPORTATION	
PAINT	SEE NOTES	PROJECT	100-101-7-002(041)003
DRAWN	5-30-97	STATION	124 + 43.8
Q. CLARK		ROADWAY	30 FT.
CHECKED	5-30-97	LOADING	HS20
BRIDGE NO.	2-053.731L	SHEET NO.	10 OF 21
		ORDER NO.	42130



SHOP BILL OF MATERIALS									
LINE NO.	NO. OF PIECES	SHAPE	LENGTH		WGT.	WELD	REMARKS	CALCULATED WEIGHT	EACH
			FEET	INCHES					
1	1	GIRDER: C5							
2	1	1/2" x 14	68	8	6	11	pc19 W70 x 55		
3	1	1/2" x 41	68	8	7	11	pb19 W70 x 55		
4	1	1/2" x 14	68	8	6	11	pc19 W70 x 55		
5	6	1/2" x 7	3	5			SB4 W70 x 55		
6	1	4" x 5 Shear Stud					mo		
7									
8									
9									
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11									
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14									
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17									
18									
19									
20									



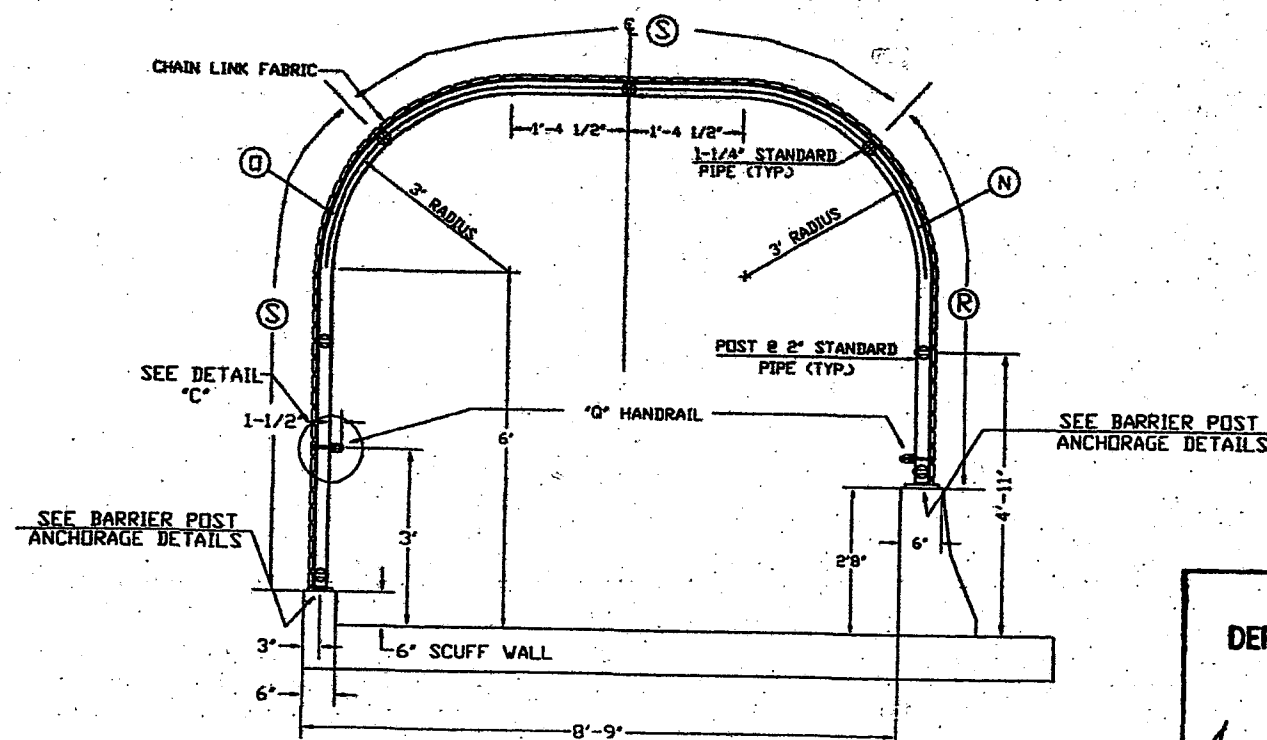
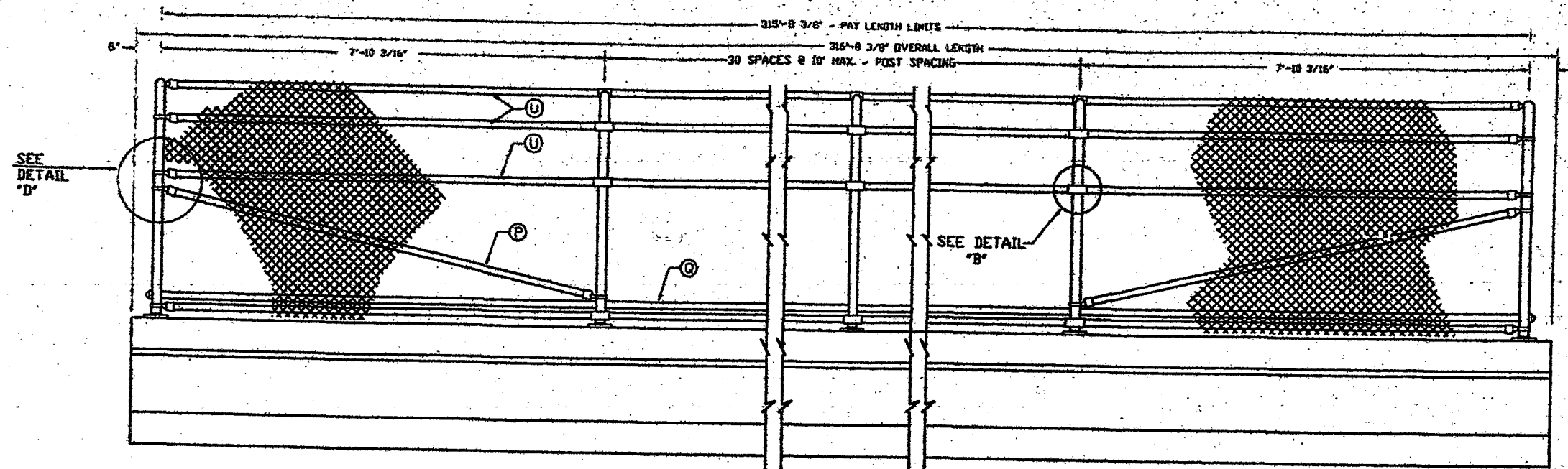
WEB PLATE LAYOUT

FOR NOTES SEE SHEET 1.
STUD SPACING OVER FIELD SPLICES
SEE SHEET 5.

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
6/4/97
DATE BY DLR

CONTRACT NO.
DESCRIPTION
GIRDER C5 DETAILS

CAPITAL CONTRACTORS, INC.			
CAPITAL STEEL DIVISION, LINCOLN, NE			
1001 N. 17TH STREET, BOX 80038, ZIP 68501, TEL. 402-476-1031			
APPROVAL DATE		CONTRACTOR	
DATE OR NOTED		INDUSTRIAL BUILDERS	
STRUCTURE		3-SPAN W.P.C. BRIDGE (SITE 1)	
DESIGN NO.		LOCATION	
		SULPHUR NORTHEN OVERROAD RY	
FIELD	AS NOTED	CUSTOMER	
FIELD	SEE NOTES	NORTH DAKOTA DEPARTMENT OF TRANSPORTATION	
DRAWN	5-30-97	PROJECT	RSN-MH-7-002(041)052
	G. CLARK	STATION	124 + 43.6
CHECKED	5-30-97	LOADING	MS25
		SKED	WRES
BRIDGE NO.	2-053.731L	SHEET NO.	20 OF 21
		ORDER NO.	42136



PARTS LIST

PART#	QTY.	DESCRIPTION	MATERIAL
A	18	RAIL END	
B	217	LINE RAIL CLAMP	GALVANIZED
C	44	2-1/2\" TENSION BANDS	GALVANIZED
D	66	1/2\" X 4-1/2\" CARRIAGE BOLTS	GR-2 GALVANIZED
E			
F	132	C CHANNEL SUPPORTS	A-36 GALVANIZED
G	467	3/8\" X 3\" CARRIAGE BOLTS	GR-2 GALVANIZED
H	33	6\" X 6\" X 5/8\" BASE PLATES	A-36 GALVANIZED
I	33	7\" X 4-1/2\" X 1/2\" BASE PLATES	A-36 GALVANIZED
J	198	1/2\" X 7\" ANCHOR BOLTS	GR-5 GALVANIZED
K	264	1/2\" NUTS	GR-2 GALVANIZED
L	198	1/2\" FLAT WASHERS	GR-2 GALVANIZED
M	33	1-1/2\" X 4\"	STD. PIPE GALVANIZED
N	33	2\" X 9'-4 1/16\"	STD. PIPE GALVANIZED
O	33	2\" X 11'-6 7/16\"	STD. PIPE GALVANIZED
P	2	1-1/4\" X 8'-5\"	STD. PIPE GALVANIZED
Q	2	1-1/4\" X 315'-8 3/8\"	STD. PIPE GALVANIZED
R	316'-0\"	9 GAUGE CHAIN LINK 6'-0\"	FABRIC GALVANIZED
S	632'-0\"	9 GAUGE CHAIN LINK 8'-0\"	FABRIC GALVANIZED
T	33	1-1/2\" X 10\"	STD. PIPE GALVANIZED
U	224	1-1/4\" X 10\"	STD. PIPE GALVANIZED
V	18	2-1/2\" BRACE BANDS	GALVANIZED
W	4	1-5/8\" DOME CAPS	GALVANIZED
X			
Y	62	1-1/4\" X 5/16\" CARRIAGE BOLTS	GR-2 GALVANIZED
Z	3004	9 GA ALUMINUM FENCE TIES	ALUMINUM
AA	2	6'-0\" TENSION BARS	GALVANIZED
BB	4	8'-0\" TENSION BARS	GALVANIZED
CC	2	8'-0\" TENSION BARS	GALVANIZED

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING

Aug 5, 97 BY DLR

Br. No. 2-053.731 R

FHWA REGION 8 FAPN RSN-NH-7-002(041)052
STRUCTURE NO. 2--053.731R
PEDESTRIAN BRIDGE CANOPY

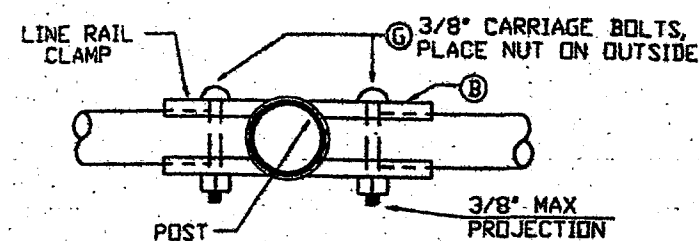
RAY, NORTH DAKOTA

SCALE: APPROVED BY: DRAWN BY: E. GARLAND
DATE: 5/14/97 REVISED:

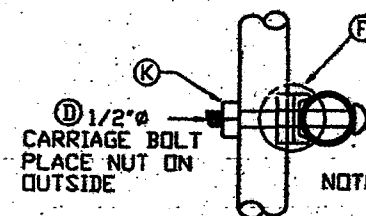
N.D. DEPARTMENT OF TRANSPORTATION -- ENGINEERS
NORTHERN IMPROVMENTS >> PRIME CONTRACTOR FARGO, ND

