

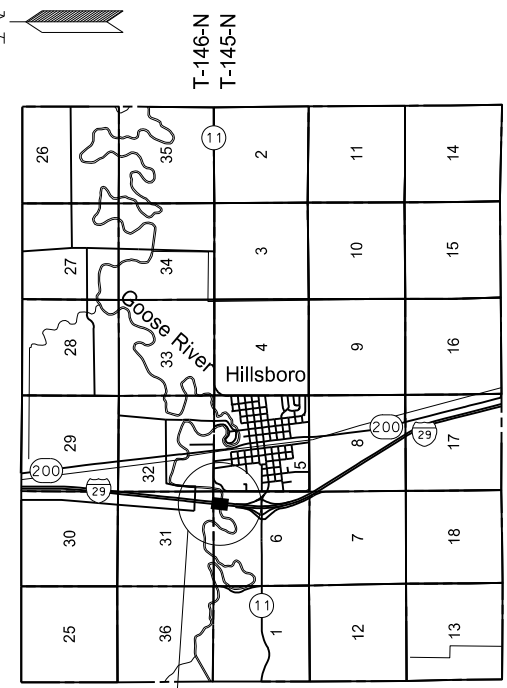
JOB #32 NORTH DAKOTA DEPARTMENT OF TRANSPORTATION

"As-Built"	STATE	PROJECT NO.	PCN	SECTION NO.	SHEET NO.
	ND	SS-6-018(073)152	22015	1	1
		SS-6-015(022)106	22014		
		SS-6-018(074)169	22047		
		SS-6-045(006)011	21999		
		IM-8-029(178)104	22168		

SS-6-018(073)152, SS-6-015(022)106,
SS-6-018(074)169, SS-6-045(006)011, IM-8-029(178)104
Grand Forks County, Griggs County & Traill County
Just N of ND 15 & 3 Mi N of ND 15, 2 Mi W of Northwood,
3 Mi N of US 2, 4 Mi E of ND 65, 4 Mi N of ND 200 S - NB/SB
Deck Overlay, Rail Retrofit & Deck Spall Repair

GOVERNING SPECIFICATIONS:
2014 Standard Specifications adopted by the North Dakota Department of Transportation and the Supplemental Specifications effective on the date the project is advertised.

PROJECT NUMBER \ DESCRIPTION	NET MILES	GROSS MILES
SS-6-018(073)152	N/A	N/A
SS-6-015(022)106	N/A	N/A
SS-6-018(074)169	N/A	N/A
SS-6-045(006)011	N/A	N/A
IM-8-029(178)104	N/A	N/A

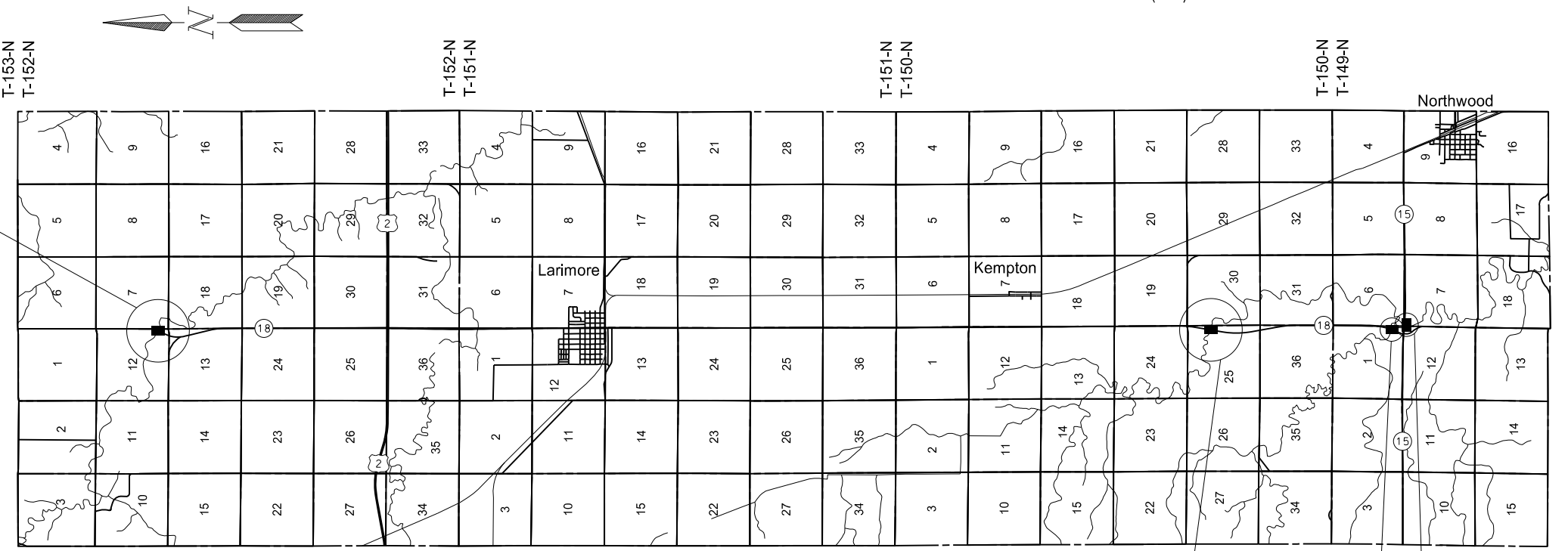


Goose River
Br. No. 29-104.576L&R
IM-8-029(178)104

North Branch Turtle River
Br. No. 18-169.414
SS-6-018(074)169

R-54-W
R-55-W

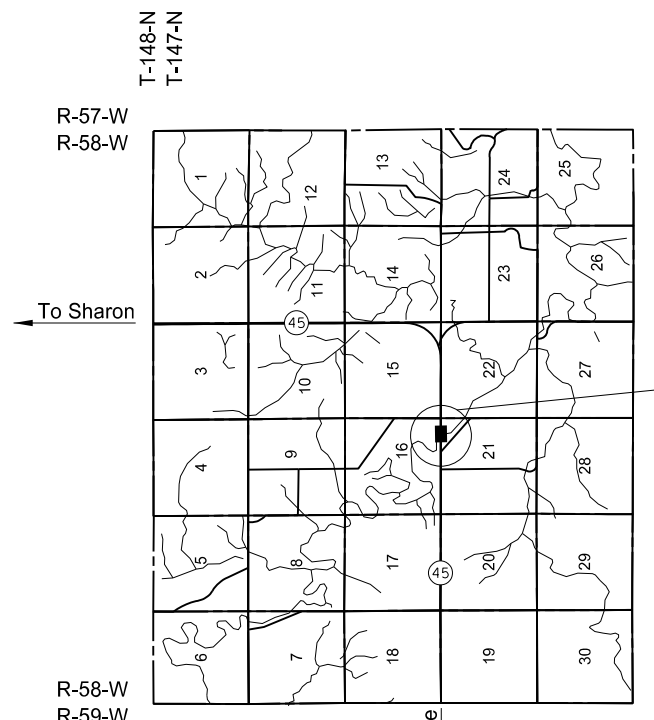
Sheyenne River
Br. No. 45-011.152
SS-6-045(006)011



Little Branch Goose River
Br. No. 18-154.872
SS-6-018(073)152

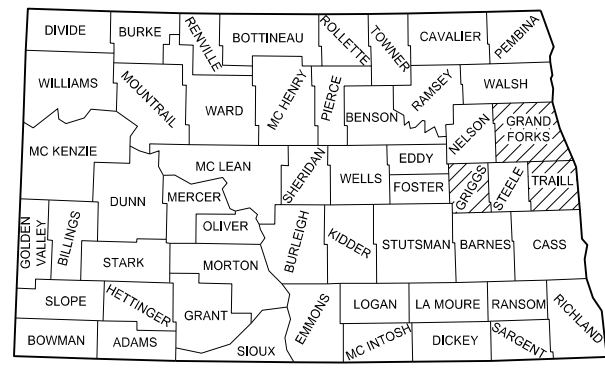
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SS-6-018(073)152

Goose River
Br. No. 15-106.930
SS-6-015(022)106



To Sharon

To Jessie

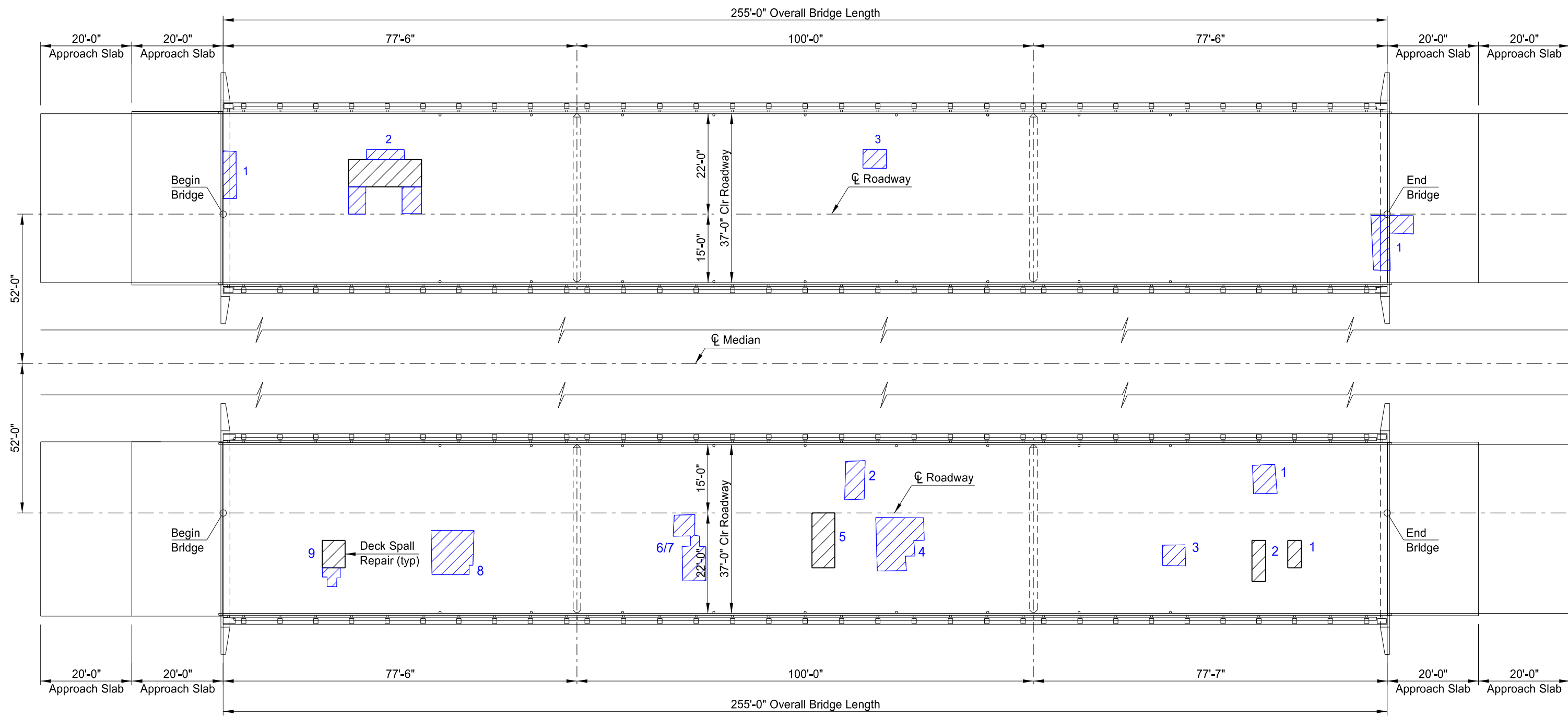
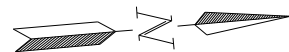


STATE COUNTY MAP

ND DEPARTMENT OF TRANSPORTATION
OFFICE OF PROJECT DEVELOPMENT
Jon Ketterling 02/21/19

BRIDGE DIVISION

This document was originally issued and sealed by
Jon Ketterling,
Registration Number
PE- 4684,
on 02/21/19 and the original document is stored at the
North Dakota Department of Transportation



Hatched areas indicate deck concrete spall areas.

PLAN

NOTES:

- 100 SCOPE OF WORK: Work at this site consists of repairing concrete spall areas on the bridge deck surfaces.
- 650 DECK SPALL REPAIR: The bridge decks have spall areas as shown. Construct the deck spall repair as a Bridge Deck Overlay meeting Section 650. Saw cut the perimeter of the repair area to a depth of 1". Remove concrete to a depth of 2 1/2". The Engineer in the field will determine the actual limits of repair. Include all material, labor, and equipment required to remove the concrete and repair the deck spall area in the bid item "Deck Spall Repair."

BRIDGE BID ITEMS

SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
650	0805	DECK SPALL REPAIR	SF	230 514

Note: Refer to attachments for Driving Lane and Passing Lane Dimensions and Calculations.

This document was originally issued and sealed by Tim L Schwagler, Registration Number PE 3151, on 02/20/19 and the original document is stored at the North Dakota Department of Transportation



NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
GOOSE RIVER
4 NORTH OF 200 SOUTH
BRIDGE LAYOUT
PROJECT: IM-8-029(178)104
TRAILL COUNTY

DATE: 02/21/19
Jon Ketterling
BRIDGE ENGINEER

DESIGN DATA				
Traffic	Average Daily - ND 200, RP 395.560			Max.Hr.
Current (2003)	Pass: 1030	Trucks 170	Total 1200	120
Forecast(2023)	Pass: 1320	Trucks 230	Total 1550	160
Traffic	Average Daily - I-29, RP 104.576			Max.Hr.
Current (2003)	Pass: 7350	Trucks 2425	Total 9775	980
Forecast(2023)	Pass: 12055	Trucks 4220	Total 16275	1630
Minimum Sight Dist. for:	Design Speed - ND 200 65 MPH			
Stopping	Design Speed - I-29 75 MPH			
Safe Passing	Bridges			
Passing for Marking				
Pavement Design Life (years)				

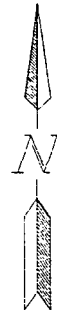
JOB# 14

STATE	PROJECT NO.	PCN	SHEET NO.
ND	HSP-8-999(012)	15417	1

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
IN TRAILL COUNTY
HSP-8-999(012)
Bridge Rail Retrofit, Guardrail, and Incidentals

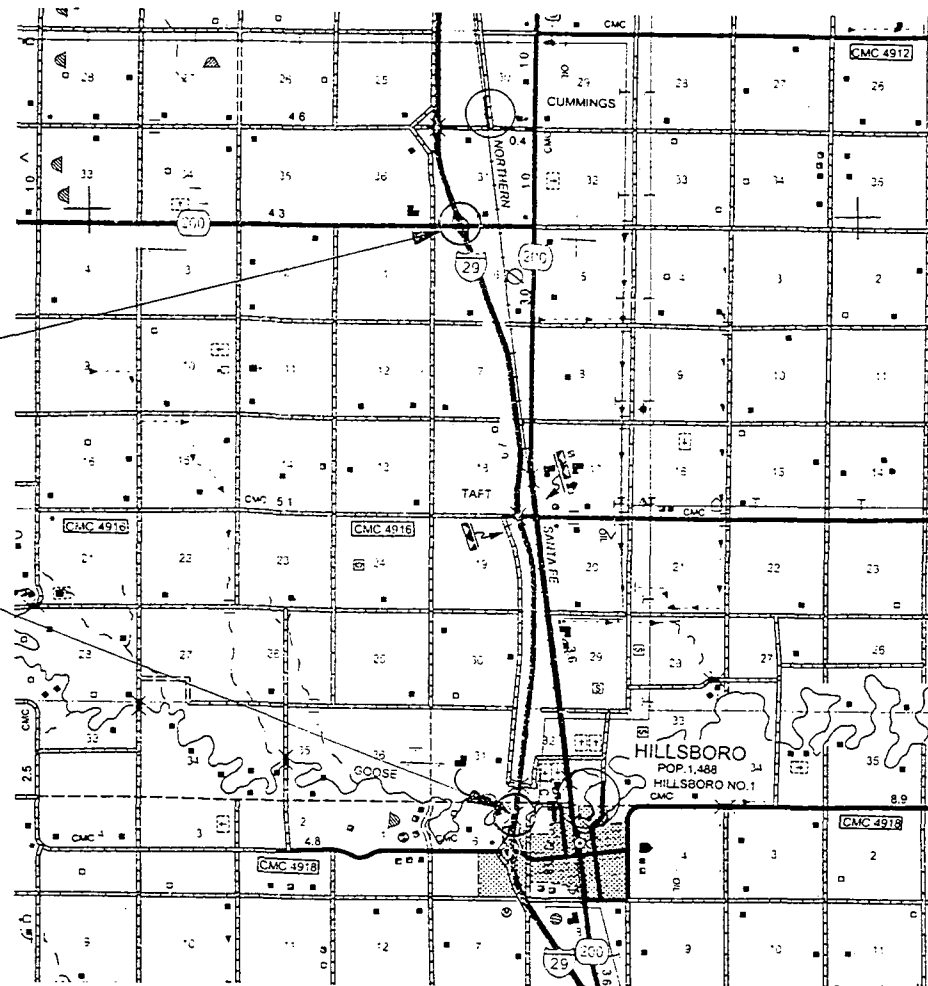
GOVERNING SPECIFICATIONS:

Standard Specifications adopted by the North Dakota Department of Transportation October 2002; Standard Drawings currently in effect; and other Contract Provisions submitted herein.