DAKOTA FENCE COMPANY SUB CONTRACTOR
FARGO BISMARCK MINOT
NORTH DAKOTA

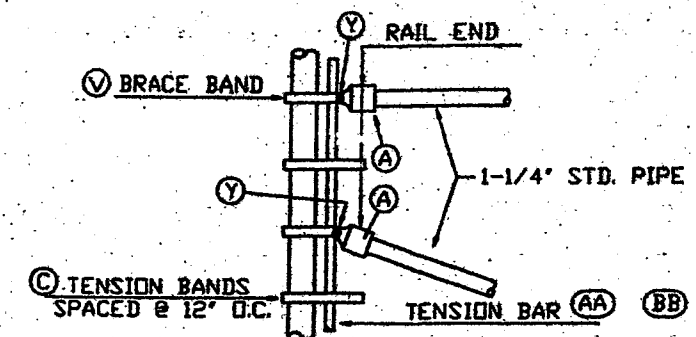
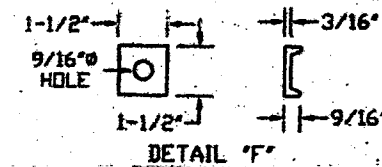
DRAWING #
9616



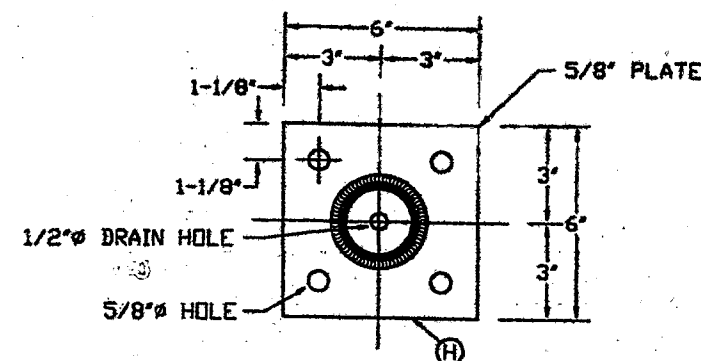
DETAIL "B"



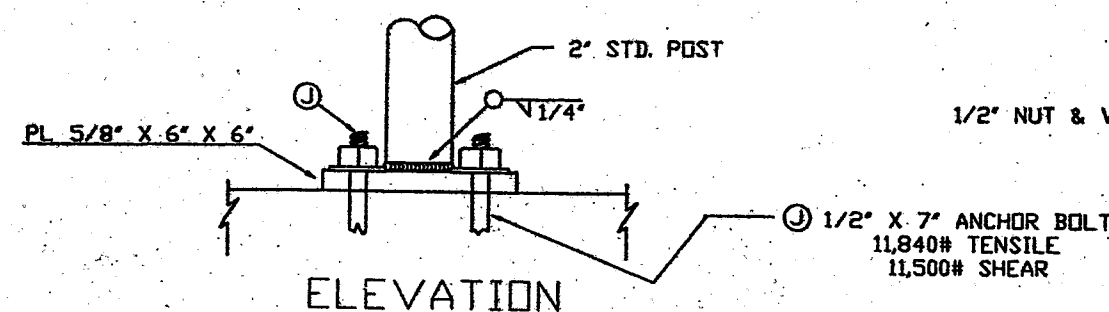
DETAIL "C"



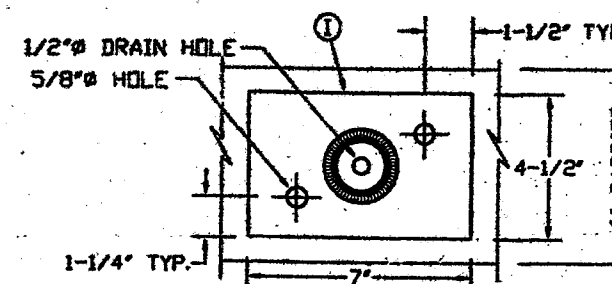
DETAIL "D"



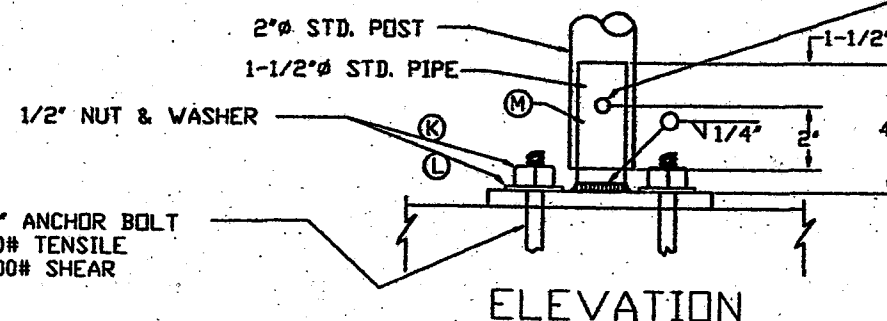
BASE PLATE PLAN



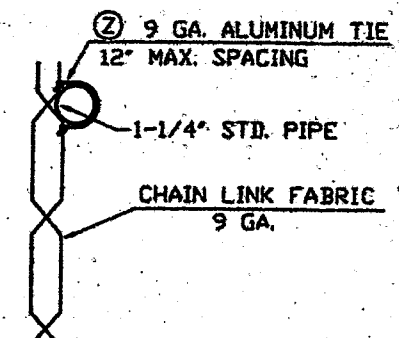
PARAPET POST ANCHORAGE DETAILS



BASE PLATE PLAN



BARRIER POST ANCHORAGE DETAILS

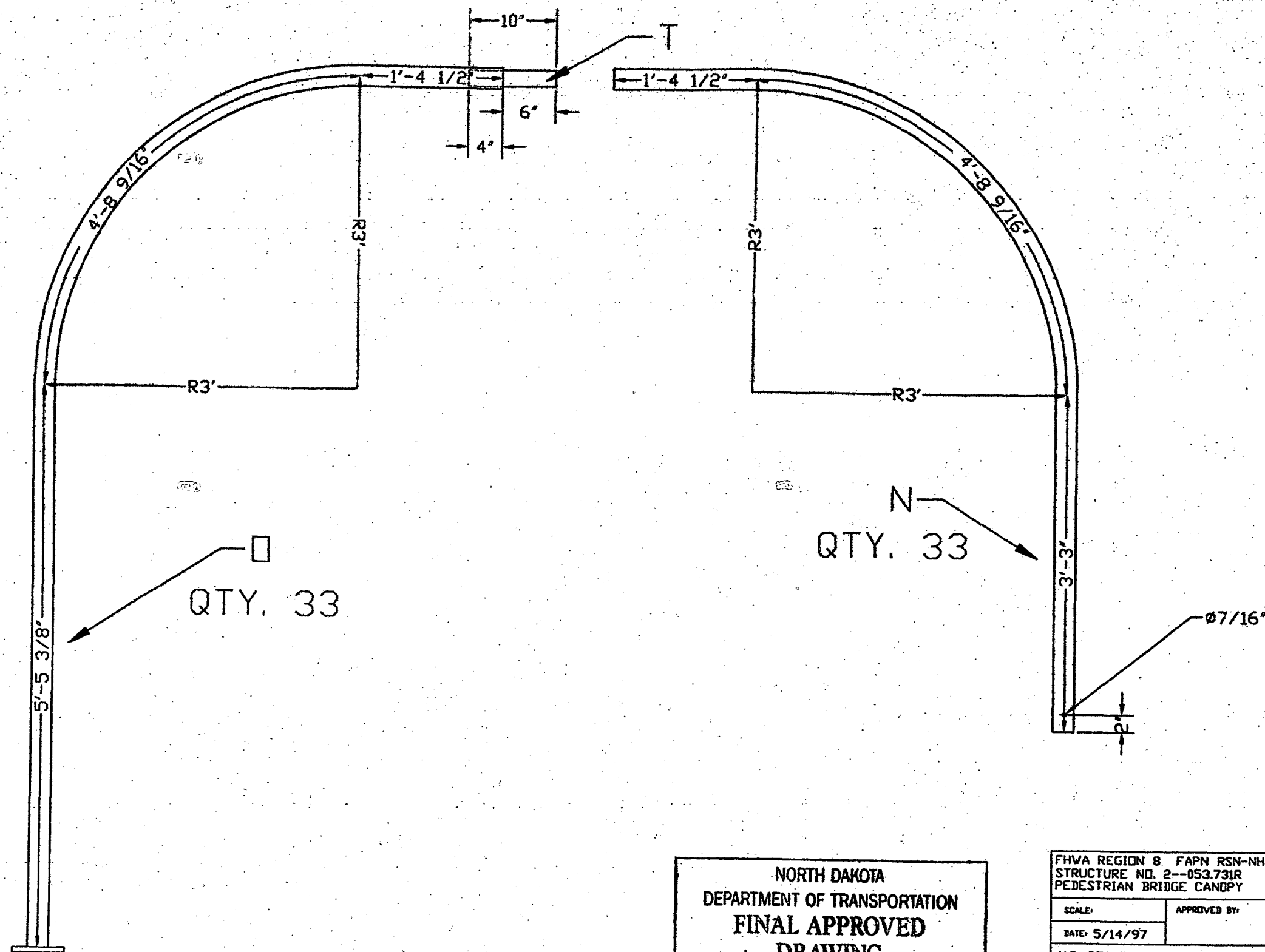


TYPICAL FABRIC ATTACHMENT

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING

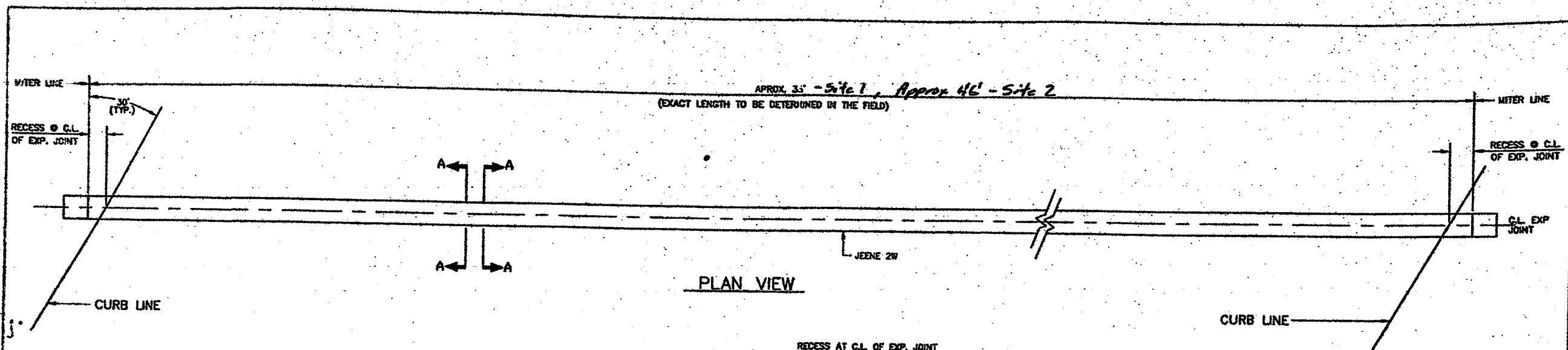
8/5/97 DATE BY DLR

FHWA REGION 8 FAPN RSN-NH-7-002(04)052 STRUCTURE NO. 2--053.731R PEDESTRIAN BRIDGE CANOPY			RAY, NORTH DAKOTA
SCALE:	APPROVED BY:	DRAWN BY: E. GARLAND	
DATE: 5/14/97		REVISED	
N.D. DEPARTMENT OF TRANSPORTATION -- ENGINEERS NORTHERN IMPROVMENTS >> PRIME CONTRACTOR FARGO, ND			
DAKOTA FENCE COMPANY SUB CONTRACTOR FARGO BISMARCK MINOT NORTH DAKOTA			DRAWING # 9616

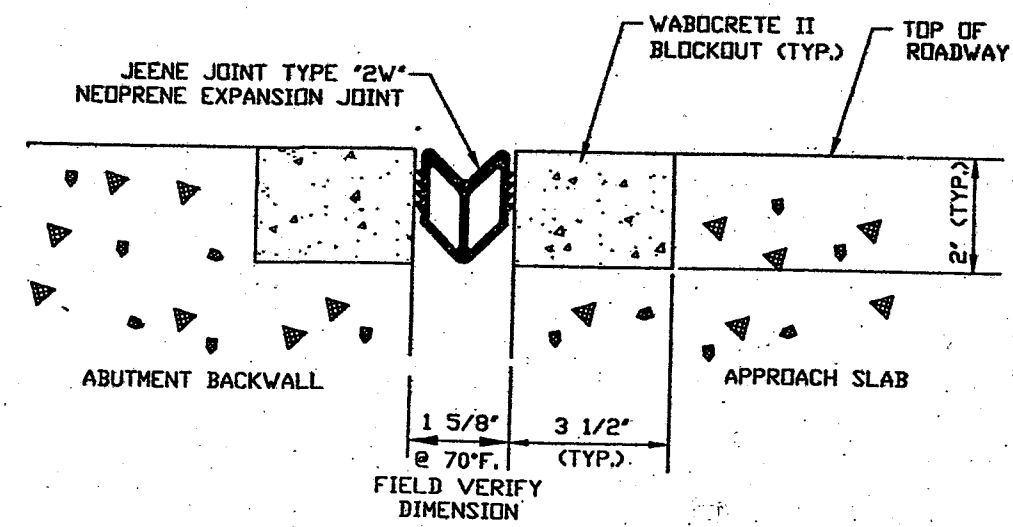


NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
8/5/97 DATE BY *DLR*

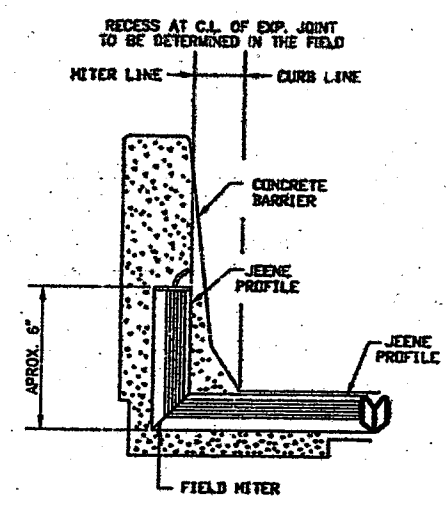
FHWA REGION 8 FAPN RSN-NH-7-002041052		
STRUCTURE NO. 2--053.731R		
PEDESTRIAN BRIDGE CANOPY		RAY, NORTH DAKOTA
SCALE:	APPROVED BY:	DRAWN BY: E. GARLAND
DATE: 5/14/97		REVISED
N.D. DEPARTMENT OF TRANSPORTATION ** ENGINEERS		
NORTHERN IMPROVMENTS >> PRIME CONTRACTOR		FARGO, ND
DAKOTA FENCE COMPANY SUB CONTRACTOR		DRAWING #
FARGO BISMARCK MINOT		9616
NORTH DAKOTA		



PLAN VIEW



SECTION VIEW A - A



SECTION VIEW
SPLIT BARRIER

TEMPERATURE ADJUSTMENT
THE CHANGE PER 10°F. IS MEASURED PERPENDICULAR TO THE JOINT FOR BOTH NORMAL AND SKEWED JOINTS.
DIMENSIONS SPANNING THE OPEN JOINT WILL INCREASE OR DECREASE FOR EVERY 1°F. RISE OR FALL RESPECTIVELY.
1/8" INCHES AT STA 122 + 60.00
1/8" INCHES AT STA 126 + 6.50



MARK SYSTEM

GENERAL NOTES:

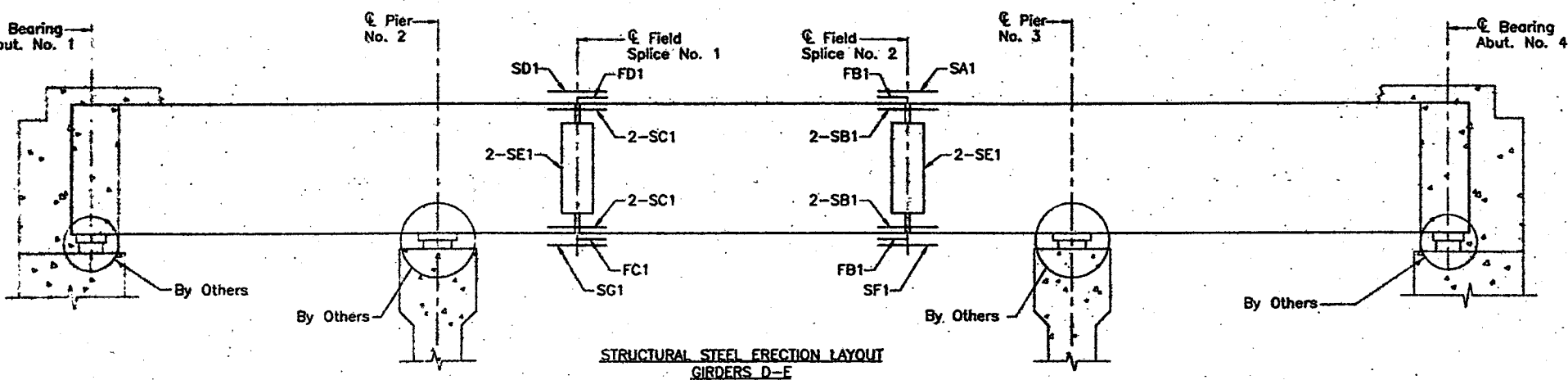
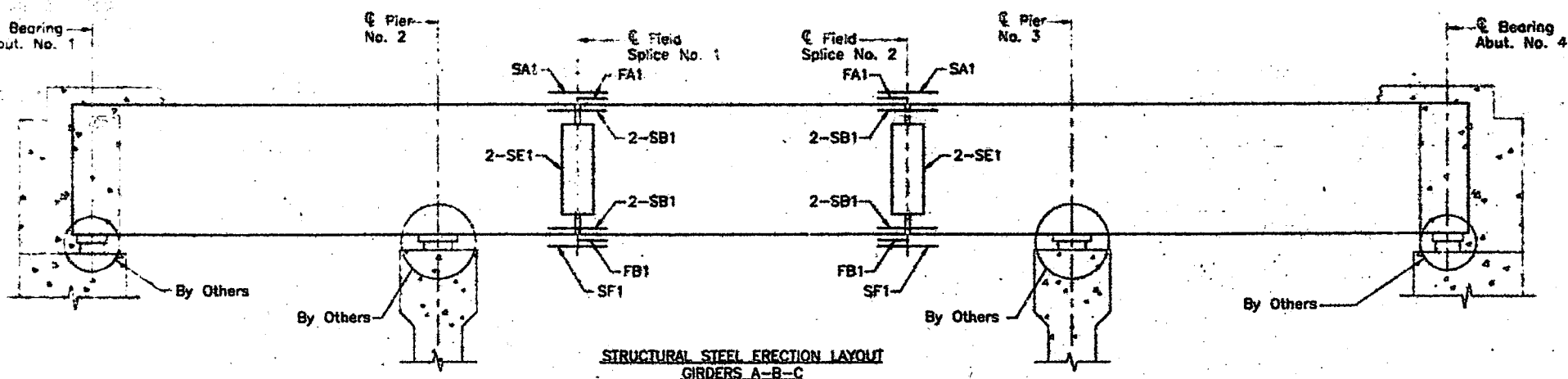
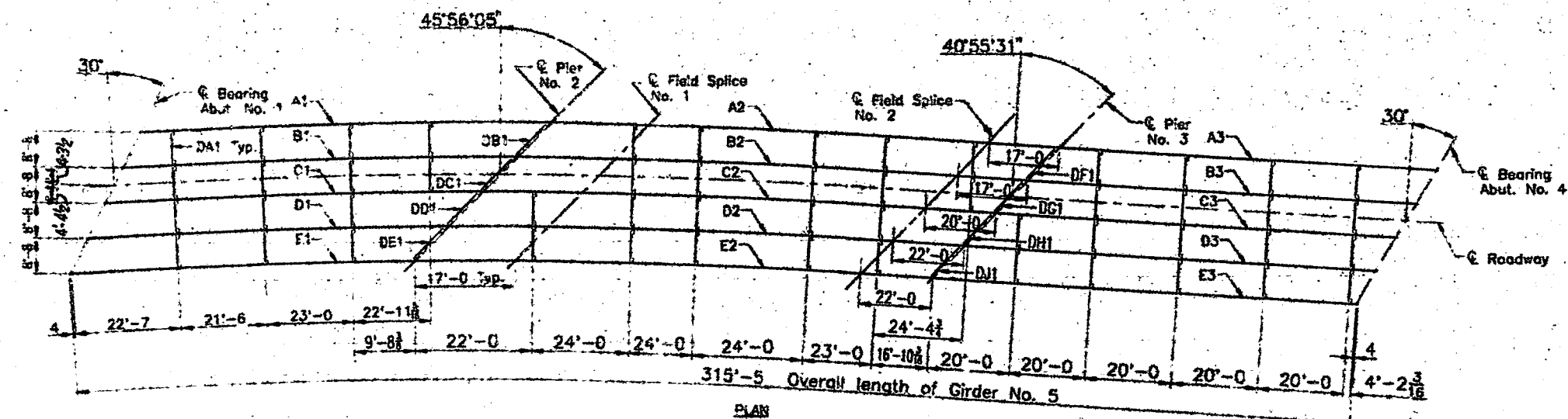
1. WABOCRETE II SHALL BE INSTALLED IN ACCORDANCE WITH WATSON BOWMAN ACME INSTALLATION PROCEDURES.
2. JEENE PROFILE SHALL BE INSTALLED IN ACCORDANCE WITH WATSON BOWMAN ACME INSTALLATION PROCEDURES.