ND 200 Interchange Overhead
RP 395.560, ND 200
= RP 110.795, I-29

Goose River Bridges
RP 104.576
Northbound and Southbound I-29

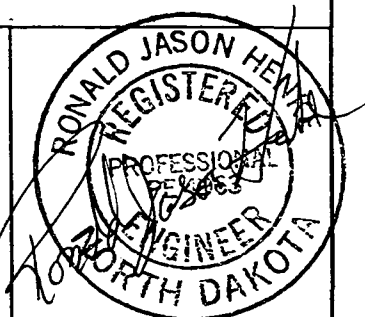


DESIGNER Brad Pfeifer
DESIGNER _____
DESIGNER _____
DESIGNER _____
DESIGNER _____

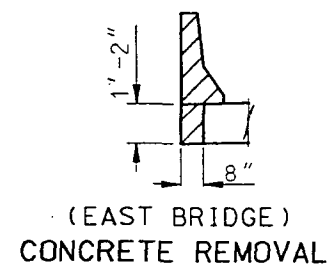
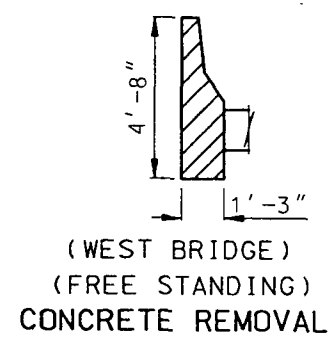
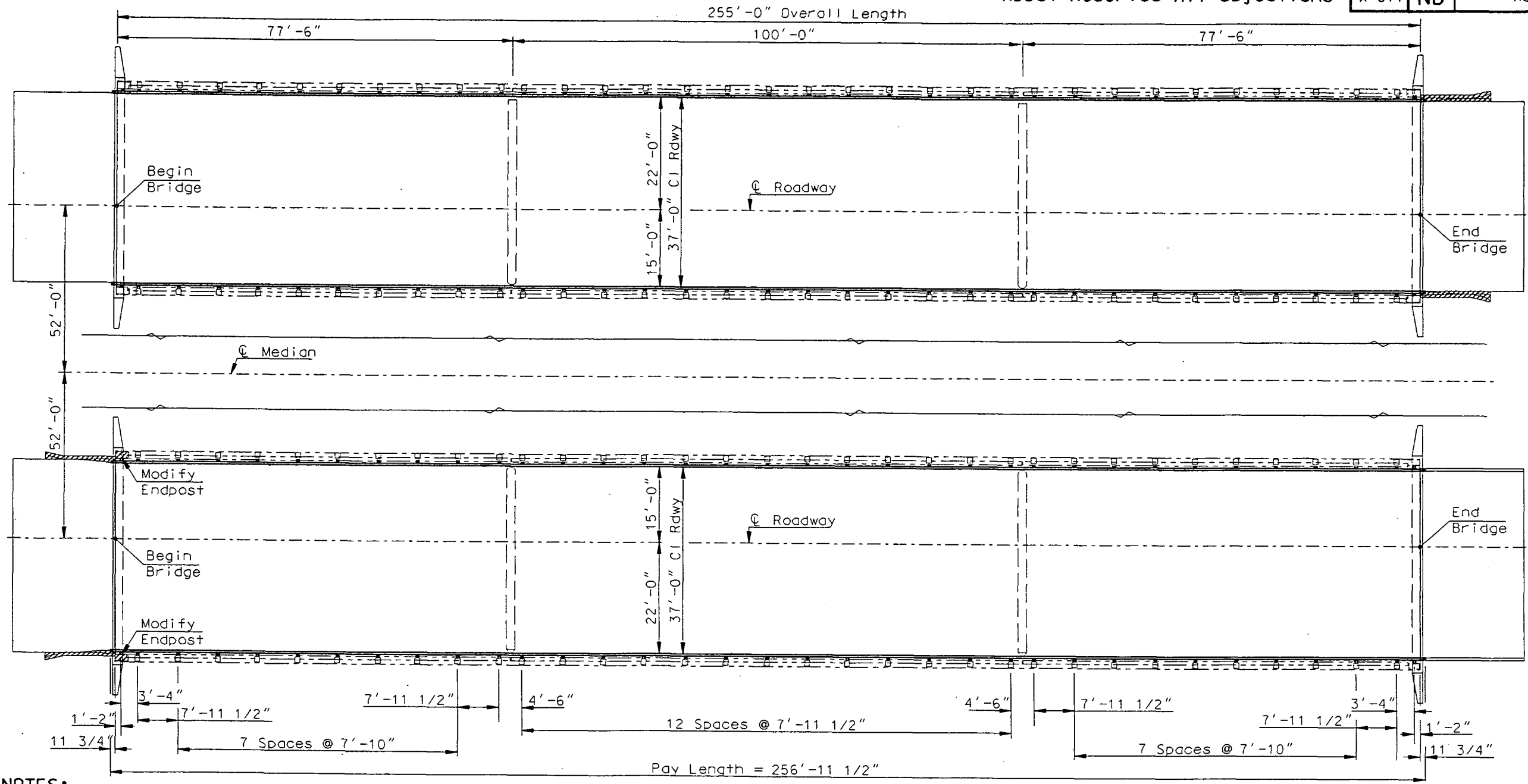
APPROVED DATE _____
DIVISION ADMINISTRATOR
FEDERAL HIGHWAY ADMINISTRATION
U.S. DEPARTMENT OF TRANSPORTATION

APPROVED DATE 5/1/03
Francis J. [Signature]
DIRECTOR,
OFFICE OF PROJECT DEVELOPMENT
ND DEPARTMENT OF TRANSPORTATION

I hereby certify that the attached plans were prepared by me or under my direct supervision and that I am a duly registered professional engineer under the laws of the state of ND.
APPROVED DATE 5/01/03
Ronald Jason Henke
DESIGN ENGINEER
NORTH DAKOTA DEPARTMENT OF TRANSPORTATION



BRIDGE CODE	STATE	PROJECT NUMBER	SHEET NO.
X-071	ND	HSP-8-999(012)	12



PLAN

NOTES:

SCOPE OF WORK: The work at this site consists of removing the safety shape transitions and a portion of the approach slab on the south approach slab of the east bridge, removing the free standing safety shape transitions on the north end of the west bridge and installing a bridge rail retrofit on both bridges and modify the end posts on the south end of the east bridge to accommodate the rail retrofit attachment.

Attachment of the rails to the intermediate posts shall be accomplished by one of the following or any combination of the following methods shown on the detail sheets:

1. Threaded rods and bars
2. U-bolts
3. Anchors

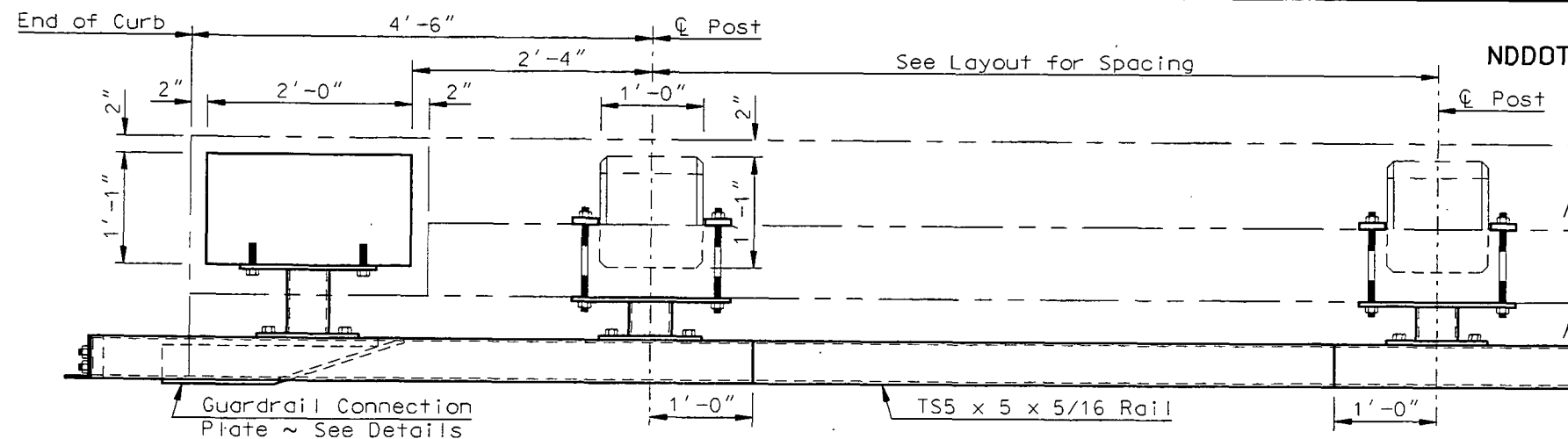
The removal of the safety shape transitions and the portion of the approach slab as shown shall be included in the price bid for "Remove Concrete Safety Shape Transition."

All material removed shall become the property of the contractor and shall be disposed of off the right of way.

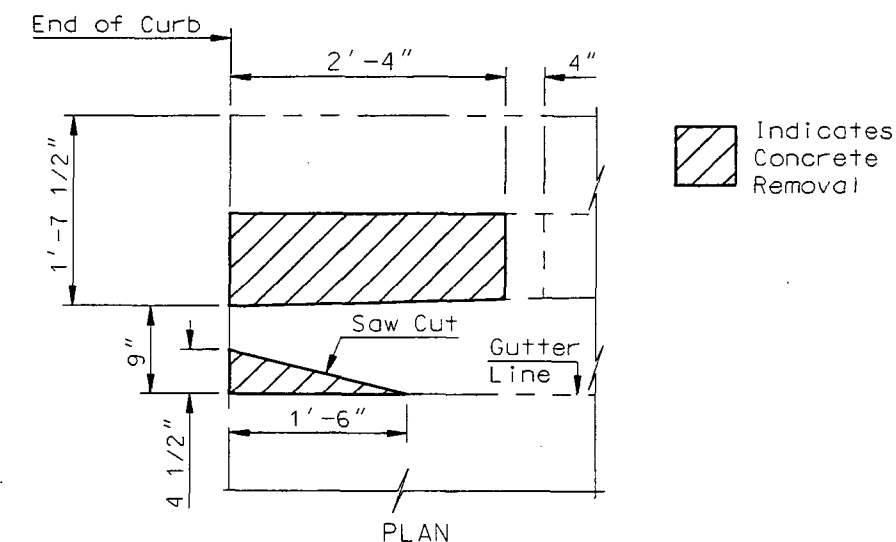
SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
602	1210	BRIDGE END POST MODIFICATION	EA	2
624	3002	DOUBLE BOX BEAM RAIL RETROFIT - E-RAIL	LF	1027.8
764	1990	REMOVE CONCRETE SAFETY SHAPE TRANSITION	EA	4

GOOSE RIVER

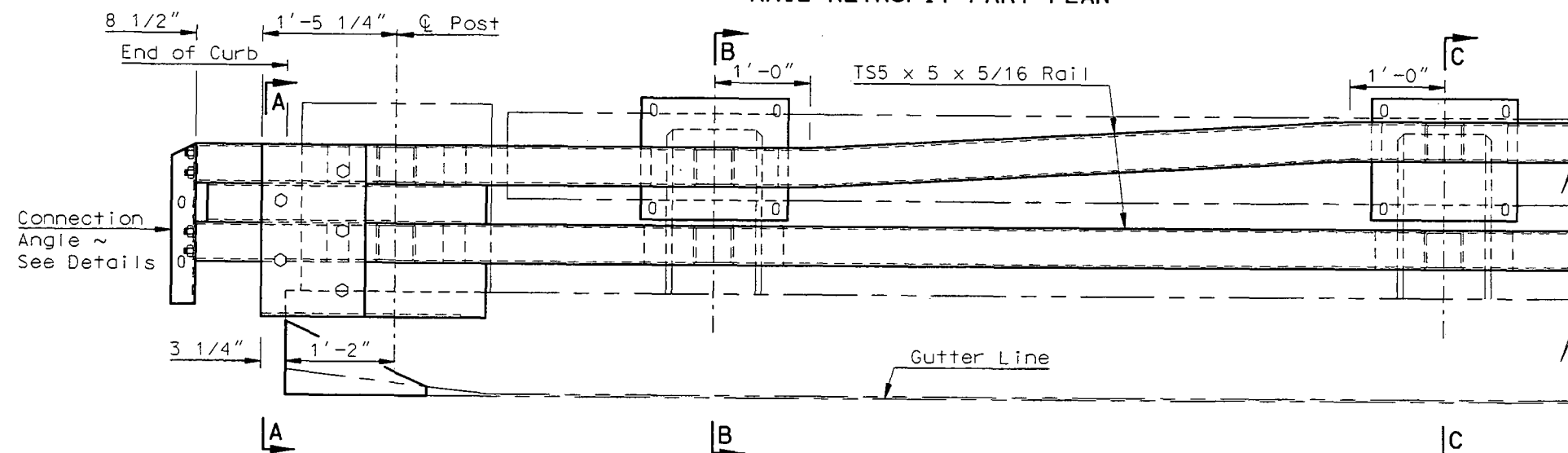
BRIDGE LAYOUT



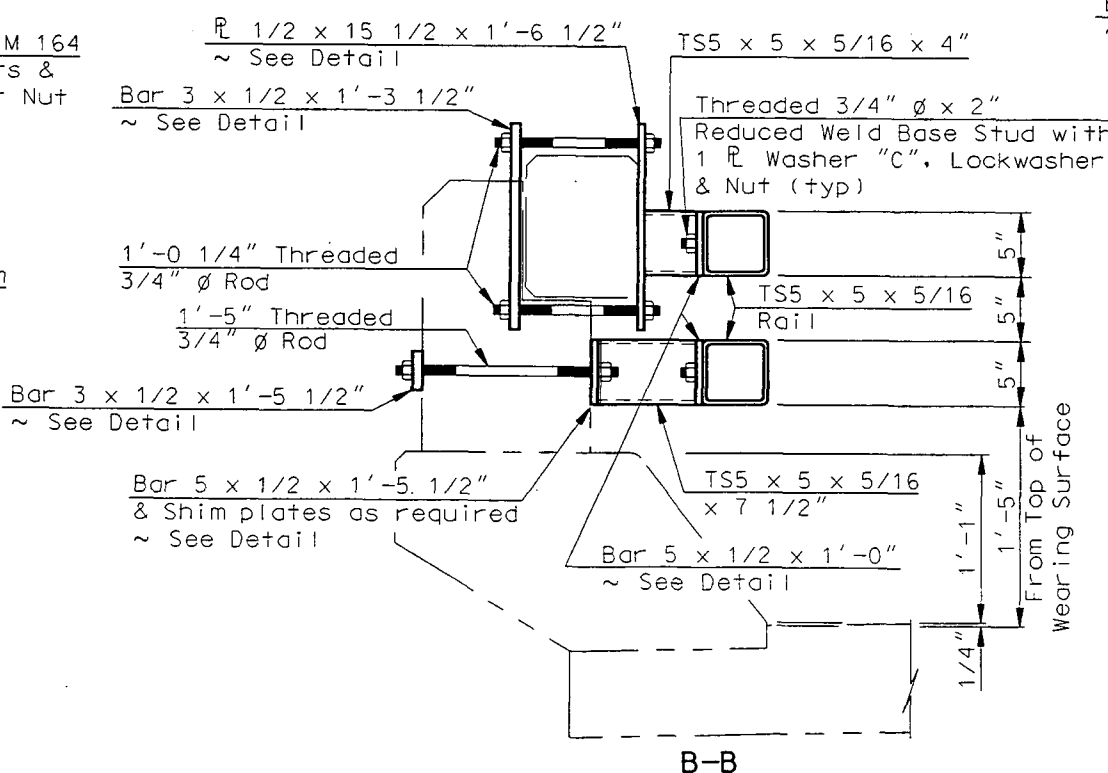
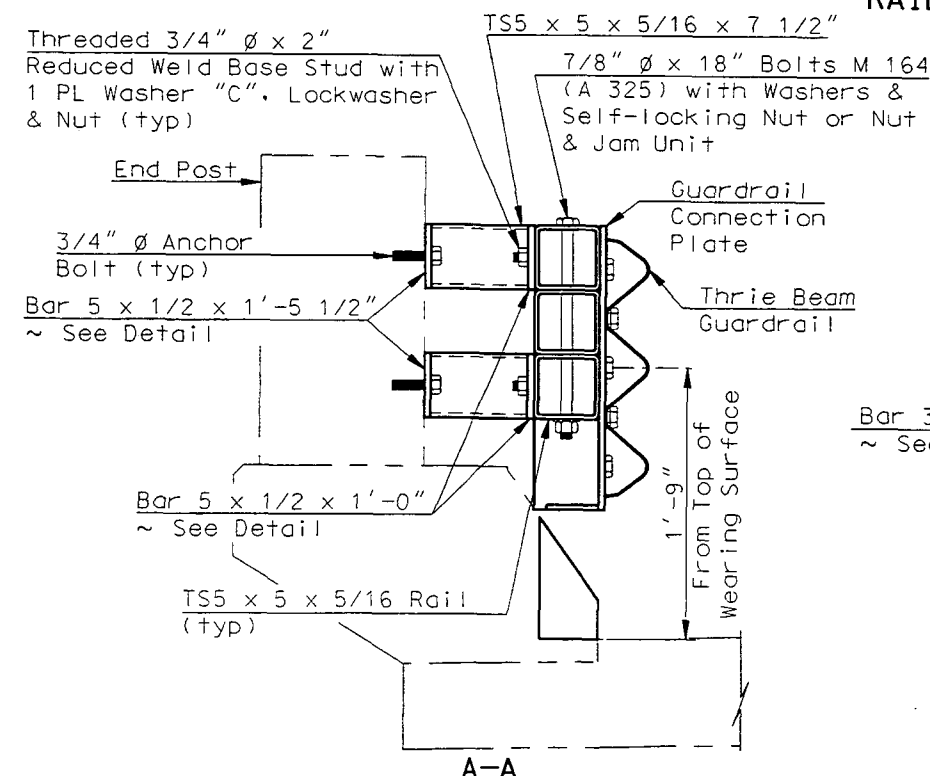
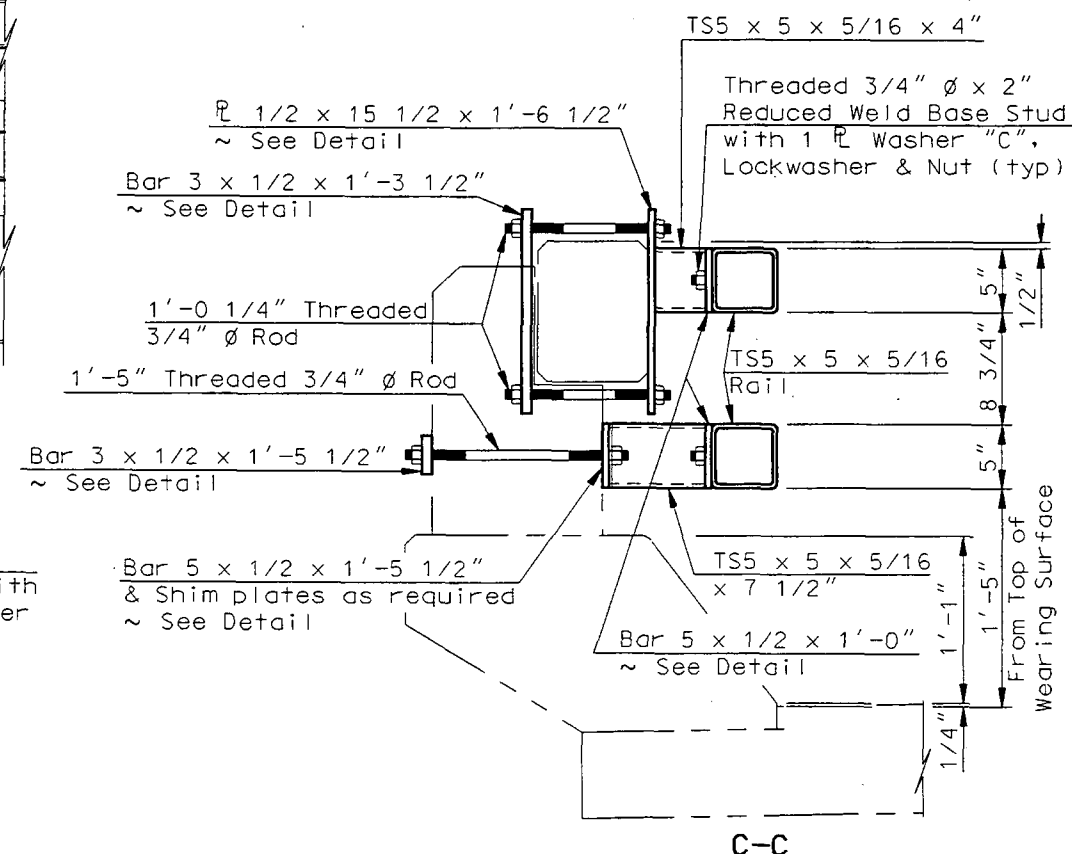
RAIL RETROFIT PART PLAN



CONCRETE REMOVAL DETAIL



RAIL RETROFIT PART ELEVATION

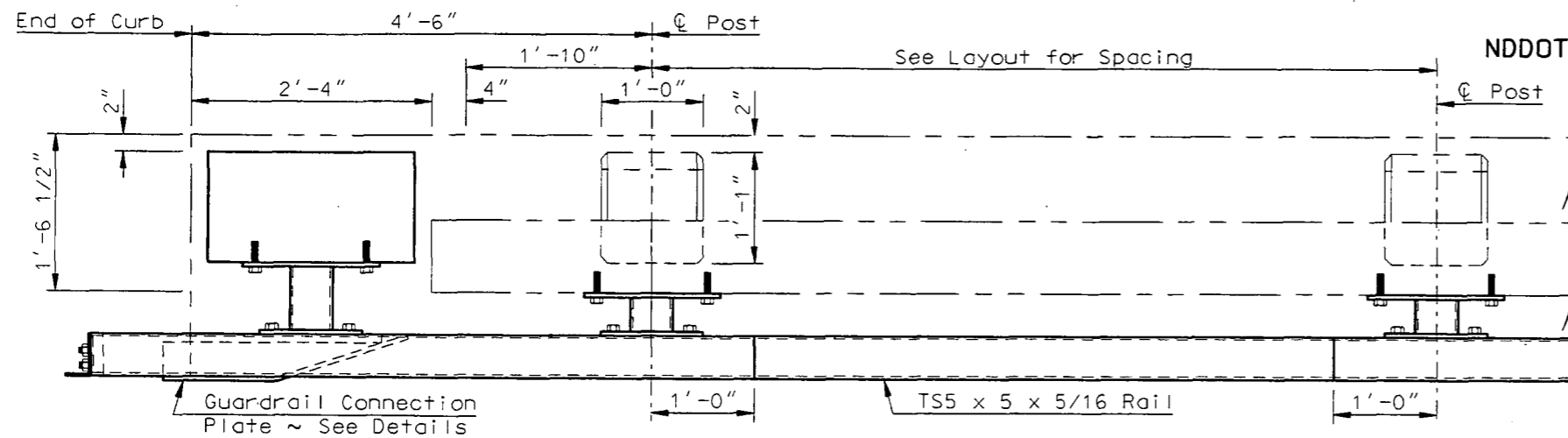


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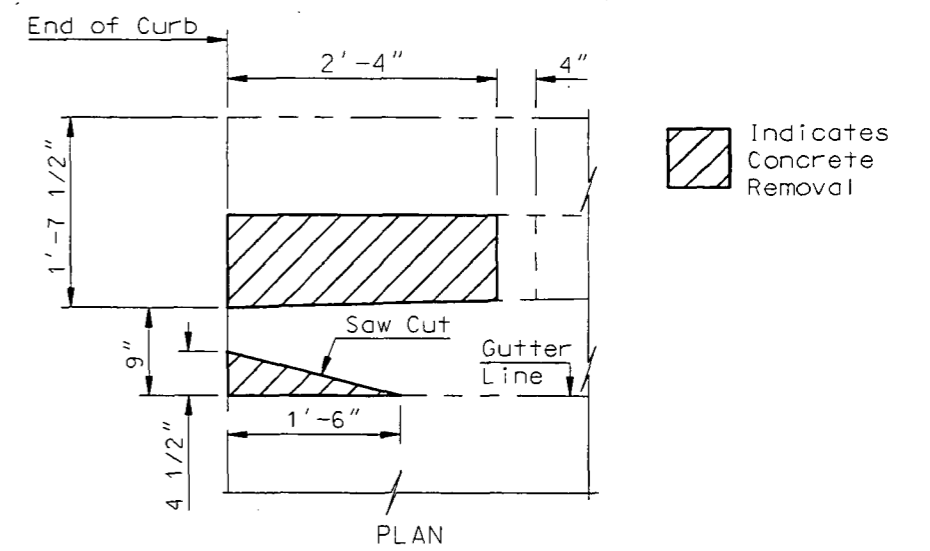
See Dwg 29-104.576L&R-4 & 5 for notes and details not shown on this drawing.

QUANTITIES
SEE DWG 29-104.576L&R-5
GOOSE RIVER (ENTRANCE END)
DOUBLE BOX BEAM E-RAIL RETROFIT DETAILS

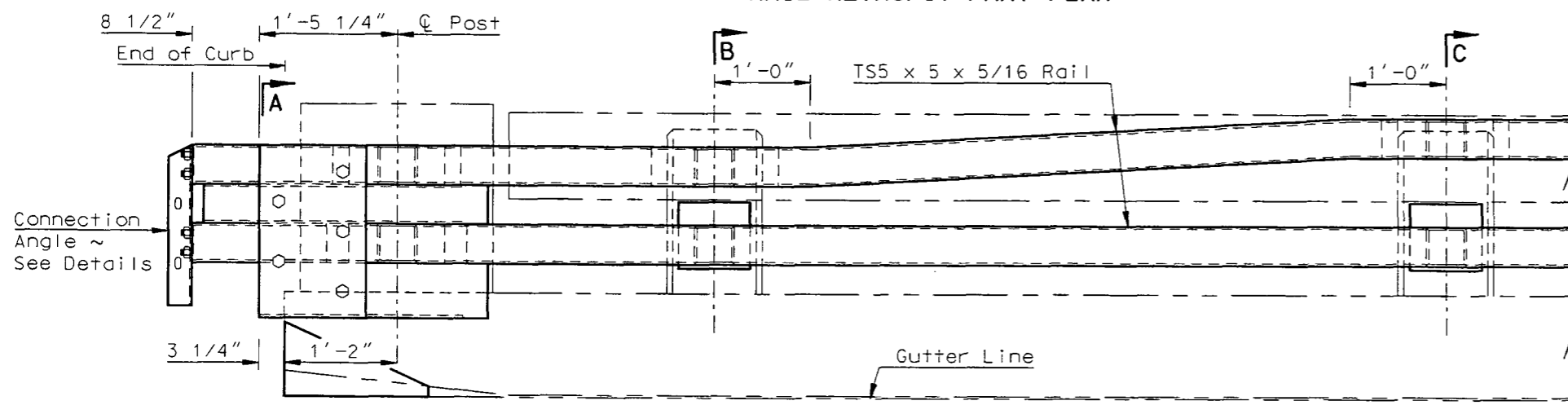
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ND	HSP-8-999(012)	14



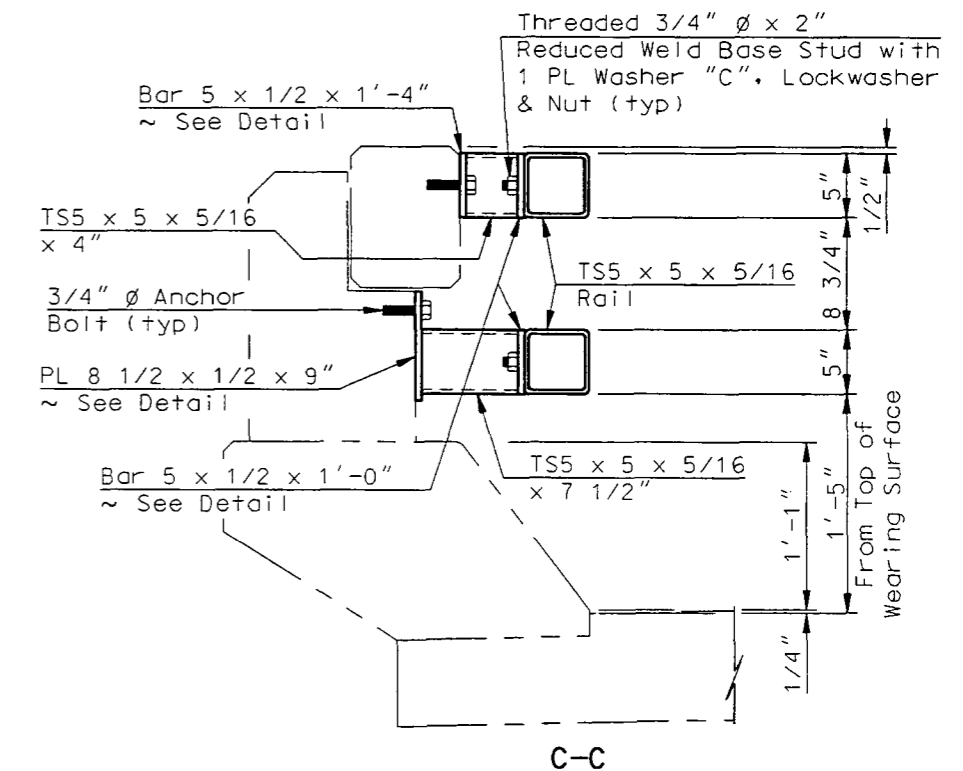
RAIL RETROFIT PART PLAN



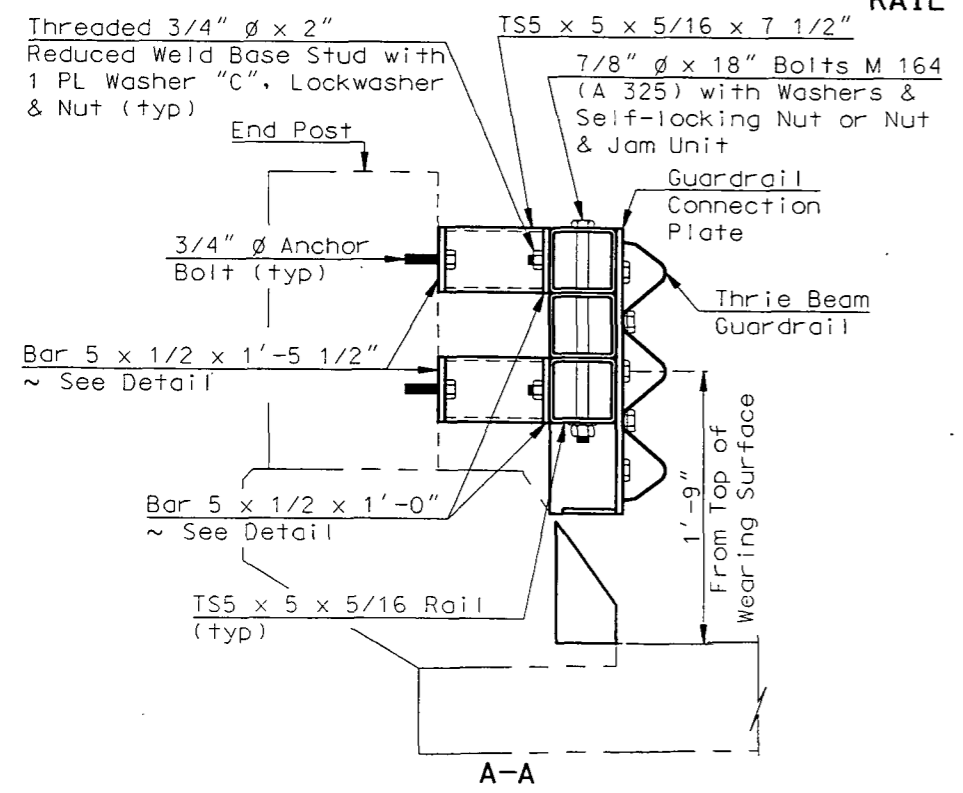
CONCRETE REMOVAL DETAIL



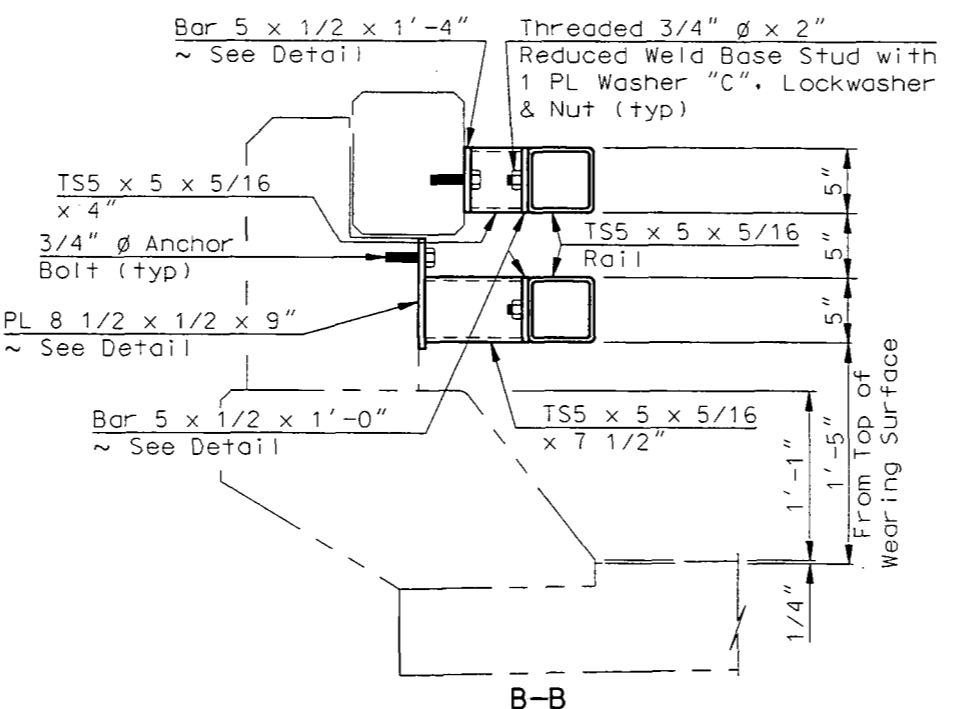
RAIL RETROFIT PART ELEVATION



C-C



A-A

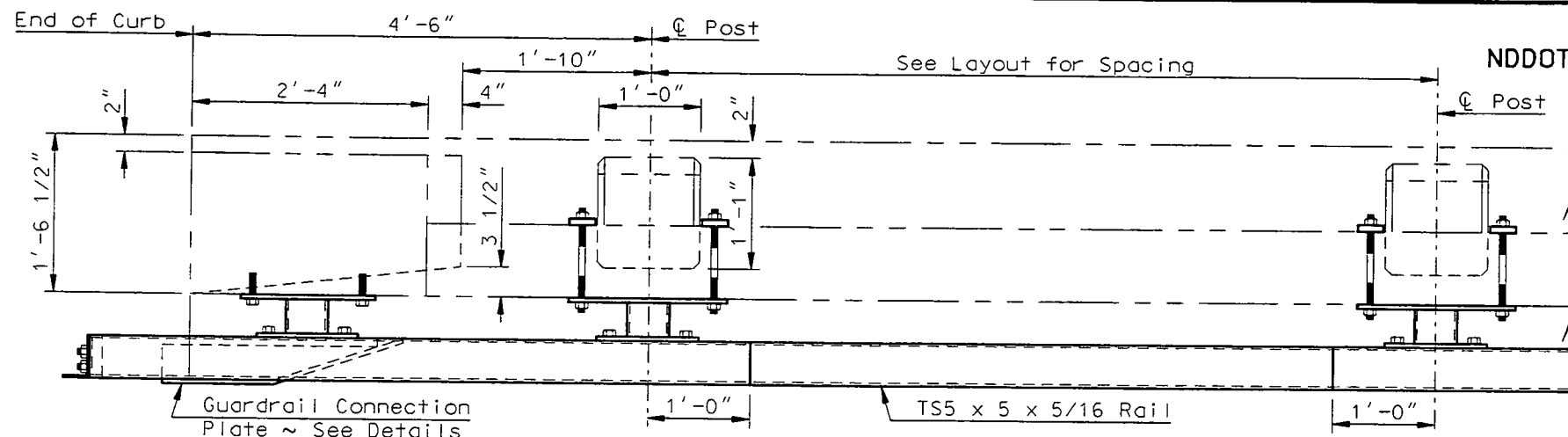


B-B

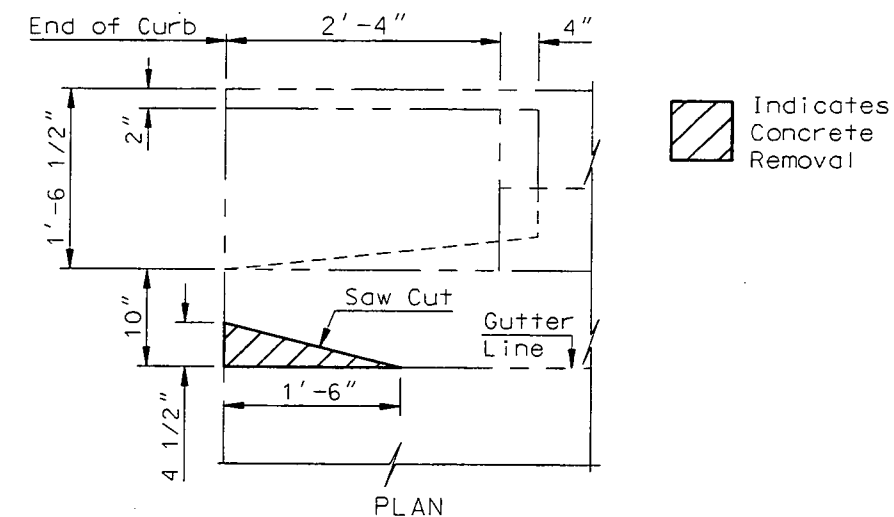
NOTE:
 See Dwg 29-104.576L&R-4 & 5 for notes and details not shown on this drawing.