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
8/26/97 DATE BY DLR

STATE: NORTH DAKOTA
COUNTY: WILLIAMS
PROJECT NO.: RSN-NH-7-002(041)052
REGION: 8 ROUTE: US 2

 A DIVISION OF HARRIS SPECIALTY CHEMICALS, INC.	DESIGNED BY	DATE
	MN	8/15/97
	CHECKED BY	DATE
	AML	8/15/97
REVISIONS	SCALE	NO. JOB FILE
	N.T.S.	48289
PROJECT	SHEET NO.	WORKING FILE
US 2 BURLINGTON NORTHERN RAILROAD SEPARATION IN RAY JEENE 2W EXPANSION JOINT DETAILS	1 OF 1	8-17078

#2-053.731 L&R



BILL OF WEIGHTS

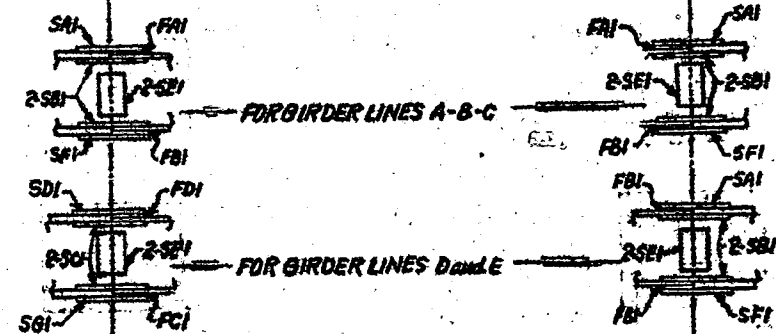
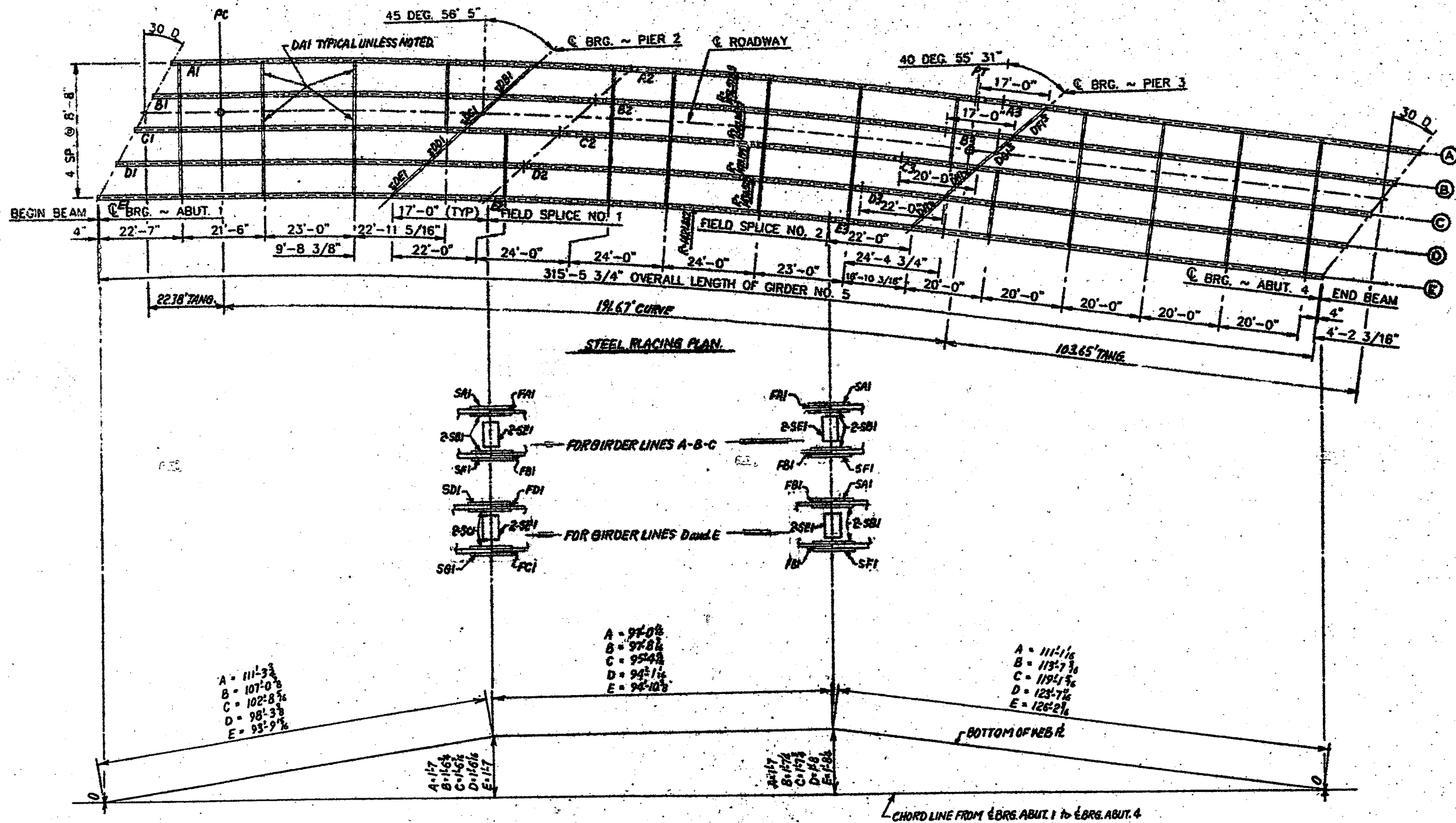
ITEM	EACH	TOTAL
Diaphragms:		
52-DA1	387 lbs.	20,124 lbs.
1-DB1	531 lbs.	531 lbs.
1-DC1	535 lbs.	535 lbs.
1-DD1	538 lbs.	538 lbs.
1-DE1	542 lbs.	542 lbs.
1-DF1	489 lbs.	489 lbs.
1-DG1	490 lbs.	490 lbs.
1-DH1	490 lbs.	490 lbs.
1-DJ1	492 lbs.	492 lbs.
Field Bolts:		3,591 lbs.
	Total	27,922 lbs.
Field Splice No. 1 & 2, Girders A,B,C	573 lbs.	3,438 lbs.
Field Splice No. 1, Girders D,E	536 lbs.	1,072 lbs.
Field Splice No. 2, Girders D,E	605 lbs.	1,210 lbs.
Girders:		
1-A1	111'-7 1/2"	20,182 lbs.
1-B1	107'-4 1/2"	19,645 lbs.
1-C1	103'-0 1/2"	18,965 lbs.
1-D1	98'-7 1/2"	16,891 lbs.
1-E1	94'-1 1/2"	15,992 lbs.
1-A2	97'-1 1/2"	16,042 lbs.
1-B2	97'-9 1/2"	16,546 lbs.
1-C2	95'-5 1/2"	16,223 lbs.
1-D2	94'-2 1/2"	16,051 lbs.
1-E2	94'-1 1/2"	15,985 lbs.
1-A3	111'-4 1/2"	20,074 lbs.
1-B3	113'-1 1/2"	20,766 lbs.
1-C3	119'-5 1/2"	23,071 lbs.
1-D3	123'-1 1/2"	25,947 lbs.
1-E3	126'-6 1/2"	26,458 lbs.

FINAL
FOR CONSTRUCTION

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
7/28/97
DATE BY DLR

CAPITAL CONTRACTORS, INC. CAPITAL STEEL DIVISION, LINCOLN, NE 1001 N. 27TH STREET, BOX 00000, ZIP 68501, TEL. 402-478-1001			
APPROVAL DATE 7-17-97	CONTRACTOR INDUSTRIAL BUILDERS		
DESIGN NO.	STRUCTURE 3-SPAN W.P.A. BRIDGE (SITE 2)		
LOCATION	BURLINGTON NORTHERN OVERSEAS RY		
FIELD NO.	CUSTOMER NORTH DAKOTA DEPARTMENT OF TRANSPORTATION		
PAINT	PROJECT RSN-107-002(041)053	COUNTY	WILLIAMS
DRAWN 7-14-97 G. CLARK	STATION 124 + 43.6	ROADWAY	30 FT.
CHECKED	LOADING	WEEKS	SKIFF
BRIDGE NO. 2-053.731R	SHEET NO. 1	OF 1	ORDER NO. 42148

CONTRACT NO.
DESCRIPTION
BILL OF WEIGHTS



A = 111'-3 3/8"
B = 107'-0 1/8"
C = 102'-8 1/8"
D = 98'-3 3/8"
E = 93'-9 1/8"

A = 94'-0 1/8"
B = 97'-8 1/8"
C = 95'-4 1/8"
D = 94'-1 1/8"
E = 94'-10 1/8"

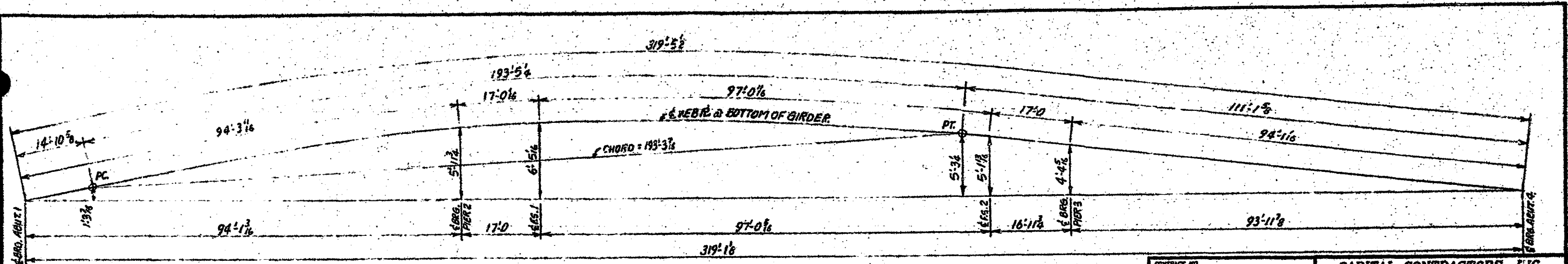
A = 111'-1 1/8"
B = 113'-7 1/8"
C = 119'-1 1/8"
D = 123'-7 1/8"
E = 126'-2 1/8"

BLOCKING DIAGRAM

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
7/28/97
DATE BY DLR

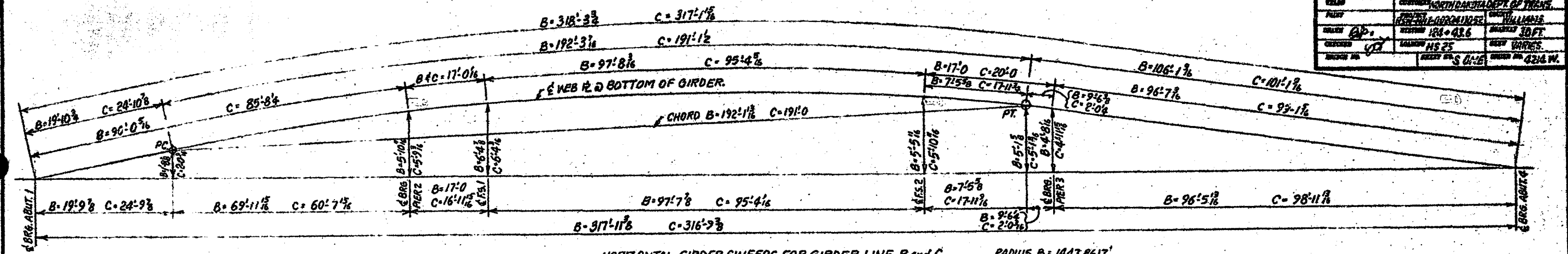
CONTRACT NO.
DESCRIPTION STEEL PLACING PLAN
and BLOCKING DIAGRAM

CAPITAL CONTRACTORS, INC. CAPITAL STEEL DIVISION, LINCOLN, NE 1001 N. 2ND STREET, BOX 60000, LIN. 685-070-0001			
APPROVAL DATE	7-17-97	CONTRACTOR	INDUSTRIAL BUILDERS
C.A.	STRUCTURE	3-SPAN W.P.G. BRIDGE (SITE 2)	
DESIGN NO.			
FIELD			
PIERS			
SPANS			
BRIDGE			
ROADWAY			
SPANS			
BRIDGE			
ROADWAY			
SPANS			
BRIDGE			
ROADWAY			