QUANTITIES
SEE DWG 29-104.576L&R-5
GOOSE RIVER (ENTRANCE END) DOUBLE BOX BEAM E-RAIL RETROFIT DETAILS

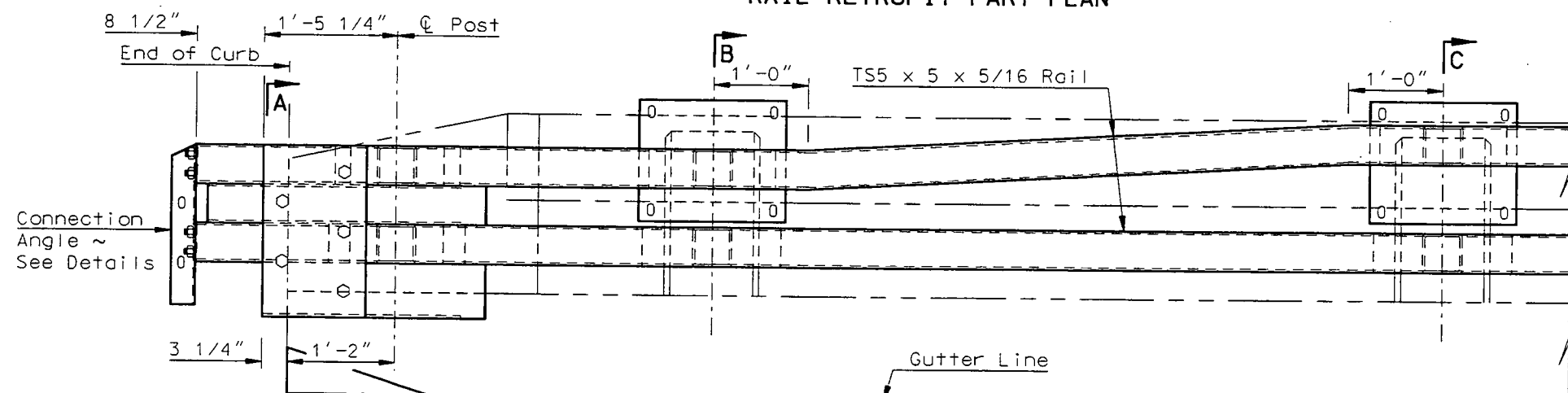
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ND	HSP-8-999(012)	15



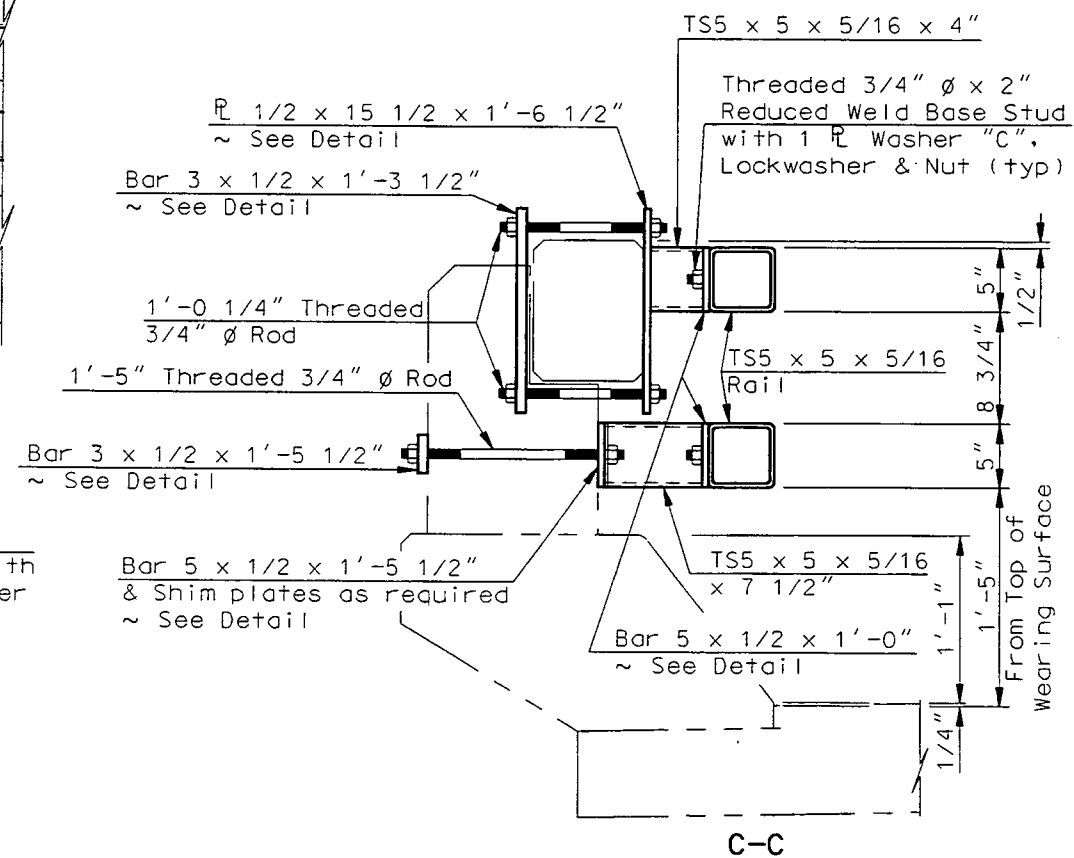
RAIL RETROFIT PART PLAN



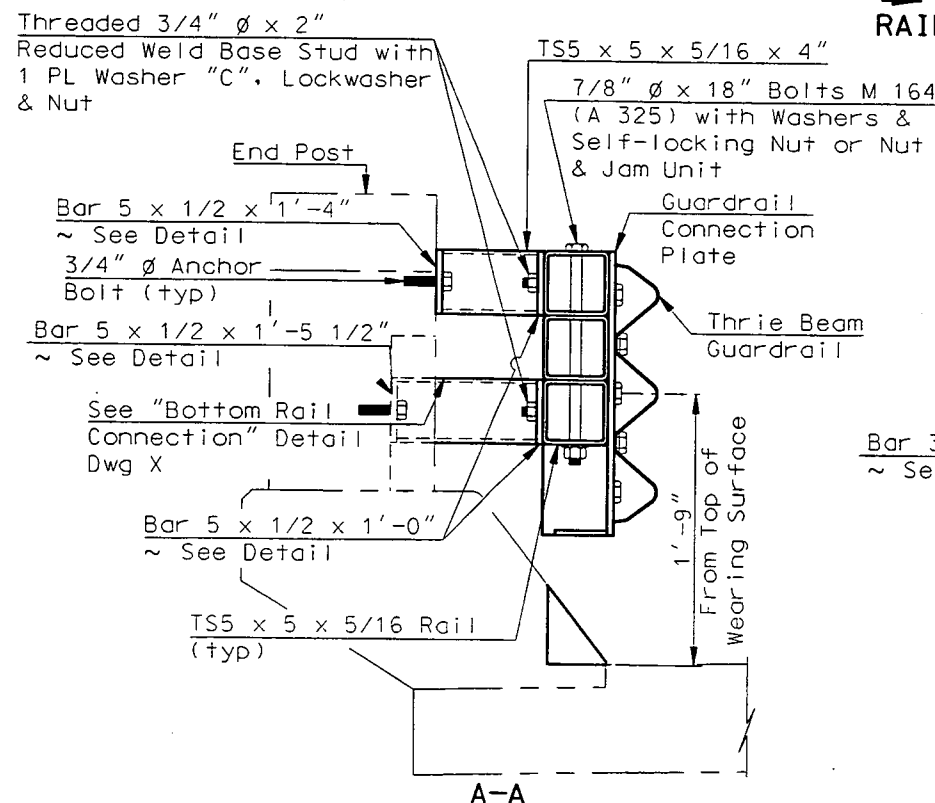
CONCRETE REMOVAL DETAIL



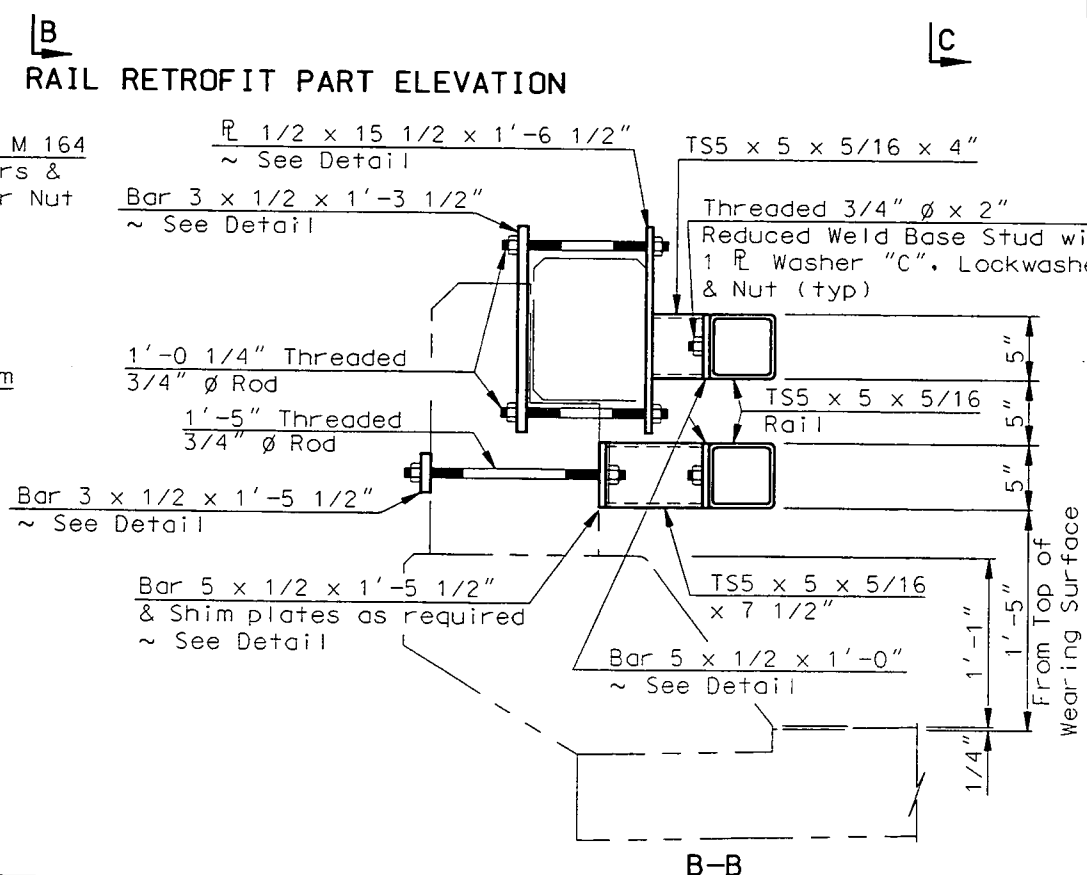
RAIL RETROFIT PART ELEVATION



C-C



A-A



B-B

NOTE:

See Dwg 29-104.576L&R-4 & 5 for notes and details not shown on this drawing.

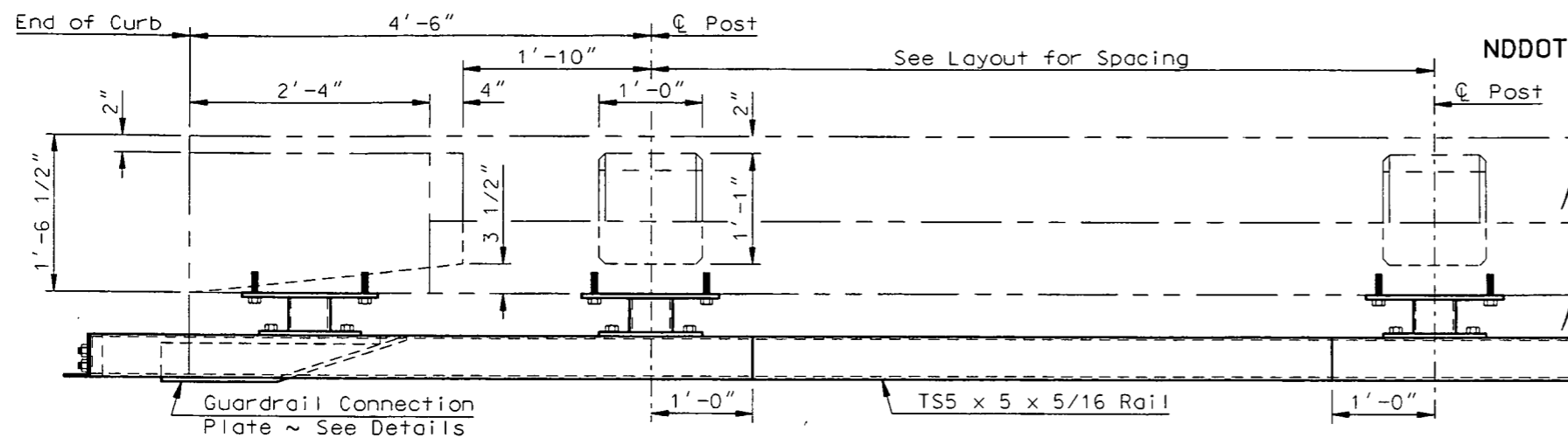
QUANTITIES

SEE DWG 29-104.576L&R-5

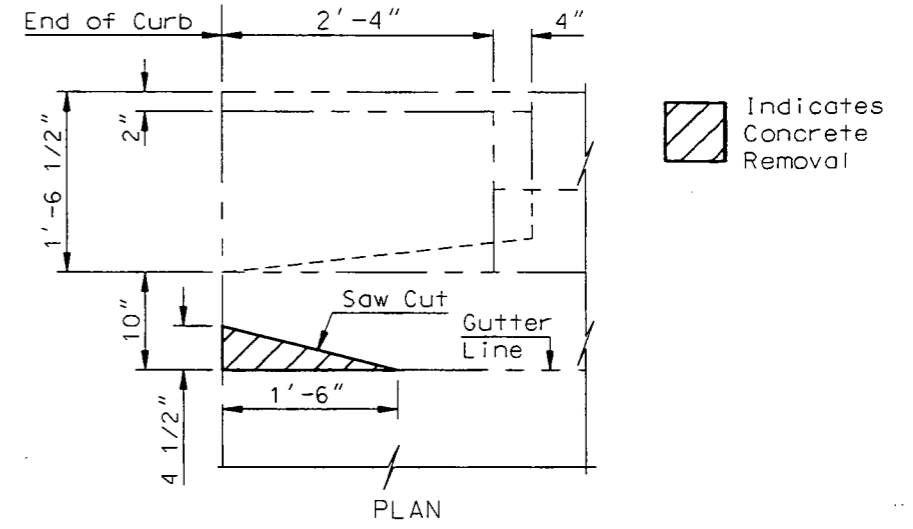
GOOSE RIVER
(ENTRANCE END)

DOUBLE BOX BEAM
E-RAIL RETROFIT DETAILS

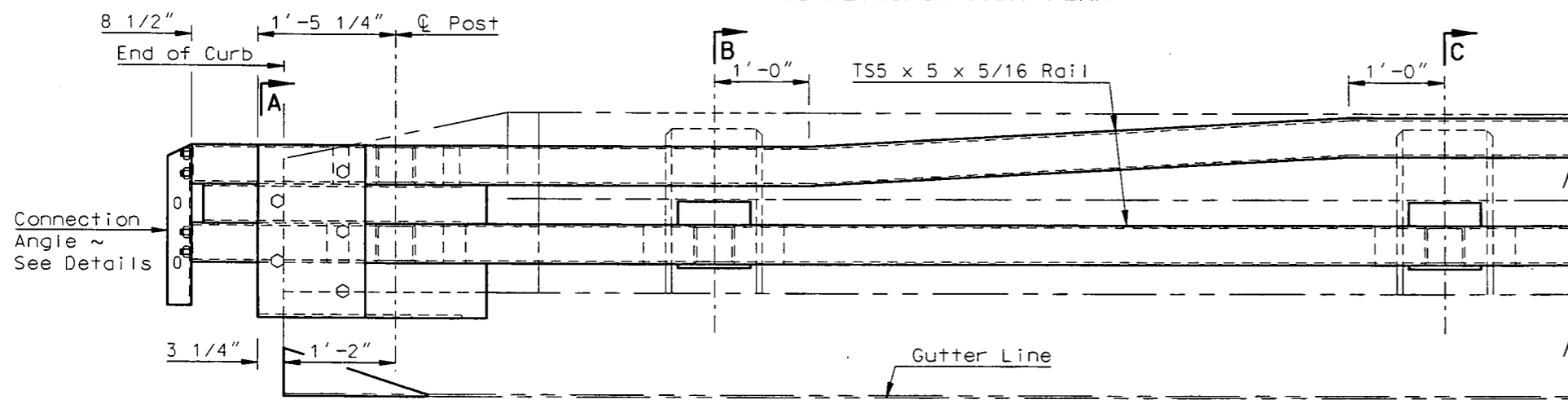
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ND	HSP-8-999(012)	16