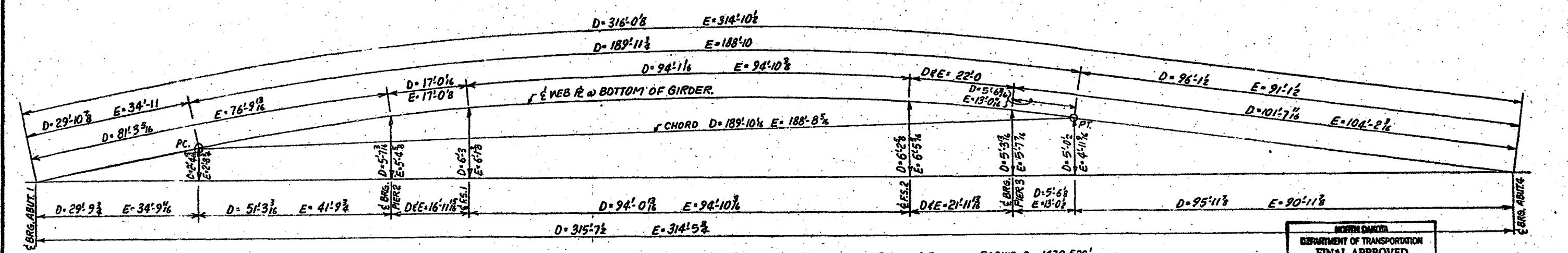


HORIZONTAL GIRDER SWEEPS FOR LINE A. RADIUS = 1456.5283'

CONTRACT NO.		CAPITAL CONTRACTORS, INC.	
DESCRIPTION		CAPITAL STEEL DIVISION, LINCOLN, NE	
HORIZONTAL GIRDER SWEEPS.		1001 N. 7TH STREET, 2ND FLOOR, 2ND FLOOR, LIN. 400-279-1201	
APPROVAL DATE	1-17-97	CONTRACT NO.	INDUSTRIAL BUILDERS
DESIGN NO.	3-SPAN WPA BRIDGE (SITE 2)	CUSTOMER	INDUSTRIAL BUILDERS
LOCATION	NORTH DAKOTA DEPT. OF TRANS.	PROJECT	INDUSTRIAL BUILDERS
PLAN	SECTION 124-43.6	SCALE	1/8" = 1'-0"
CHECKED	MS 25	DATE	1/28/97
DESIGNED BY	MS 25	DATE	1/28/97

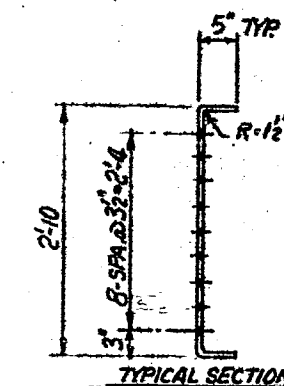
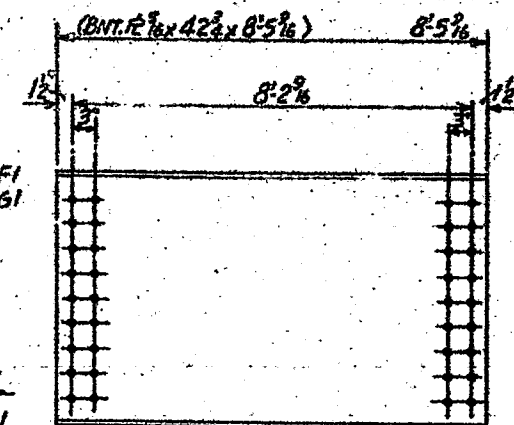
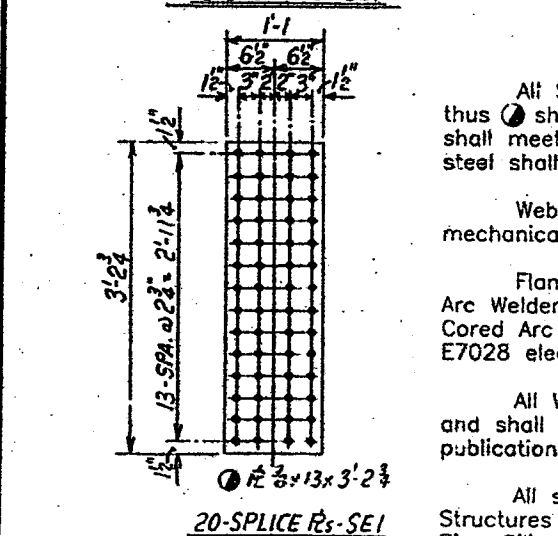
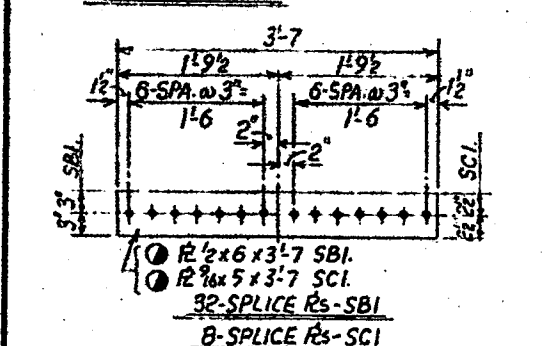
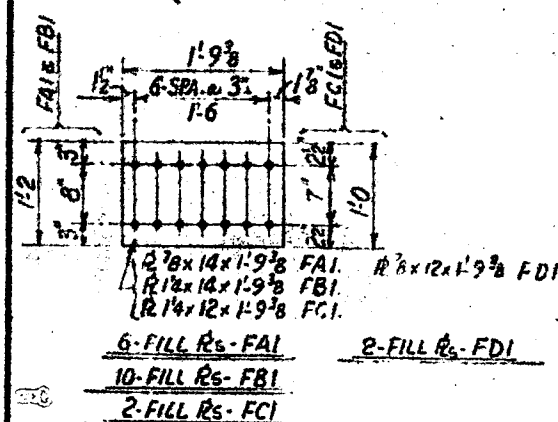
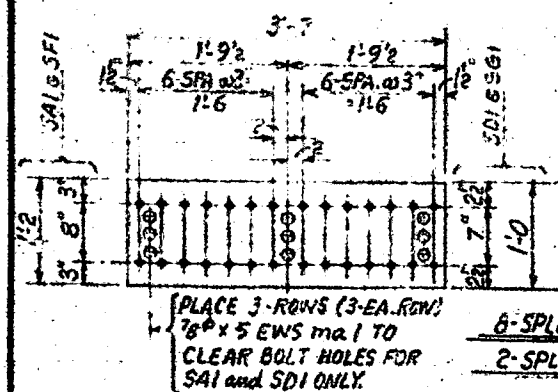


HORIZONTAL GIRDER SWEEPS FOR GIRDER LINE B and C. RADIUS B = 1447.8617' RADIUS C = 439.195'



HORIZONTAL GIRDER SWEEPS FOR GIRDER LINE D and E. RADIUS D = 1430.528' RADIUS E = 1421.862'

NORTH DAKOTA	
DEPARTMENT OF TRANSPORTATION	
FINAL APPROVED	
DATE	1/28/97
BY	DLR
FINAL	



NOTES

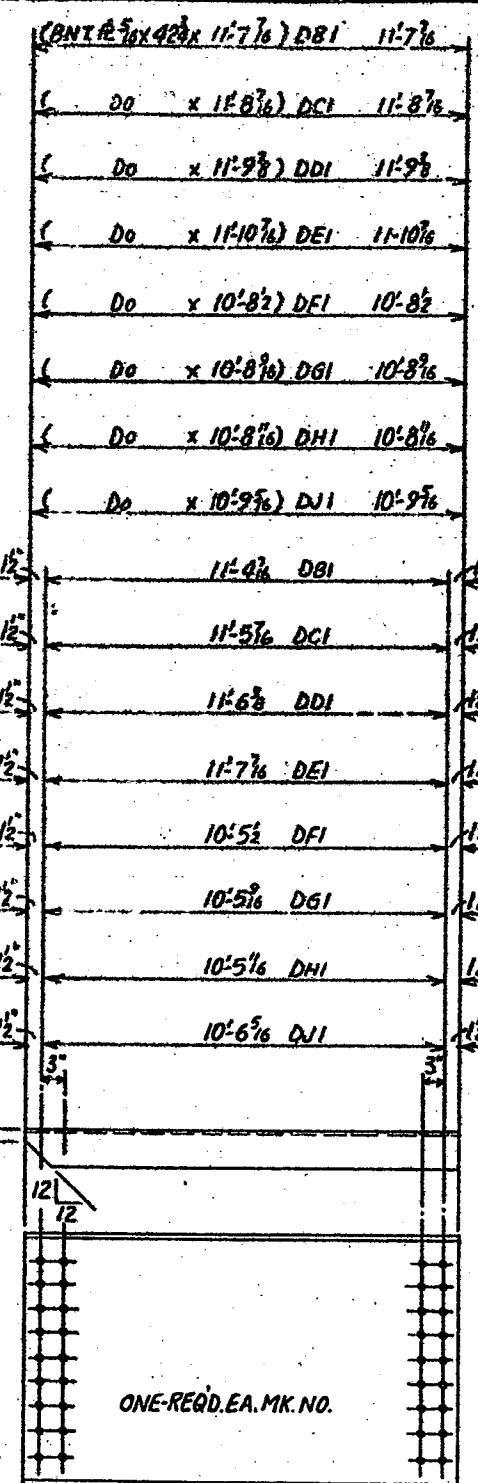
All Steel shall conform to the requirements of A.A.S.H.T.O. M270 Grade 50T2 and shall meet the longitudinal Charpy V-notch test of 15 ft. lbs. @ 40° F. All other steel shall be AASHTO M270 Grade 36.

Web plates shall be cut to camber using the "Automatic Torch" with a mechanical guide. See shop drawing details.

Flange plate to web plate welds shall be by the "Automatic Submerged Arc Welder". All other welds shall be by the "Submerged Arc Process," "Flux-Cored Arc Process," or by the Shielded Metal Arc Process," using E7018, or E7028 electrodes.

All Welding shall be done in accordance with the Standard Specifications and shall conform to the American Welding Society and A.A.S.H.T.O. joint publication D1.5-95 and approved Capital Steel Welding procedures.

All surfaces to be painted shall be blast-cleaned in accordance with Steel Structures Painting Council Specification SSPC-SP10. Apply one coat of Inorganic Zinc-Silicate primer with a dry film thickness a minimum of 3.0 mils and a finish coat of Aliphatic Polyurethane (See Specifications).