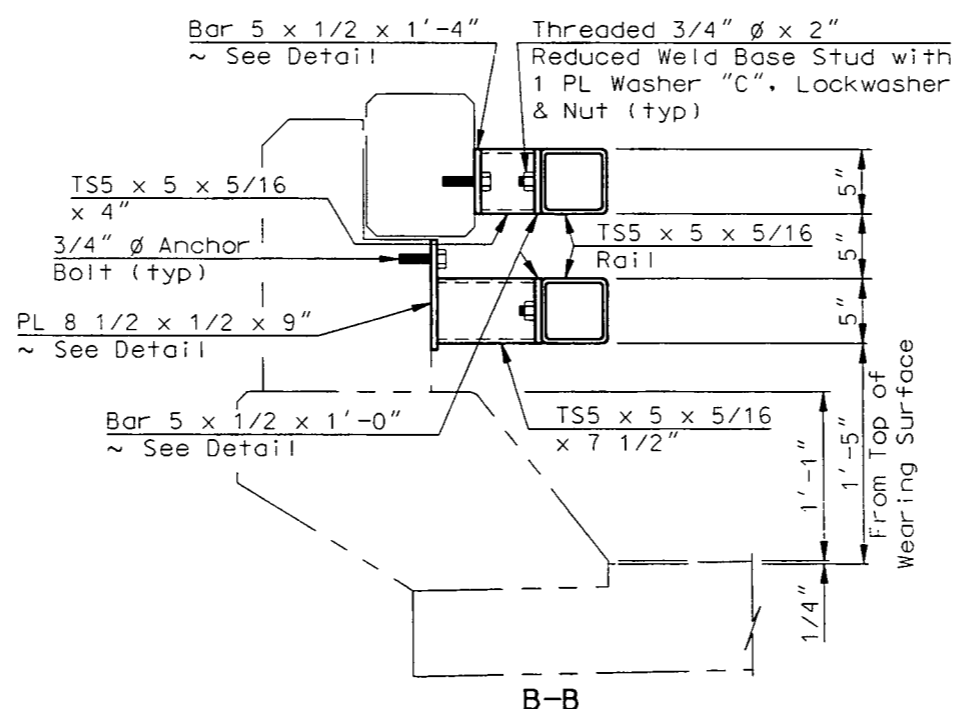
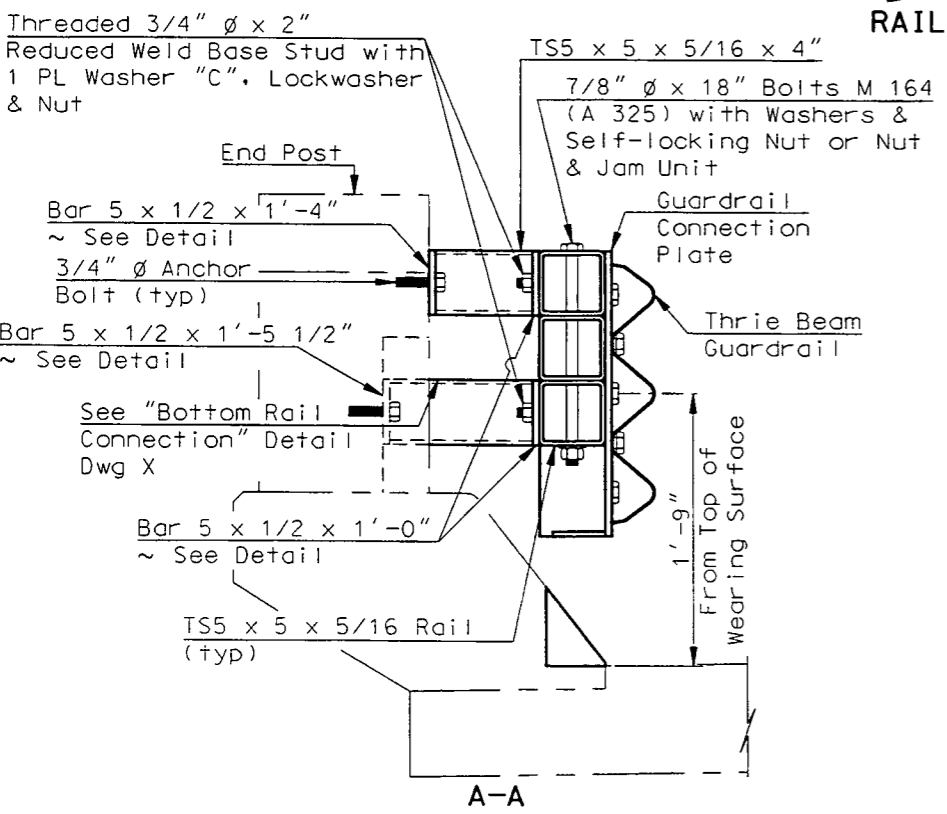
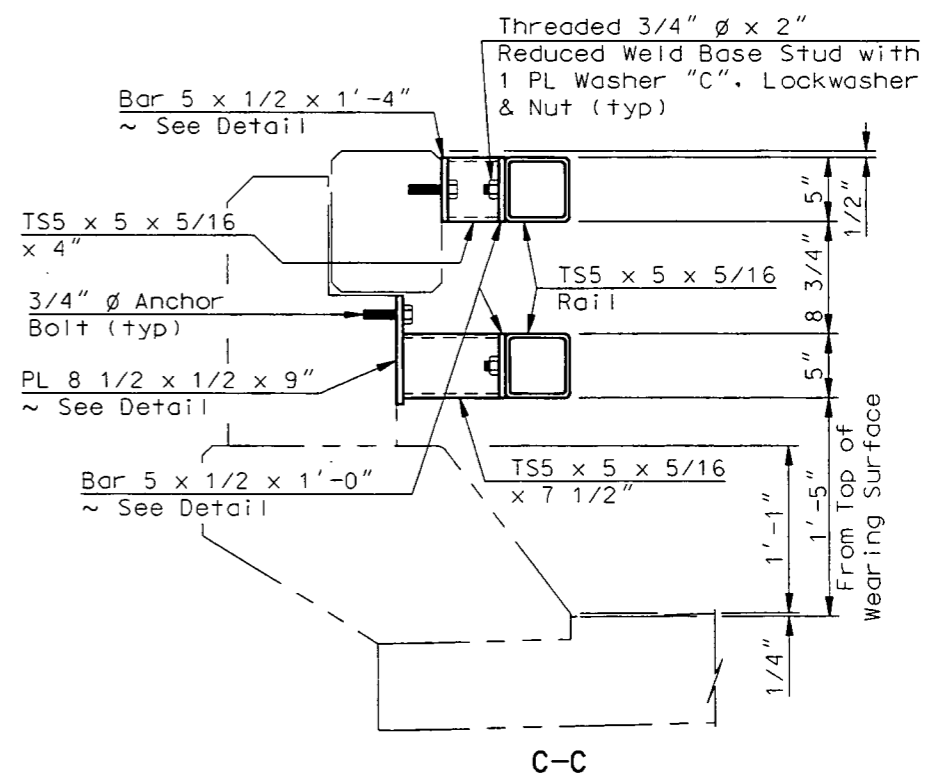
RAIL RETROFIT PART PLAN



CONCRETE REMOVAL DETAIL



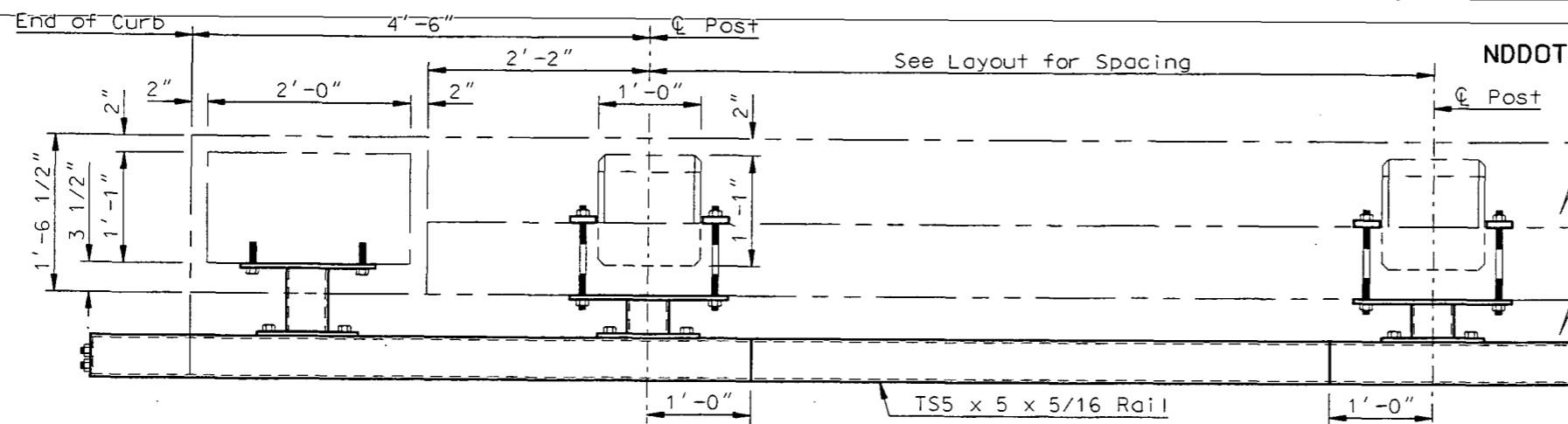
RAIL RETROFIT PART ELEVATION



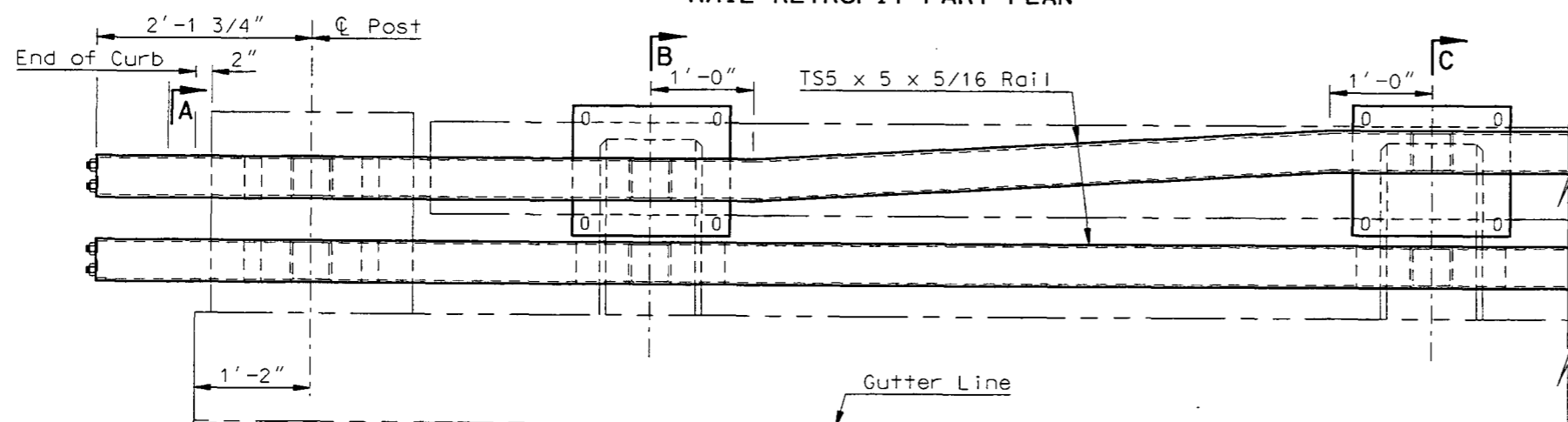
NOTE:
 See Dwg 29-104.576L&R-4 & 5 for notes and details not shown on this drawing.

QUANTITIES
SEE DWG 29-104.576L&R-5
GOOSE RIVER (ENTRANCE END)
DOUBLE BOX BEAM E-RAIL RETROFIT DETAILS

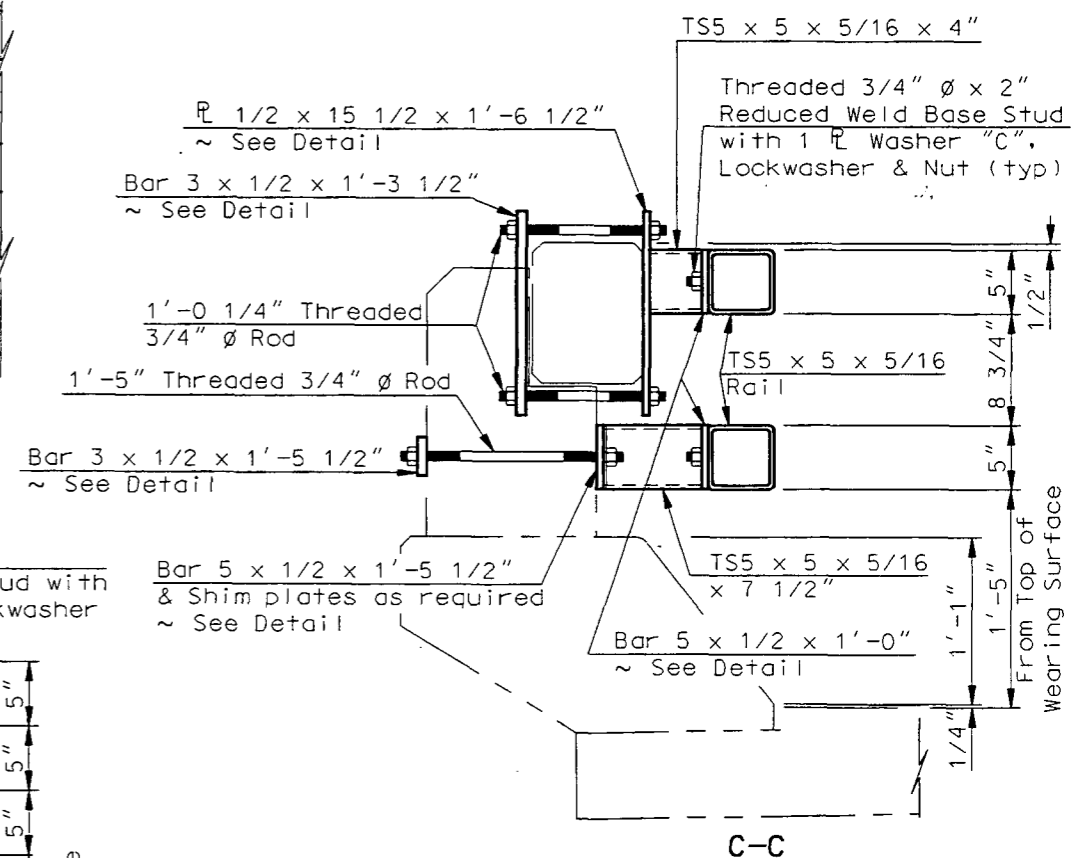
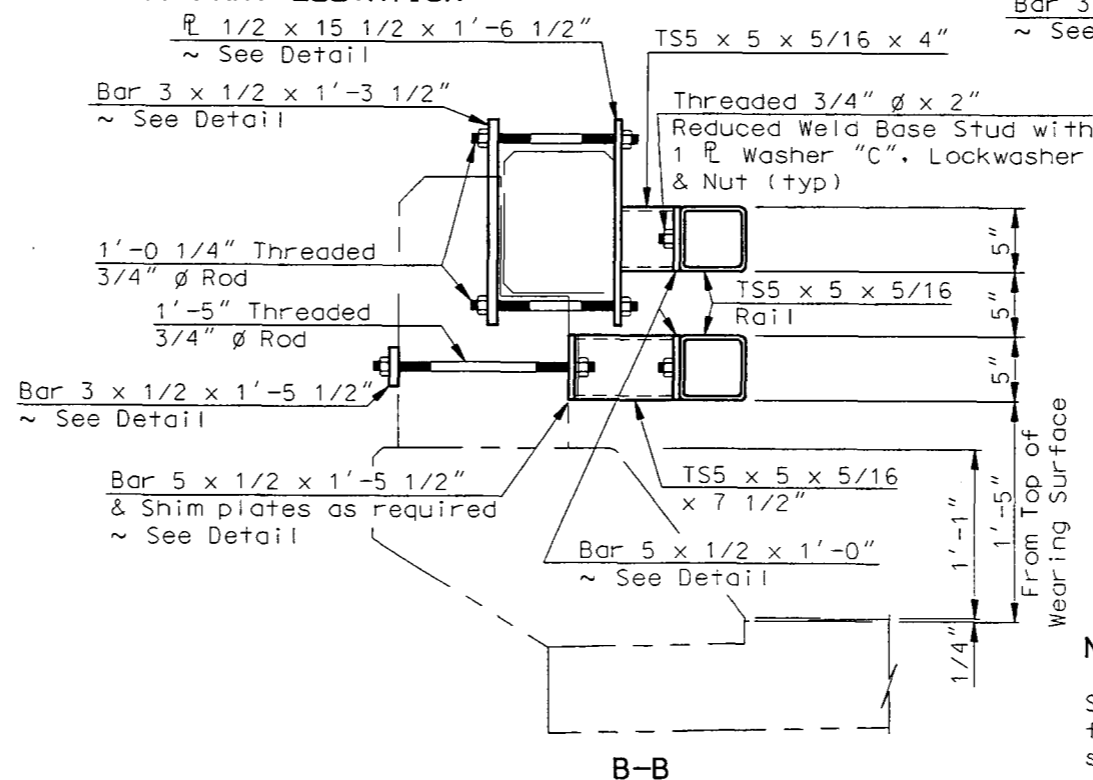
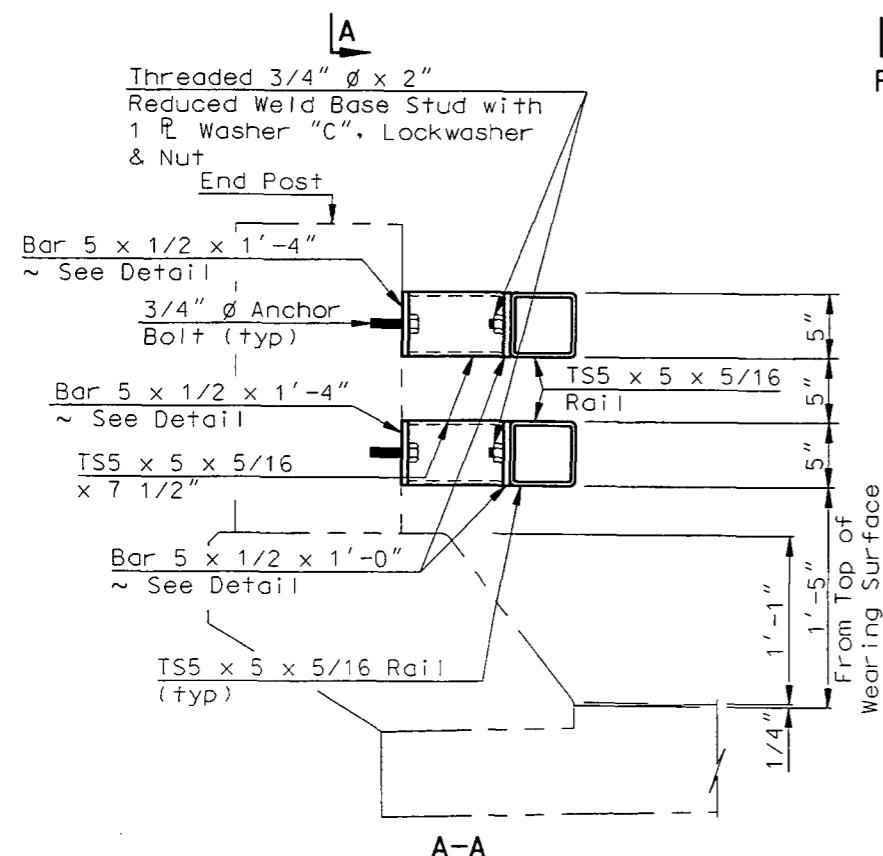
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ND	HSP-8-999(012)	17



RAIL RETROFIT PART PLAN



RAIL RETROFIT PART ELEVATION

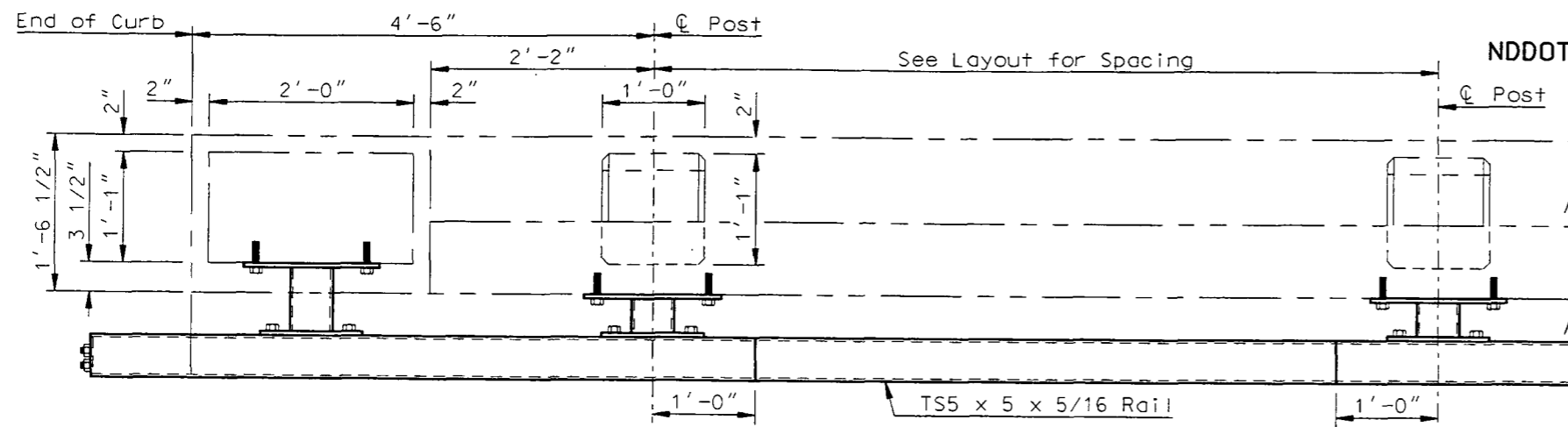


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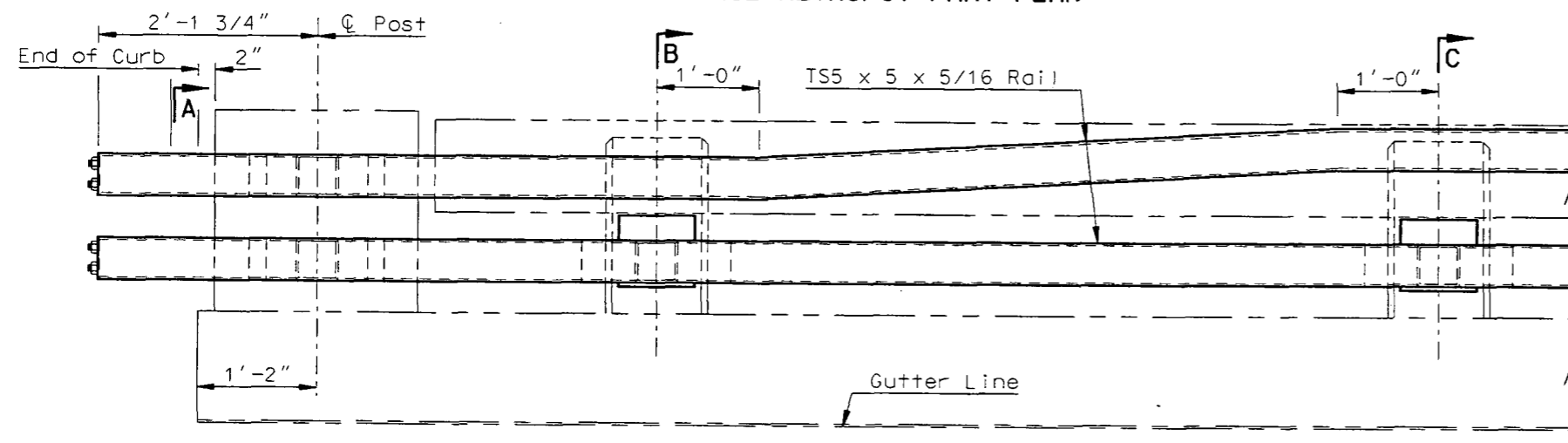
See Dwg 29-104.576L&R-4 & 5 for notes and details not shown on this drawing.

QUANTITIES
SEE DWG 29-104.576L&R-5
GOOSE RIVER (EXIT END) DOUBLE BOX BEAM E-RAIL RETROFIT DETAILS

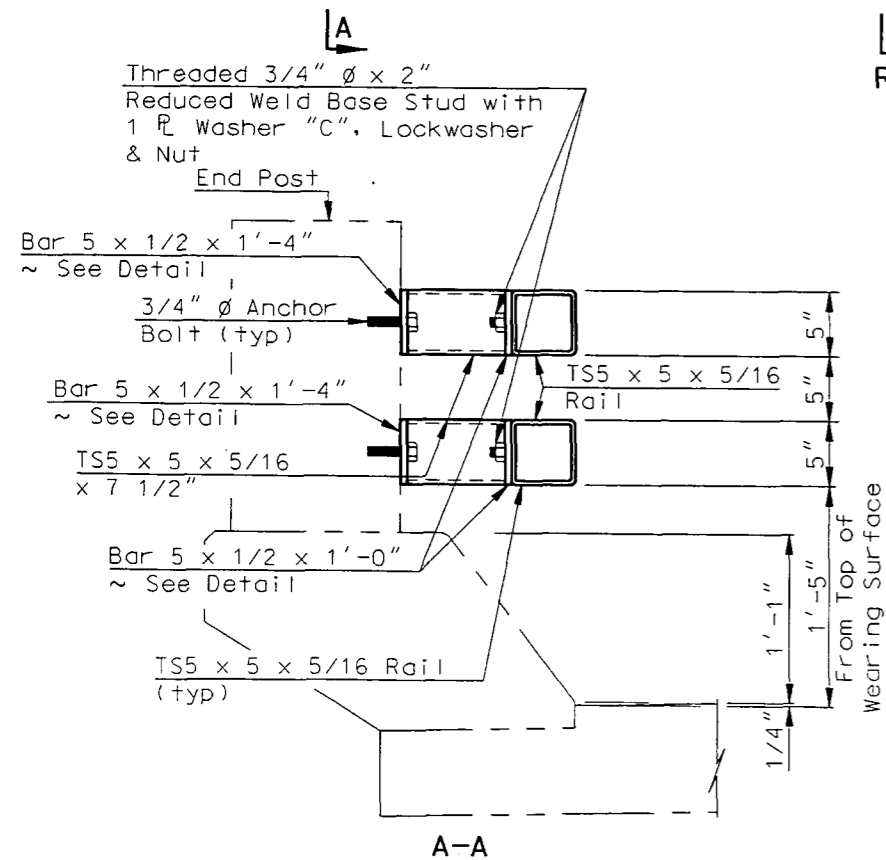
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ND	HSP-8-999(012)	18



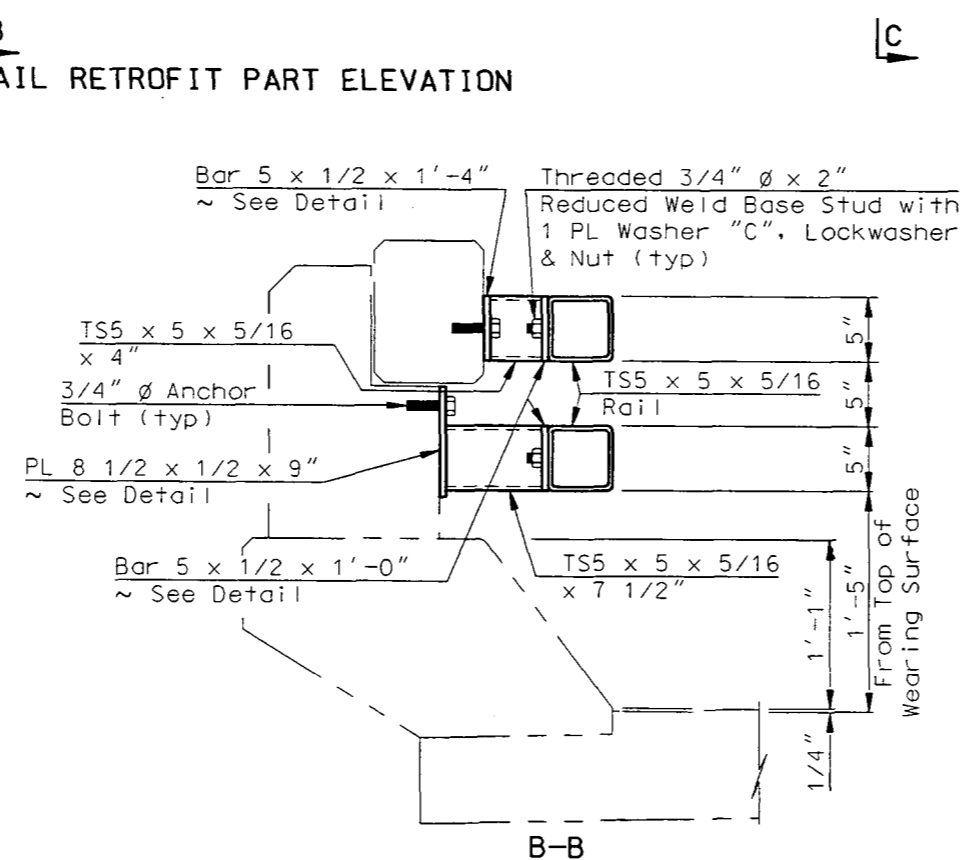
RAIL RETROFIT PART PLAN



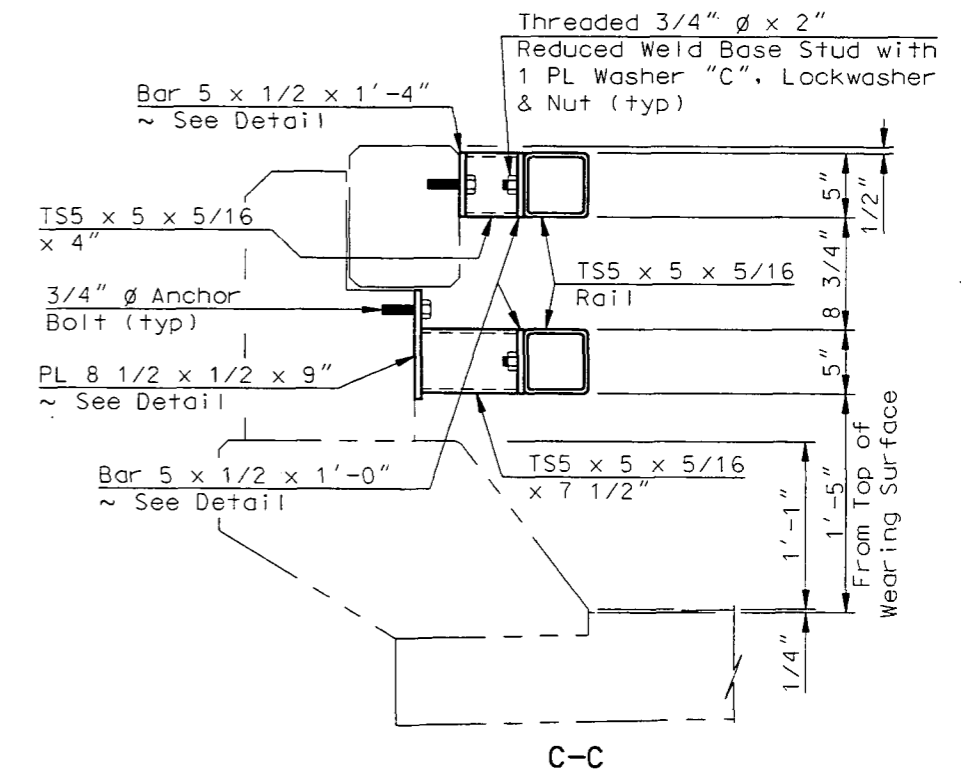
RAIL RETROFIT PART ELEVATION



A-A



B-B



C-C

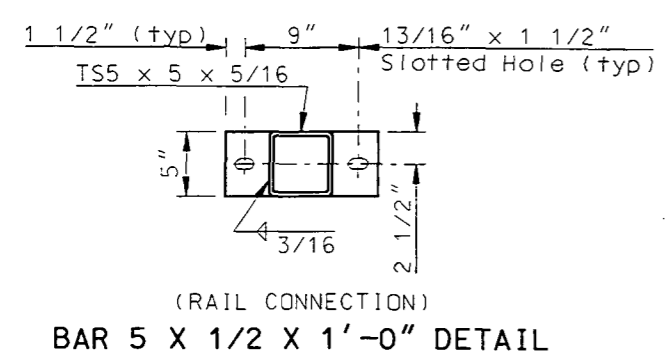
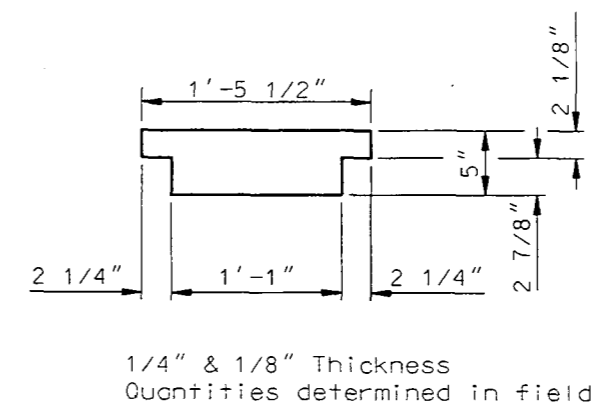
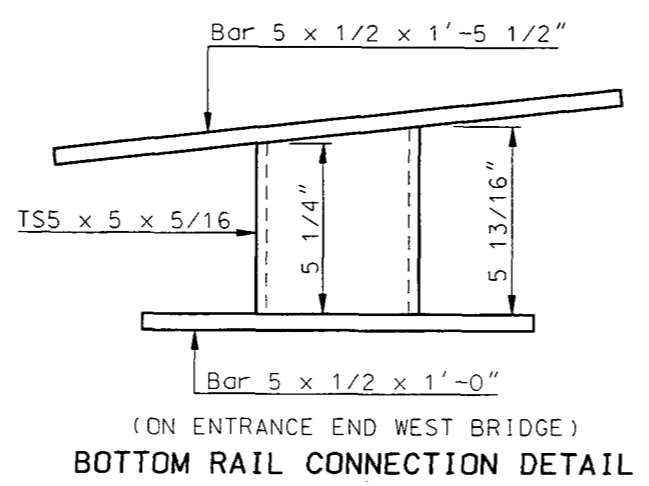
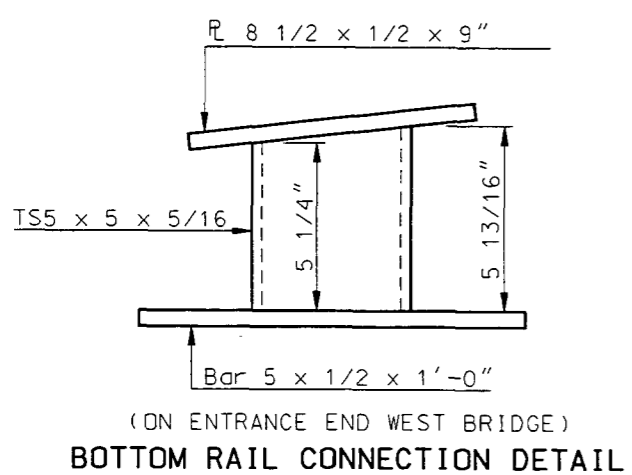
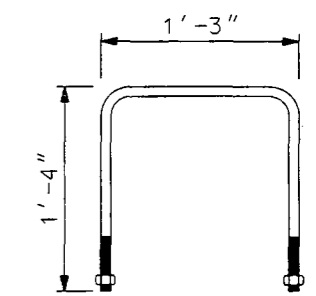
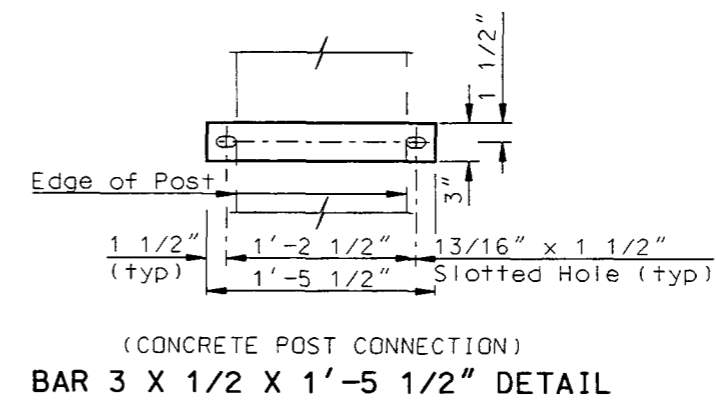
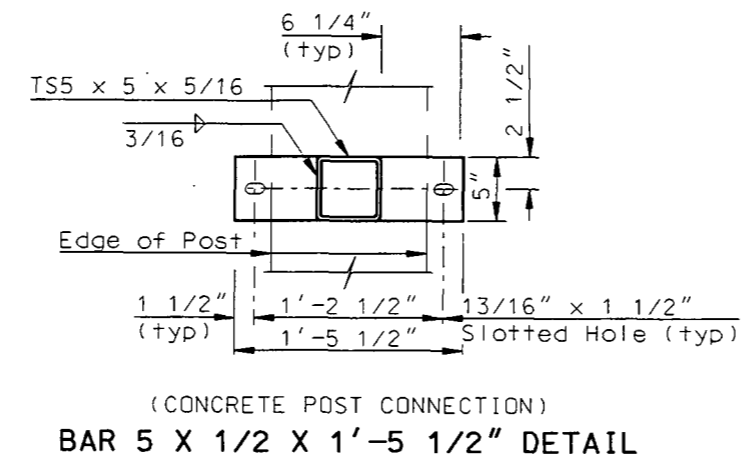
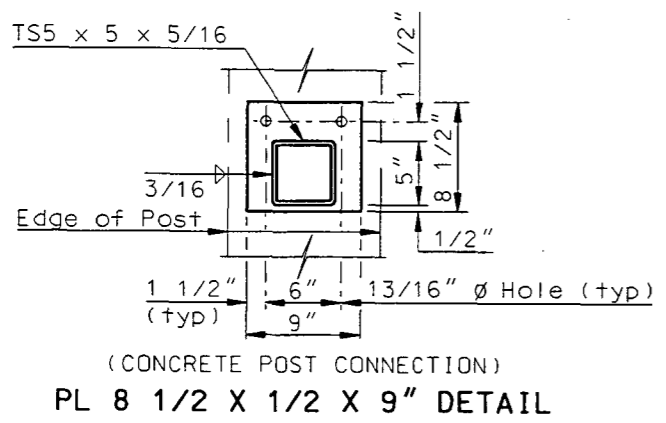
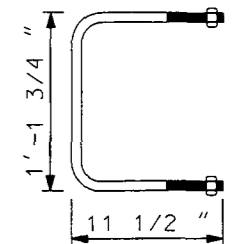
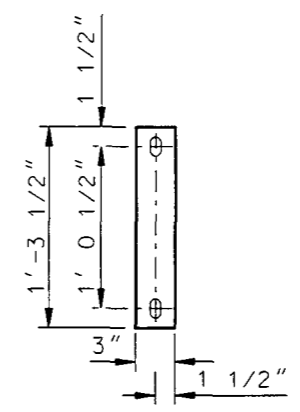
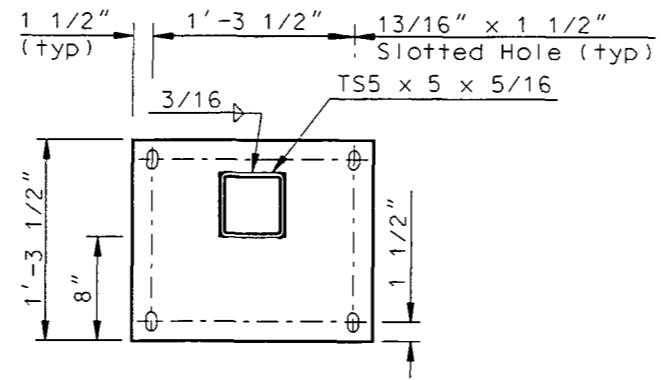
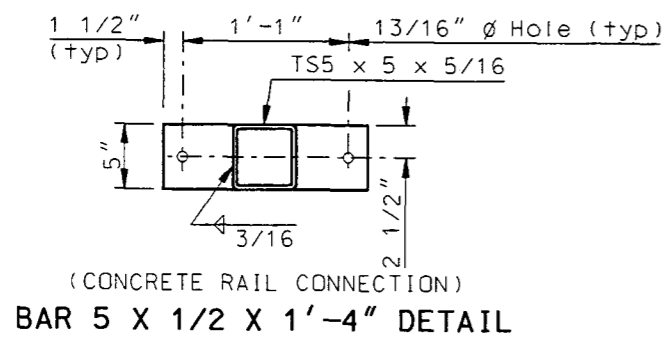
NOTE:

See Dwg 29-104.576L&R-4 & 5 for notes and details not shown on this drawing.