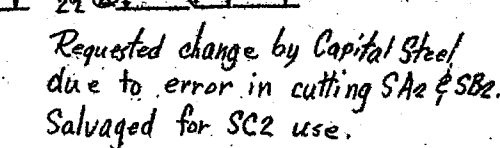
BILL OF MATERIAL					
QTY	DESCRIPTION	UNIT	QTY	DESCRIPTION	UNIT
8	SAI SPLICE PLATES		3	7	① M270 GR. 50T2
8	R ⁵ 8x14		3	7	① M270 GR. 50T2
12	R ⁵ 8x5 EWS				
8	SFI SPLICE PLATES		3	7	① M270 GR. 50T2
8	R ⁵ 8x14		3	7	① M270 GR. 50T2
2	SDI SPLICE PLATES		3	7	① M270 GR. 50T2
2	R ⁵ 8x12		3	7	① M270 GR. 50T2
18	R ⁵ 8x5 EWS				
2	SGI SPLICE PLATES		3	7	① M270 GR. 50T2
2	R ⁵ 8x12		3	7	① M270 GR. 50T2
	SPLICE PLATES				① M270 GR. 50T2
5	SBI R ⁵ 8x6		3	7	
8	SCI R ⁵ 8x5		3	7	
20	SEI R ⁵ 8x13		3	2	
	FILL PLATES M270 GR. 36				
6	FAI R ⁵ 8x18		1	9	
10	FBI R ⁵ 8x14		1	9	
2	FCI R ⁵ 8x12		1	9	
2	FDI R ⁵ 8x12		1	9	
	DIAPHRAGM PLATE M270 GR. 36				
52	DAI BNT R ⁵ 8x42 1/2		8	5	
ONE	DBI		11	7 1/2	
ONE	DCI		11	8 1/2	
ONE	DDI		11	9 1/2	
ONE	DEI		11	10 1/2	
ONE	DFI		10	8 1/2	
ONE	DGI		10	8 1/2	
ONE	DHI		10	8 1/2	
ONE	DJI		10	9 1/2	

FINAL
FOR CONSTRUCTION

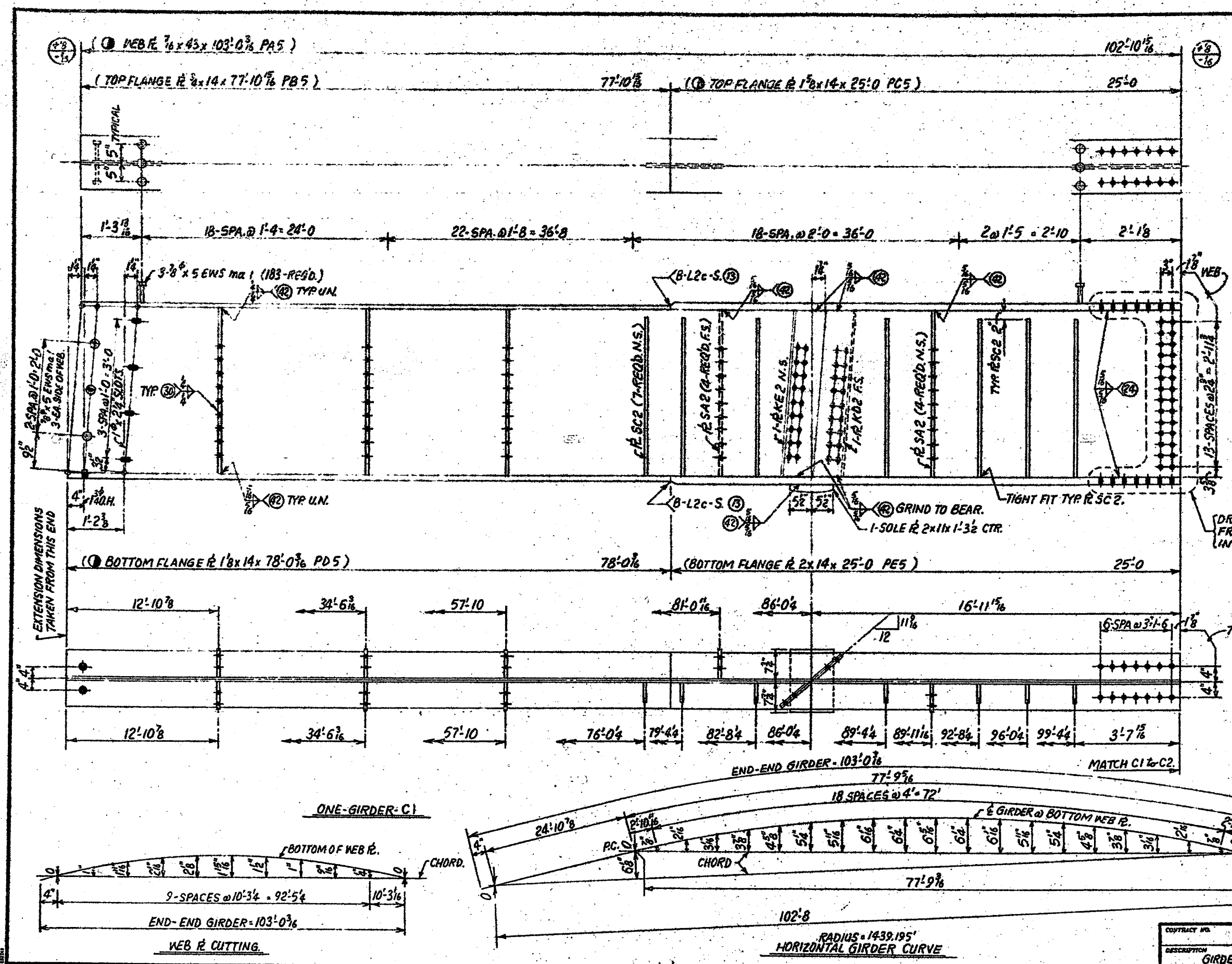
NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
DATE 1/26/77 BY JLR

CONTRACT NO.
DESCRIPTION SPLICE PLATES
NOTES and DIAPHRAGMS.

CAPITAL CONTRACTORS, INC.
CAPITAL STEEL DIVISION, LINCOLN, NE
1001 N. 2ND STREET, SUITE 200, LINCOLN, NE 68502-4700
APPROVED DATE 1-17-77
SHEET NO. 1 OF 1
PROJECT 3-SPAN M.P.G. BRIDGE (SITE 2)
DRAWN BY JLR
CHECKED BY JLR
DATE 1-26-77
SCALE 1/2" = 1'-0"



NORTH DAKOTA DEPARTMENT OF TRANSPORTATION FINAL APPROVED DRAWING 7/22/97 DATE BY <i>DJR</i>		FINAL FOR CONSTRUCTION	
CONTRACT NO.		CAPITAL CONTRACTORS, INC. CAPITAL STEEL DIVISION, LINCOLN, NE 1801 N. NEW STREET, BOX 60000, NEP 68501, TEL. 402-770-1907	
APPROVAL DATE 7-17-97	CONTRACT TYPE INDUSTRIAL BUILDING	PROJECT NO.	
D.R. # 16	STRUCTURE 3-SPAN WFG BRIDGE (SIDE 2)	LOCATION MINNAPOLIS INDUSTRIAL PARK, EDEN PRAIRIE	
DESIGN NO.	DRAWING NO.	CLIENT NORTH DAKOTA DEPT. OF TRANS.	
WEIGHT	CONTRACT NO.	COUNTY WILLAMETTE	
PLANT SEE NOTES	PROJECT NO.	COUNTY WILLAMETTE	
DRAWN G.S.P.	STRUCTURE 124 + 43.6	ELEVATION 20 FT	
CHECKED G.P.	ELEVATION HS 25	ROAD VARIATION NONE	
DESIGNER NO.	COUNTY NO.	COUNTY NO.	
2-053-731R	2	2	
DESCRIPTION STIFFENER PLATES.		COUNTY NO.	
2-053-731R		2	



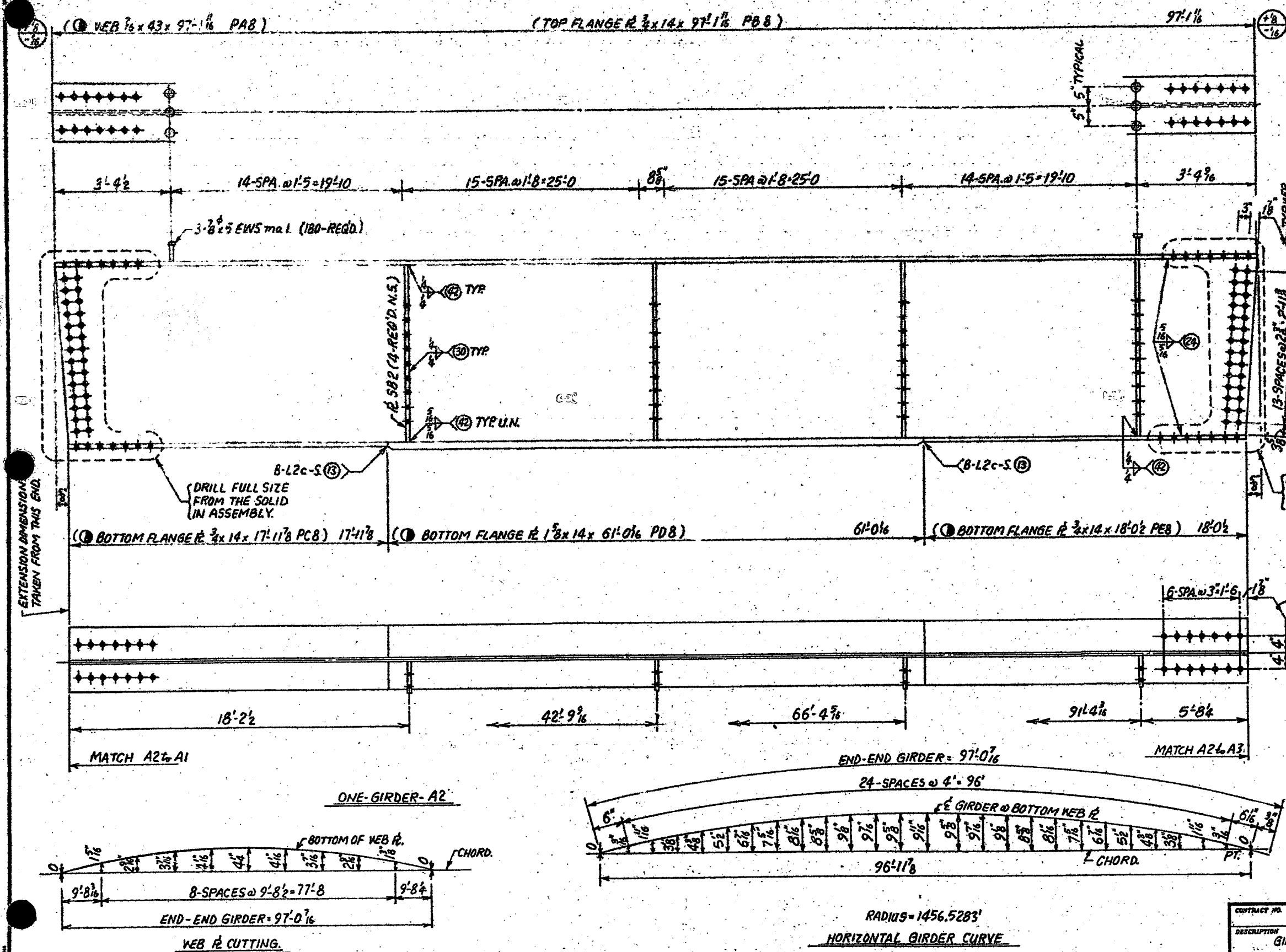
BILL OF MATERIALS				
NO.	ITEM	QUANTITY	UNIT	REMARKS
GIRDER				
1	PA5 R 1/2 x 14	163.01	LB	M270 GR 50 T2
1	PD5 R 1/2 x 14	77.10	LB	M270 GR 50
1	PC5 R 1/2 x 14	85.0	LB	M270 GR 50 T2
1	PE5 R 1/2 x 14	78.01	LB	M270 GR 50 T2
1	FE5 R 2 x 14	25.0	LB	M270 GR 50
189	1/8" x 5 EWS			
1	SOLE R 2 x 11	1.31	LB	
1	K02 R 1/8 x 10 1/2	3.71	LB	M270 GR 50
1	KE2 R 1/8 x 10 1/2	3.71	LB	M270 GR 50
8	SA2 R 2 x 7	3.7	LB	M270 GR 50
7	SC2 R 1/2 x 5	3.5	LB	M270 GR 50