QUANTITIES
SEE DWG 29-104.576L&R-5
GOOSE RIVER (EXIT END) DOUBLE BOX BEAM E-RAIL RETROFIT DETAILS

23 U.S.C. 409
 NDDOT Reserves All Objections

STATE	PROJECT NUMBER	SHEET NO.
ND	HSP-8-999(012)	19

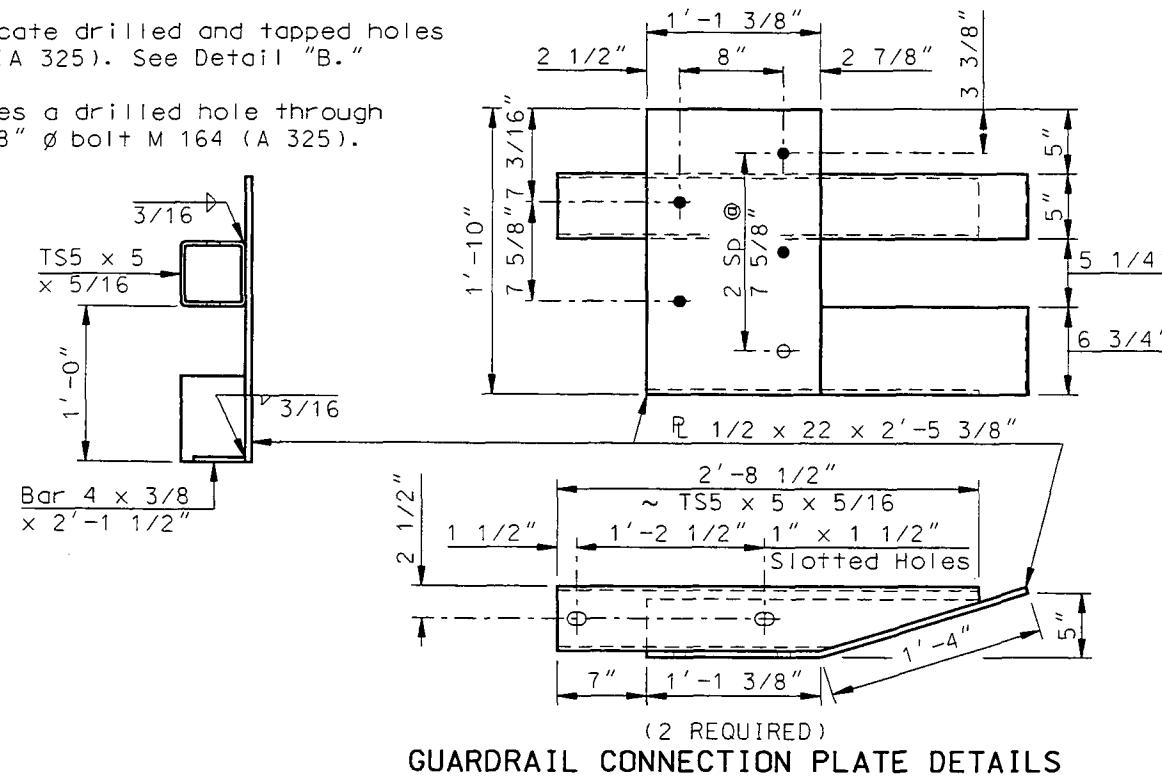


QUANTITIES
SEE DWG 29-104.576L&R-5
GOOSE RIVER
DOUBLE BOX BEAM E-RAIL RETROFIT DETAILS

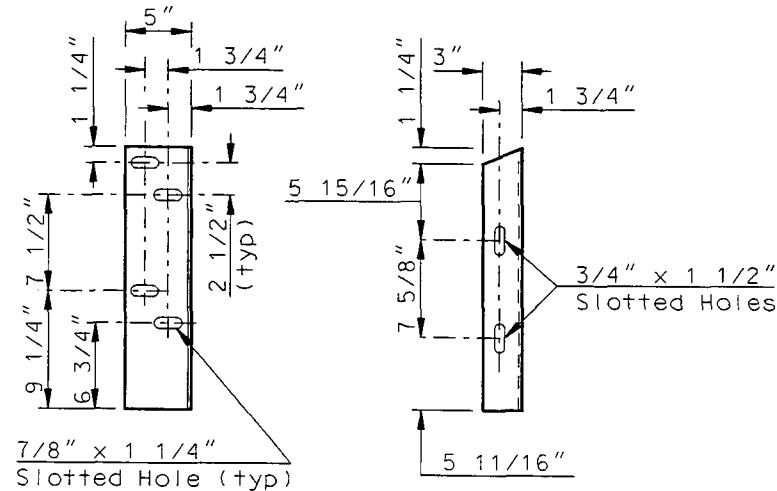
STATE	PROJECT NUMBER	SHEET NO.
ND	HSP-8-999(012)	20

The filled circles indicate drilled and tapped holes for 7/8" ø bolts M 164 (A 325). See Detail "B."

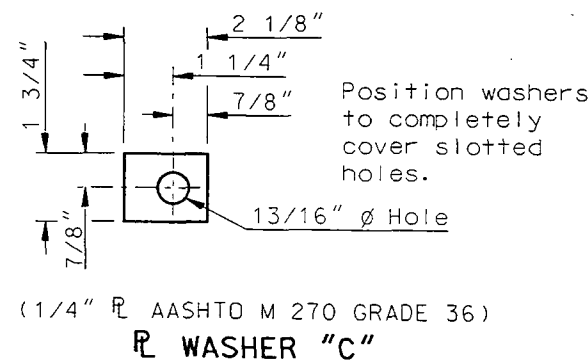
The open circle indicates a drilled hole through the 1/2" plate for a 7/8" ø bolt M 164 (A 325).



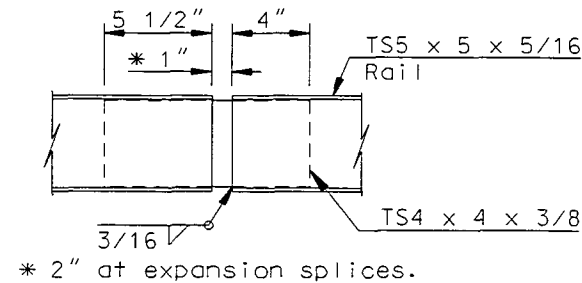
(2 REQUIRED)
 GUARDRAIL CONNECTION PLATE DETAILS



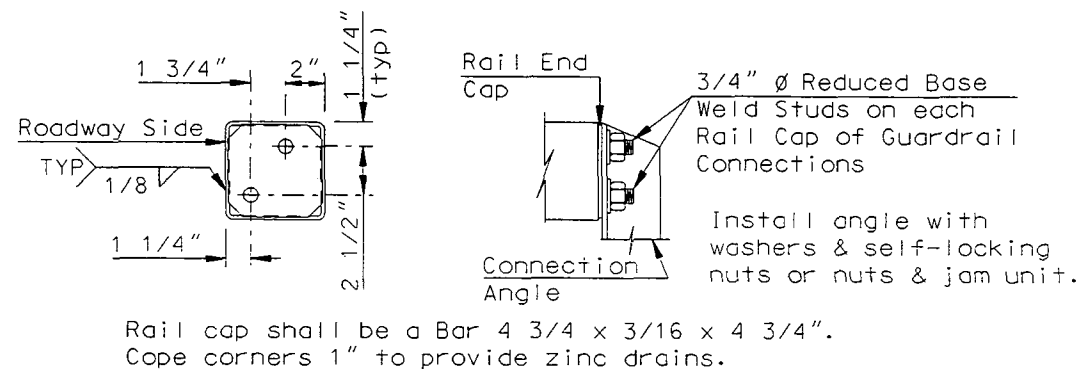
(L5 X 3 X 1/4 X 1'-8 1/2")
 (2 REQUIRED)
 CONNECTION ANGLE DETAILS



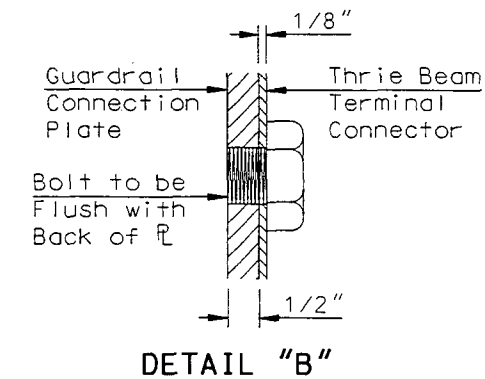
(1/4" PL AASHTO M 270 GRADE 36)
 R WASHER "C"



* 2" at expansion splices.
 RAIL SPLICE DETAIL



RAIL CAP DETAILS



DETAIL "B"

NOTES:

The bid item shall be "Double Box Beam Rail Retrofit - E-Rail." The pay length shall be end to end and in linear feet.

Rail elements shall be square structural tubing in accordance with ASTM Specification A 500 Grade B.

Steel plates and angles shall conform to AASHTO Specification M 270 Grade 36, unless otherwise noted.

Railing shall be fabricated to the horizontal and vertical alignment of the structure.

Payment for the railing shall include compensation for furnishing and installing the guardrail connection plates.

All structural steel shall be hot-dip galvanized after fabrication according to AASHTO M 111.

Rails shall be fabricated so that each rail segment between splices is attached to a minimum of two posts.

The threaded rods & u-bolts shall be M 270 Grade 36 Steel and galvanized according to M 232. The u-bolts shall be tightened to provide a minimum tensile force of 2,500 lbs. and a maximum tensile force of 2,700 lbs.

The anchor bolts shall be embedded into the concrete with a chemical adhesive system that can develop a tensile strength of at least 17,500 lbs.

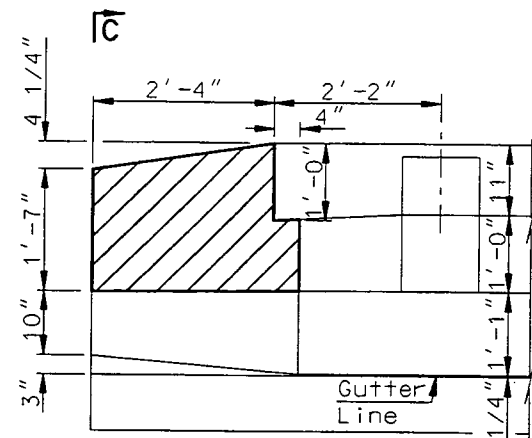
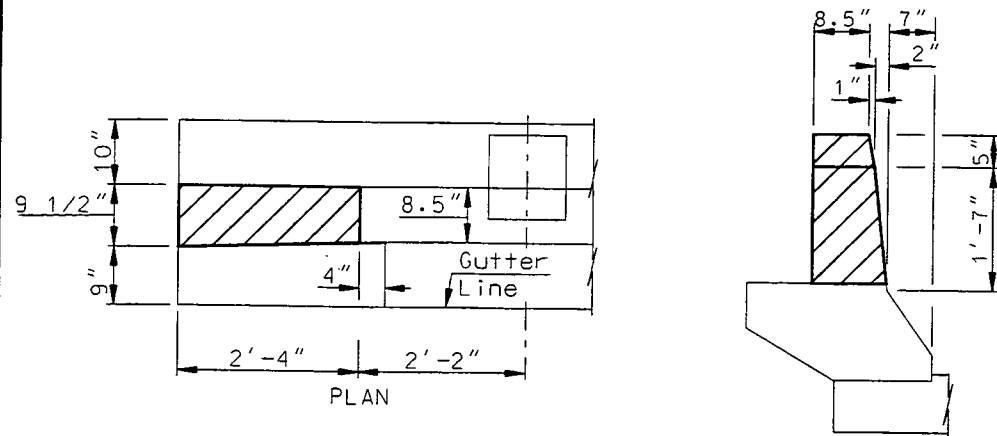
All anchor and splice bolts shall be AASHTO M 164 (A 325) and shall be galvanized according to M 232.

The Contractor shall field verify all dimensions and incorporate them into the shop drawings. The double box beam rail retrofit shop drawings shall be submitted for approval to the Construction Office before fabrication.

QUANTITIES	
E-RAIL RETROFIT	1027.8 LF

GOOSE RIVER
 DOUBLE BOX BEAM
 E-RAIL RETROFIT DETAILS

STATE	PROJECT NUMBER	SHEET NO.
ND	HSP-8-999(012)	21

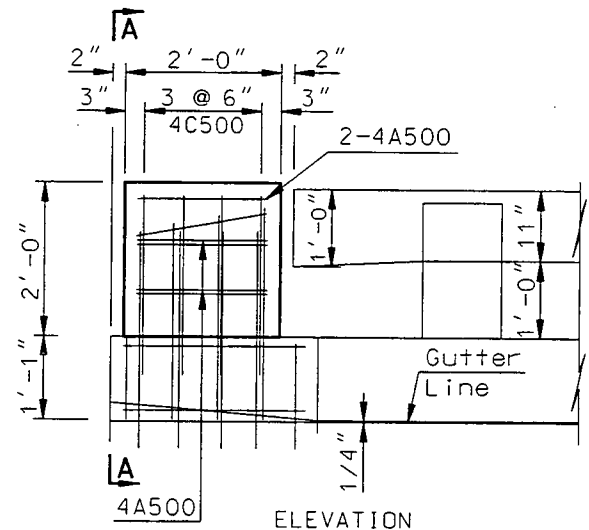
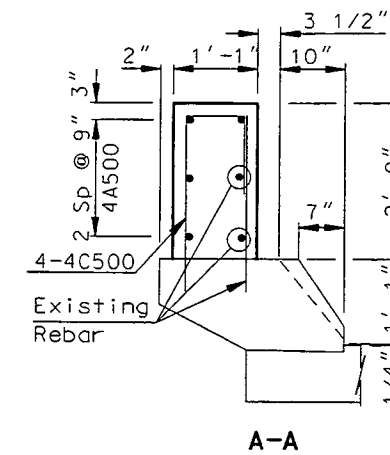
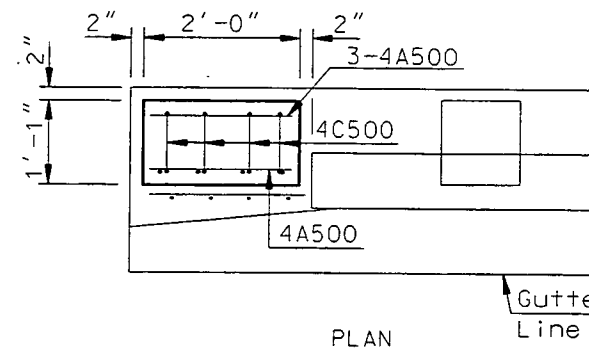


C ELEVATION
 END POST REMOVAL DETAILS
 END POST REMOVAL DETAILS



Hatched areas indicate concrete to be removed.

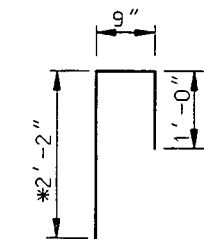
Care shall be taken to ensure no damage is done to the existing reinforcing steel that is to remain in place.



A ELEVATION
 END POST MODIFICATION DETAILS

BAR LIST (ONE POST)				
SIZE	MARK	NO.	LENGTH	SHAPE
4	A500	4	1'-8"	STR.
4	C500	4	*3'-11"	BENT

ESTIMATE OF QUANTITIES	
REMOVAL OF CONCRETE	0.19 CY
CLASS AAE-3 CONCRETE	0.16 CY
REINFORCING STEEL	15 LBS



C500
 Dimensions shown are out to out
BENT BAR DETAILS
 * Length may vary depending on manufacturer's recommendations for anchorage.

NOTE:

4C500 bars shall be installed according to the manufacturer's recommendations, with a high strength adhesive specifically intended for concrete anchorage, in accordance with Sec. 806.02 of the NDDOT Standard Specifications.

The concrete shall be class AAE-3 and the reinforcing steel shall be grade 60.

The existing end posts shall be removed and properly disposed of.

The quantities shown are for informational purposes only. All materials, labor and equipment including concrete and reinforcing bars required to remove and replace the end posts shall be included in the pay item "Bridge End Post Modification".

Surface Finish "D" shall be required for all surfaces of the end posts.