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
1/23/97
DATE

FINAL
FOR CONSTRUCTION

FOR NOTES SEE SHEET NO. ONE.			
CAPITAL CONTRACTORS, INC.			
CAPITAL STEEL DIVISION, LINCOLN, NE			
1401 N. 10TH STREET, SUITE 100, LINCOLN, NE 68502-1001			
APPROVAL DATE	7-17-97	CONTRACT NO.	INDUSTRIAL BUILDERS
PROJECT NO.	3-SPAN W/RA BRIDGE (SITE 2)	DESCRIPTION	3-SPAN W/RA BRIDGE (SITE 2)
DESIGN NO.		LOCATION	BUILDING NORTH OF CHURCH ST.
SCALE AS NOTED		CONTRACTOR	NORTH DAKOTA DEPT OF TRANS.
PROJECT	SEE NOTES	DESIGNER	WILLIAMS
CALCULATED BY	DP	CHECKED	124-416
CONTRACT NO.		DESIGNED BY	HS 25
REVISION NO.		REVISION	5

CONTRACT NO.
DESCRIPTION
GIRDER C1 DETAILS.



BILL OF MATERIAL				STANDARD	
QTY	DESCRIPTION	QTY	ADD	QTY	STANDARD
ONE A2	GIRDER				
1	PAB $\frac{1}{2} \times 43$	97' 1 $\frac{1}{8}"$		1	M210 GR. 50 T2
1	PB8 $\frac{1}{2} \times 14$	97' 1 $\frac{1}{8}"$		1	M210 GR. 50
1	PB8 $\frac{1}{2} \times 14$	17' 11 $\frac{1}{8}"$		1	M210 GR. 50 T2
1	PB8 $\frac{1}{2} \times 14$	61' 0 $\frac{1}{8}"$		1	M210 GR. 50 T2
1	PB8 $\frac{1}{2} \times 14$	18' 0 $\frac{1}{2}"$		1	M210 GR. 50 T2
180	$\frac{3}{8} \times 5$ EWS				
4	S82 $\frac{1}{2} \times 7$	3' 7"			M210 GR. 50

NORTH DAKOTA
 DEPARTMENT OF TRANSPORTATION
 FINAL APPROVED
 DRAWING
 7/14/47
 DLR

FINAL
 FOR CONSTRUCTION

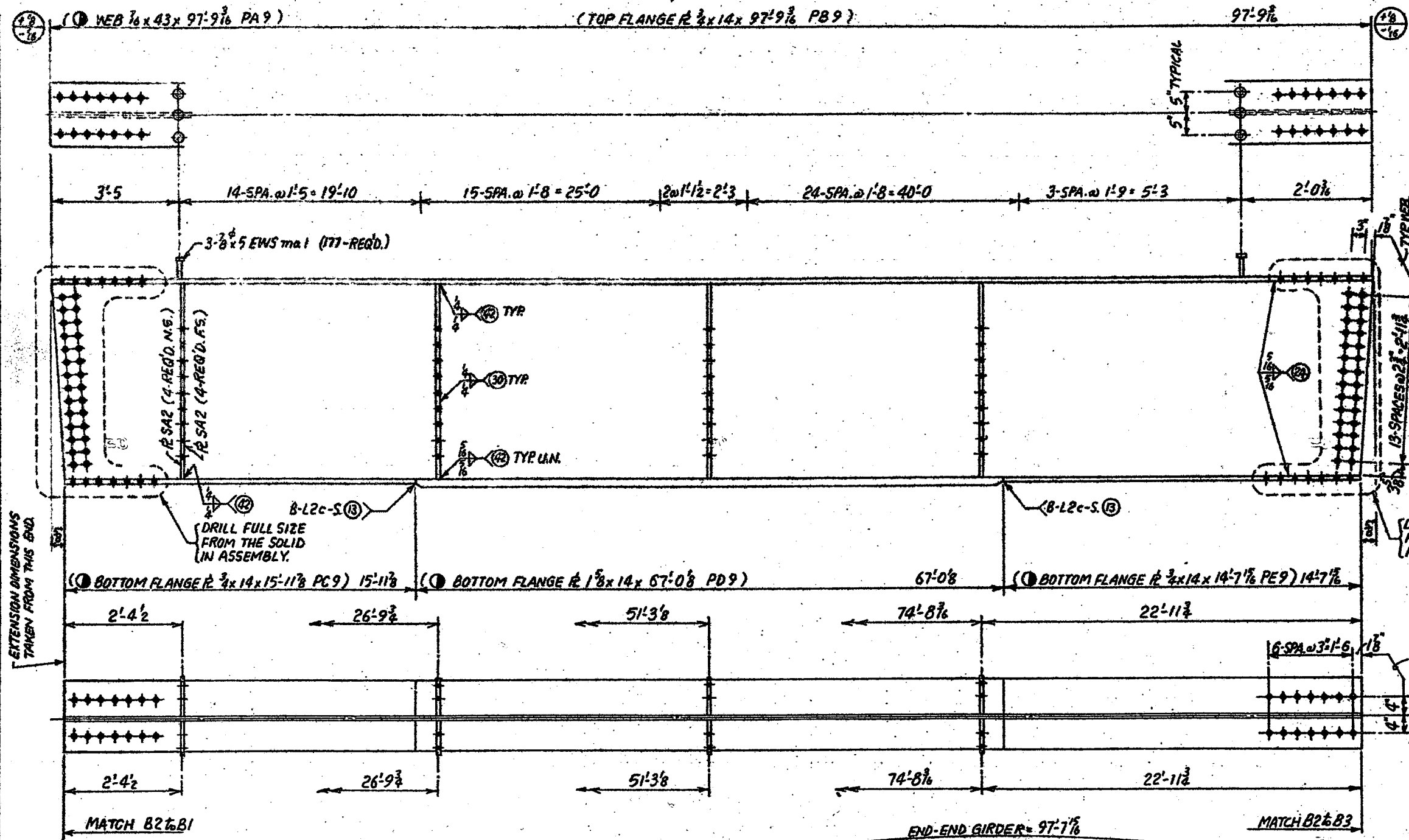
FOR NOTES SEE SHEET NO. ONE.

CAPITAL CONTRACTORS, INC.
 CAPITAL STEEL DIVISION, LINCOLN, NE
 1001 N. 2ND STREET, BOX 6000, LINCOLN, NE 68506-4000

APPROVAL DATE: 7-17-47
 DATE: 7/15/47
 DRAWN BY: DLR
 CHECKED BY: GCF
 BRIDGE NO. 8

CONTRACT NO. 124-436
 DESCRIPTION: GIRDER A2 DETAILS

INDUSTRIAL BUILDERS
 3-SPAN WLG. BRIDGE (SITE)
 NORTH DAKOTA DEPT. OF TRANS.
 SPECTER, JR. (LINDSEY)
 WILLIAMS
 SOFT.
 VARIES
 4214 N.



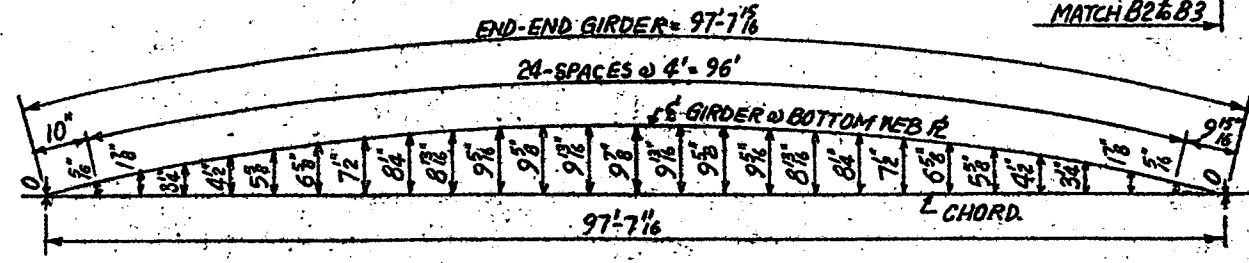
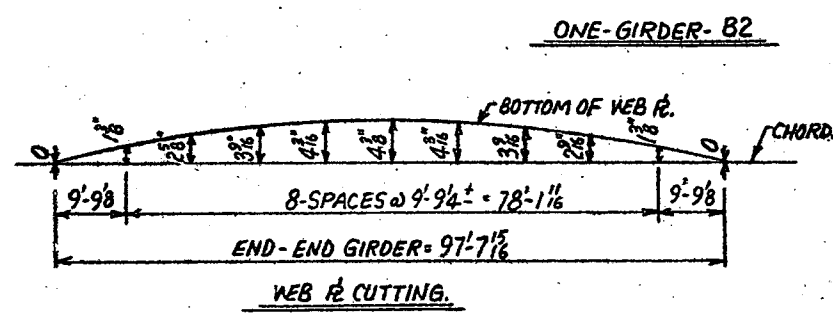
BILL OF MATERIAL			
NO.	DESCRIPTION	QTY	REMARKS
ONE B2 GIRDER			
1	PA9 1/2 x 14	97' 9 1/8"	M210 GR 50 T2
1	PD9 1/2 x 14	97' 9 1/8"	M210 GR 50
1	PD9 1/2 x 14	15' 1 1/8"	M210 GR 50 T2
1	PD9 1/2 x 14	67' 0 1/8"	M210 GR 50 T2
1	PD9 1/2 x 14	14' 7 1/8"	M210 GR 50 T2
177	3/8 x 5 EWS		
8	SA2 1/2 x 7	3' 7"	M210 GR 50

EXTENSION DIMENSIONS
TAKEN FROM THIS END.

DRILL FULL SIZE
FROM THE SOLID
IN ASSEMBLY.

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
7/29/97
DATE

FINAL
FOR CONSTRUCTION

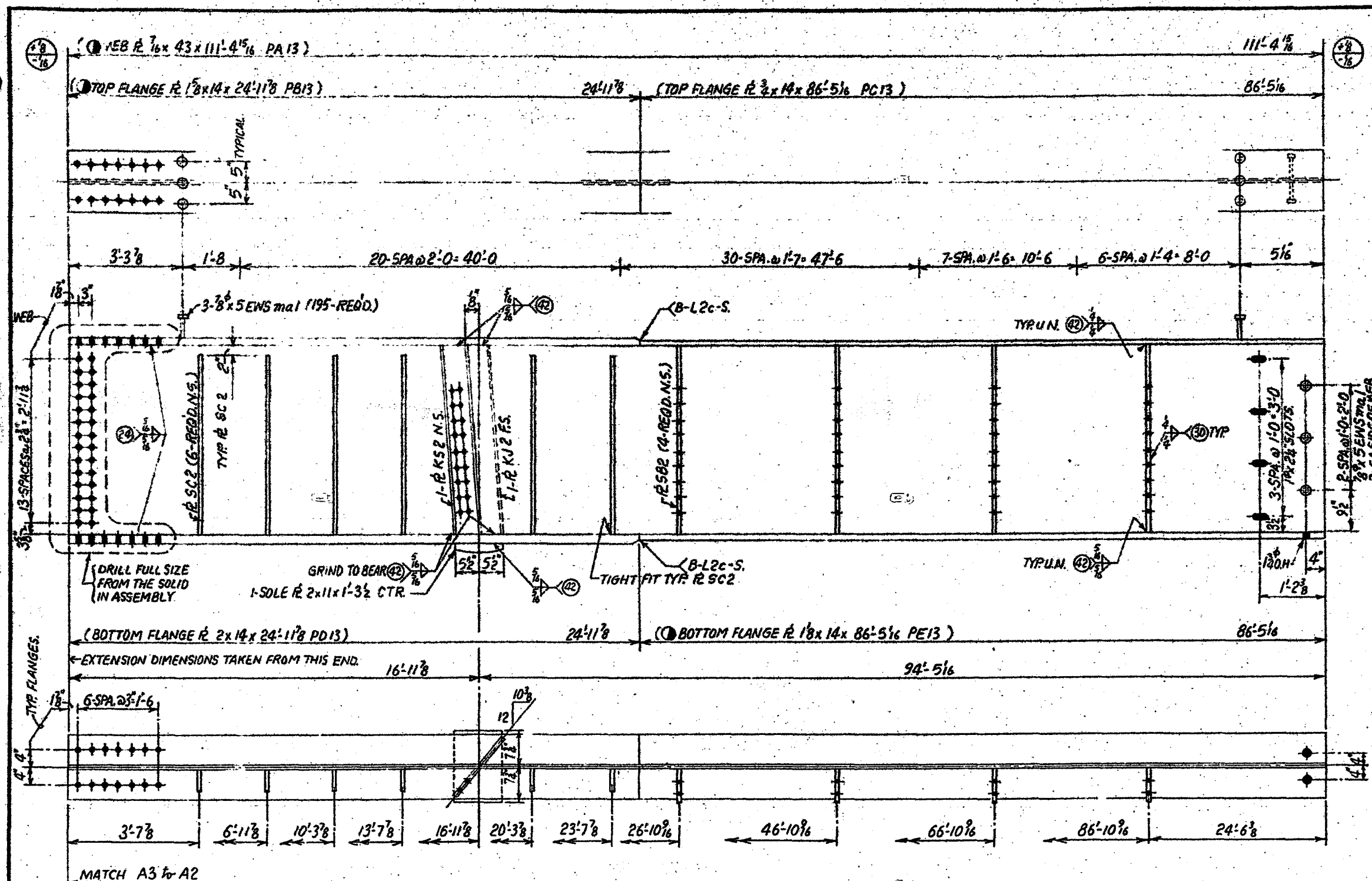


FOR NOTES SEE SHEET NO. ONE.

CAPITAL CONTRACTORS, INC.
CAPITAL STEEL DIVISION, LINCOLN, NE
1001 N. 2ND STREET, 4TH FLOOR, LINCOLN, NE 68502-1001
TELEPHONE 781-1777 FAX 781-1777

APPROVAL DATE 7-17-97
DESIGNED BY 3-SHAFT BRIDGE (SITE 2)
CHECKED BY
REVIEWED BY
CONTRACT NO. 124-436
PROJECT NO. 124-436
SHEET NO. 9

CONTRACT NO.
DESCRIPTION
GIRDERS B2 DETAILS



BILL OF MATERIAL				
NO.	QTY.	DESCRIPTION	ADD.	REMARKS
ONE A3		GIRDER		
1	111	$\frac{1}{2} \times 43$	4	M270 GR. 50 T2
1	86	$\frac{1}{2} \times 14$	11	M270 GR. 50 T2
1	86	$\frac{1}{2} \times 14$	5	M270 GR. 50
1	24	$\frac{1}{2} \times 14$	11	M270 GR. 50
1	86	$\frac{1}{2} \times 14$	5	M270 GR. 50 T2
201		1" x 5 EWS		
1	1	SOLE R 2 x 11	3	
1	3	$\frac{1}{2} \times 10$	7	M270 GR. 50
1	3	$\frac{1}{2} \times 10$	7	M270 GR. 50
4	3	$\frac{1}{2} \times 7$	7	M270 GR. 50
6	3	$\frac{1}{2} \times 5$	5	M270 GR. 50



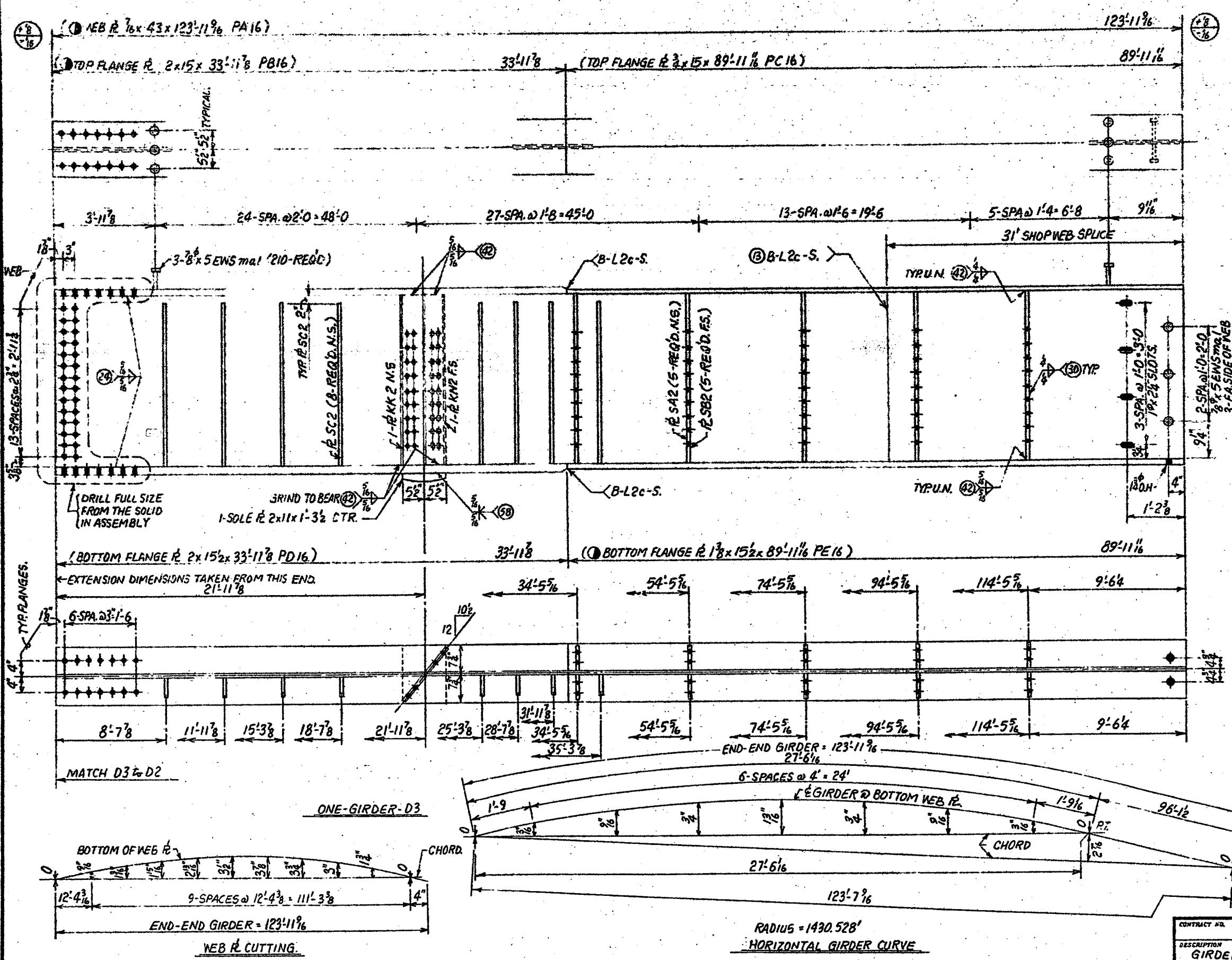
SHOP NOTE: HORIZONTAL GIRDER CURVING NOT REQUIRED FOR THIS GIRDER.

NORTH DAKOTA
 DEPARTMENT OF TRANSPORTATION
 FINAL APPROVED
 DRAWING
 1/28/97
 DATE BY DLR

CONTRACT NO.
 DESCRIPTION
 GIRDER A3 DETAILS

CAPITAL CONTRACTORS, INC.
 CAPITAL STEEL DIVISION, LINCOLN, NE
 1001 N. 6TH STREET, BOX 9000, LINCOLN, NE 68504, TEL. 402-479-1001
 APPROVAL DATE 7-17-97
 STRUCTURE 3-SPAN W.P.G. BRIDGE (SITE 2)
 PROJECT NORTH DAKOTA DEPT. OF TRANS.
 DRAWN BY RPT
 CHECKED BY V
 BRIDGE NO. 13
 ORDER NO. 4212 W

FINAL FOR CONSTRUCTION



BILL OF MATERIAL				
QTY	ITEM	UNIT	ADD	REMARKS
ONE	D3 GIRDER			
1	PA16 R 7/8 x 43	123' 11 7/8"		① M270 GR. 50 T2
1	PA16 R 2 x 15	33' 11 7/8"		① M270 GR. 50 T2
1	PC16 R 3/4 x 15	89' 11 1/8"		M270 GR. 50
1	PA16 R 2 x 15 1/2	33' 11 7/8"		M270 GR. 50
1	PA16 R 7/8 x 15 1/2	89' 11 1/8"		① M270 GR. 50 T2
216	max 1/4 x 5 EWS			
1	SOLE R 2 x 11	1 3/4"		
1	KN2 R 1/8 x 10 3/4	3 7/8"		M270 GR. 50
1	KN2 R 1/8 x 10 3/4	3 7/8"		M270 GR. 50
5	SA2 R 1/2 x 7	3 7/8"		M270 GR. 50
5	SB2 R 1/2 x 7	3 7/8"		M270 GR. 50
8	SC2 R 7/8 x 5	3 5/8"		M270 GR. 50

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
FINAL APPROVED
DRAWING
7/28/91
DATE BY DLR

FINAL
FOR CONSTRUCTION

CAPITAL CONTRACTORS, INC.			
CAPITAL STEEL DIVISION, LINCOLN, NE			
1001 N. 2ND STREET, BOX 8000, ZIP 68501, TEL 402-476-1801			
APPROVAL DATE 1-17-97	CONTRACTOR	INDUSTRIAL BUILDERS	
DATE 1-17-97	STRUCTURE	3-SPAN W.P.B. BRIDGE (SITE 2)	
DESIGN NO.	LOCATION	WARRINGTON NORTHERN OVERHEAD	
WELDS AS NOTED	CUSTOMER	NORTH DAKOTA DEPT. OF TRANS.	
PAINT SEE NOTES	DESIGNER	RSC/WHI-DORRANCE	
DRAWN BY PT	SECTION	124+43.6	
CHECKED BY	LOADING	HS 25	
BRIDGE NO.	SPAN NO.	16	
DESCRIPTION		GIRDERS D3 DETAILS	
		BRIDGE NO. 4214 N	