QUANTITIES	
BRIDGE END POST MODIFICATION	2 EA
GOOSE RIVER	
ENDPOST MODIFICATION DETAILS	

DESIGN DATA				
Traffic	Average Daily			Est. Max. Hr.
Current .1998	Pass: 3,235	Trucks: 825	Total: 4,060	.410
Forecast .2018	Pass: 5,180	Trucks: 1,405	Total: 6,585	.660
Minimum Sight Dist. for:		Design Speed .70		
Stopping 625'		Bridges.		
Limited Access Control				

JOB# 21

FHWA REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	IM-8-029(042)100	1

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION

Federal Aid Project IM-8-029(042)100
In Traill County
Concrete Pavement Repair, Dowel Bar Retrofit,
Grinding & Incidentals
(Southbound Roadway)

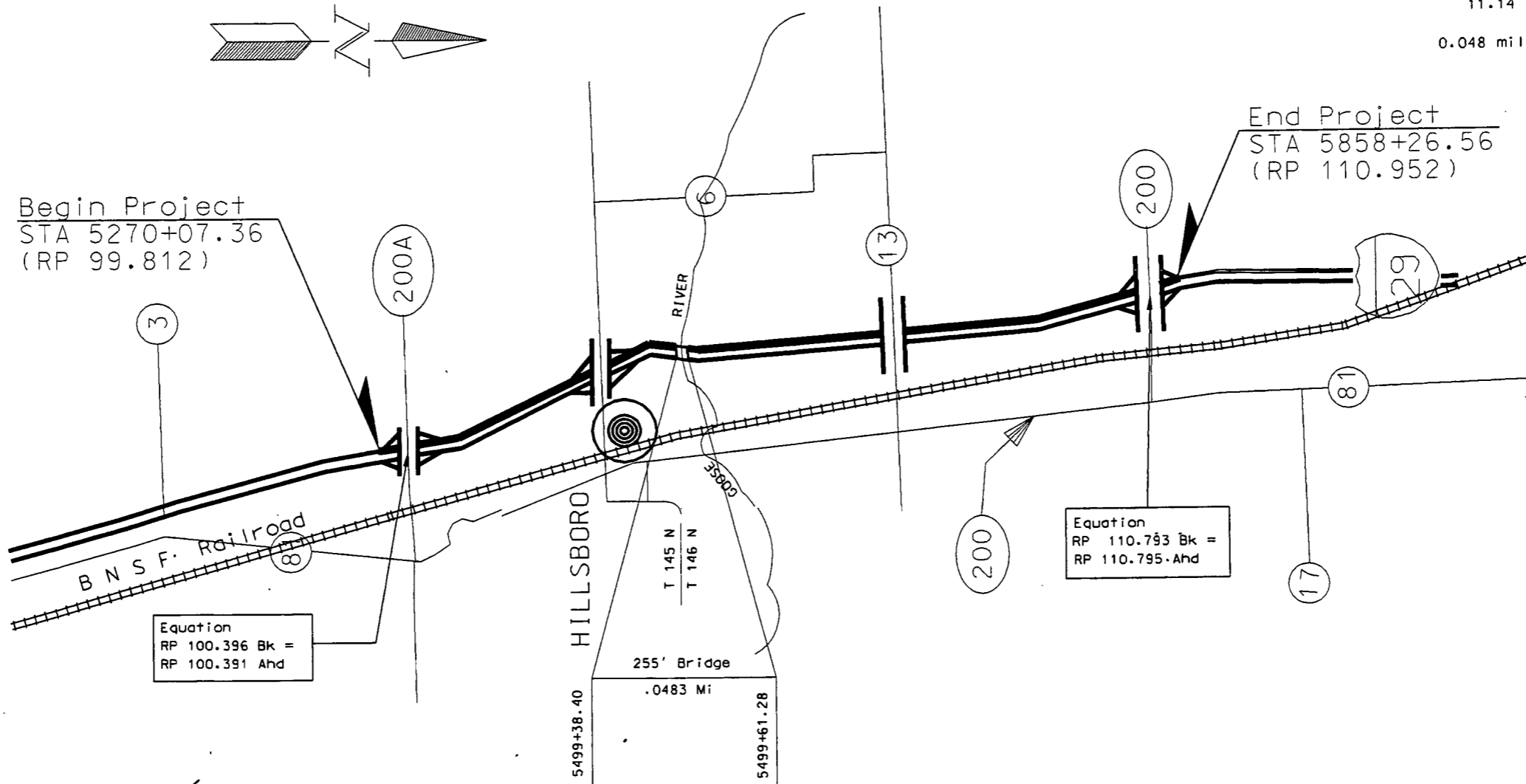
GOVERNING SPECIFICATIONS:

Standard Specifications adopted by the North Dakota Department of Transportation October 1997; Standard Drawings currently in effect; and other Contract Provisions submitted herein.

LENGTH OF PROJECT

Miles Gross	Miles Net
11.14	11.092

0.048 miles deducted for bridge



DESIGNER Donald F. Fike
 DESIGNER Harlan R. Heberich
 DESIGNER _____
 RECOMMEND APPROVAL _____ .19 _____
 DESIGN ENGINEER _____

U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

APPROVED _____

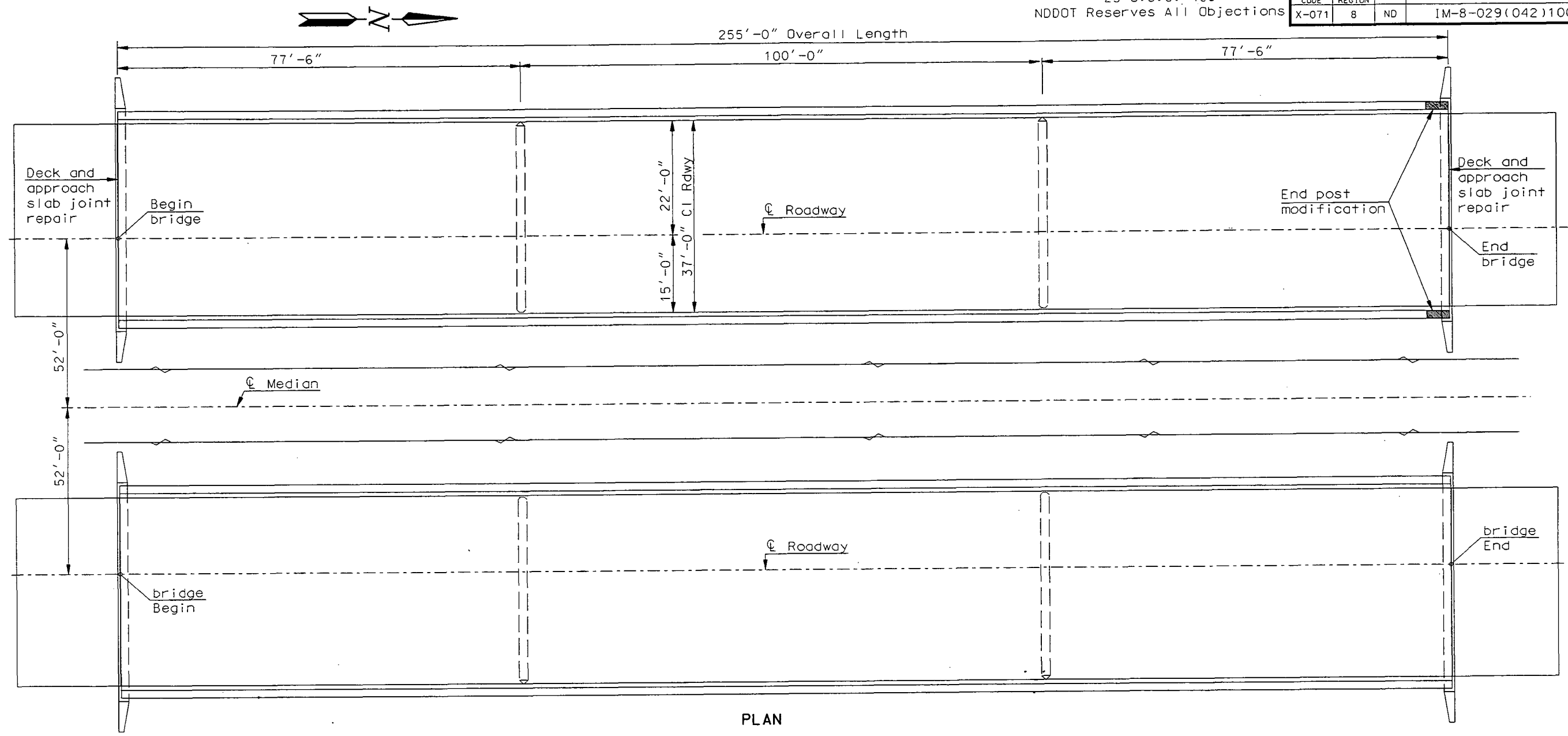
 DIVISION ADMINISTRATOR DATE

APPROVED DATE 3-26-99

Earl E. Nelson
 DIRECTOR OF HIGHWAYS
AND ENGINEERING

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION





PLAN

100 SCOPE OF WORK: The work at this site consists of modifying the end posts at the north end of the bridge and repairing the joints between the approach slab and the deck. All work is on the southbound roadway.

930 NOSING CONCRETE: The nosing concrete material shall be an elastomeric concrete or a polymeric concrete that will provide a durable edge that can withstand live-load traffic without chipping or spalling. The nosing concrete material shall be SILSPEC 900 PNS, manufactured by Silicone Specialties Inc.; WABOCRETE II, manufactured by Watson Bowman Acme; ELASTOMERIC CONCRETE, manufactured by D.S. Brown Company or an approved equal. The nosing concrete shall be mixed and installed according to the manufacturer's recommendation. All labor and materials required to install the nosing concrete shall be included in the bid item "Nosing Concrete".

930 SILICONE: The silicone sealant shall be a rapid cure, self leveling, cold applied, two component silicone sealant that will bond to and be compatible to the nosing concrete used. The sealant shall be installed according to the manufacturer's recommendations. The silicone sealant and the nosing concrete must be supplied by the same manufacturer as a complete system. The backer rod and any necessary bonding materials shall be included in the bid item "Silicone Sealant".

930 TECHNICAL ASSISTANCE: The Contractor shall acquire technical assistance from the manufacturer of the nosing concrete and the silicone sealant for the surface preparation and installation of the nosing concrete and silicone sealant. A technical representative must be present for the start of surface preparation and installation for at least one day. The Contractor shall contact the manufacturer at least two weeks prior to the installation. The technical assistance shall be provided at no additional cost to the Department.

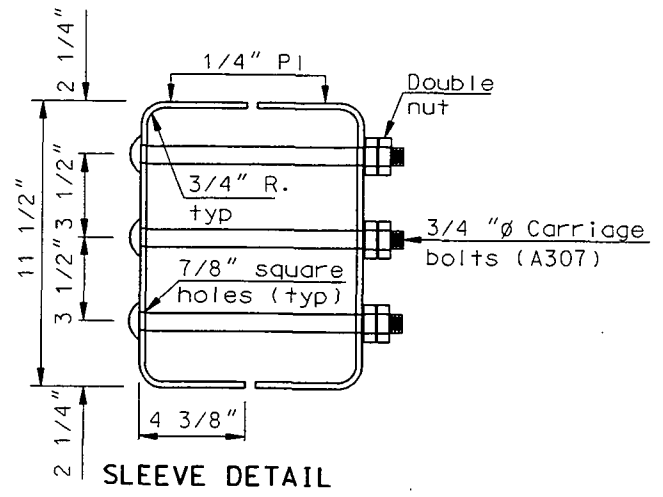
SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
602	1210	BRIDGE END POST MODIFICATION	EA	2.0
930	8642	NOSING CONCRETE	CF	34.7
930	8644	SILICONE SEALANT	LF	74.0

GOOSE RIVER
 BRIDGE LAYOUT

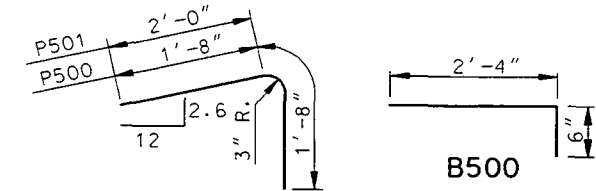
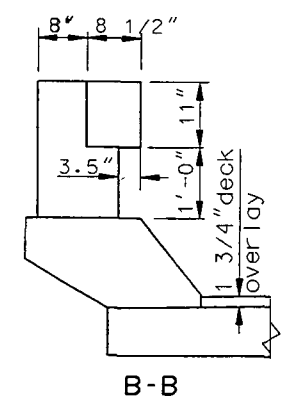
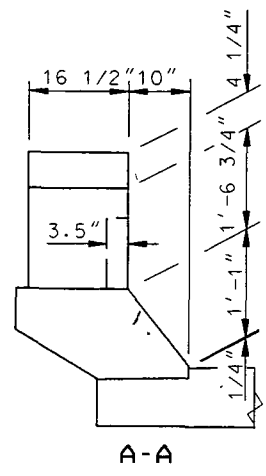
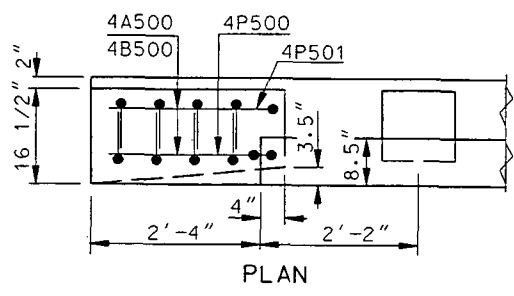
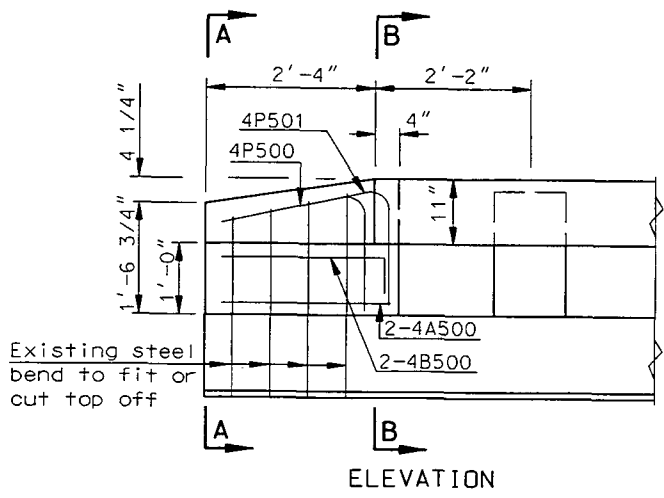
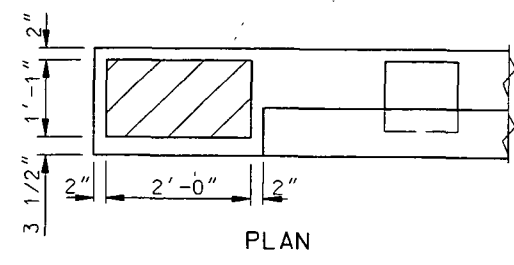
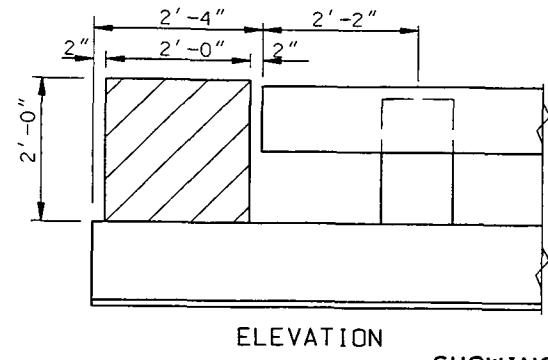
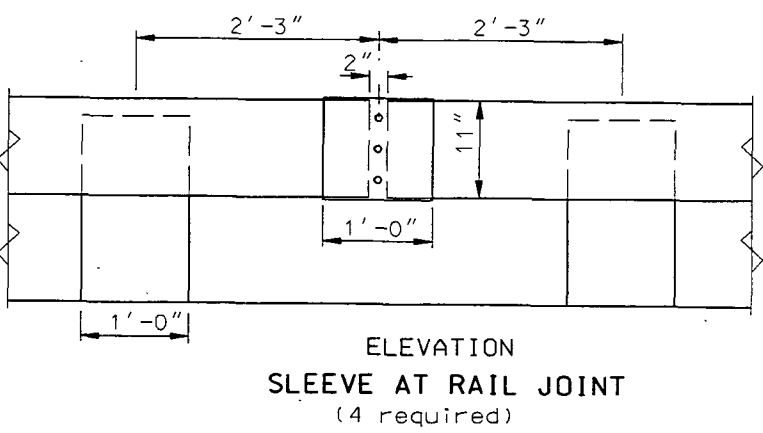
FHWA REGION	STATE	FEDERAL AID PROJECT NUMBER	SHEET NO.
8	ND	IM-8-029(042)100	40

BAR LIST (ONE POST)				
SIZE	MARK	NO.	LENGTH	SHAPE
4	A500	2	2'-4"	STR.
4	B500	2	2'-10"	BENT
4	P500	1	3'-4"	BENT
4	P501	1	3'-8"	BENT

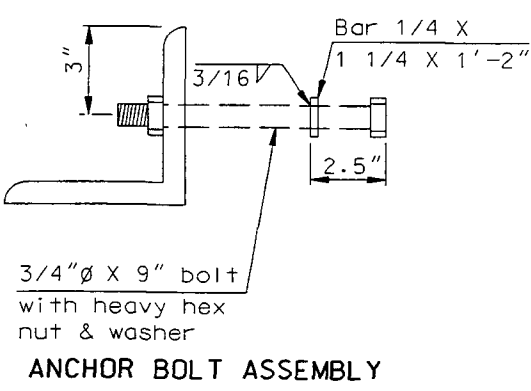
ESTIMATE OF QUANTITIES		
REMOVAL OF CONC.	0.16	C.Y.
CLASS AAE-3 CONC.	0.21	C.Y.
REINFORCING STEEL	12	LBS.



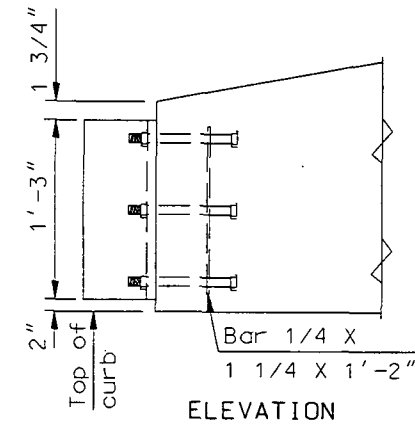
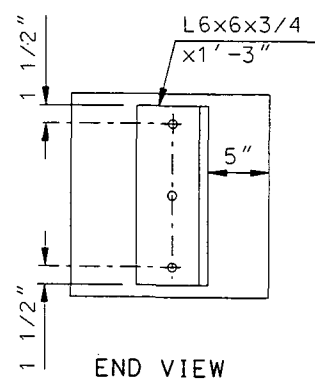
NOTE:
 The steel plates shall be M183 steel. Plates, bolts and nuts shall be galvanized in accordance with AASHTO M111. Materials and labor to install the rail sleeves shall be incidental to "BRIDGE END POST MODIFICATION".



NOTE:
 Modify bridge end posts at the north end of the bridge.
 The concrete shall be class AAE-3 and the reinforcing steel shall be Grade 60. The existing end posts shall be removed and properly disposed of. The quantities shown are for informational purposes only. All materials, labor and equipment including concrete, structural steel and reinforcing bars required to remove and replace the end posts shall be included in the pay item "BRIDGE END POST MODIFICATION".
 Surface finish "D" shall be required for all surfaces of the end posts.



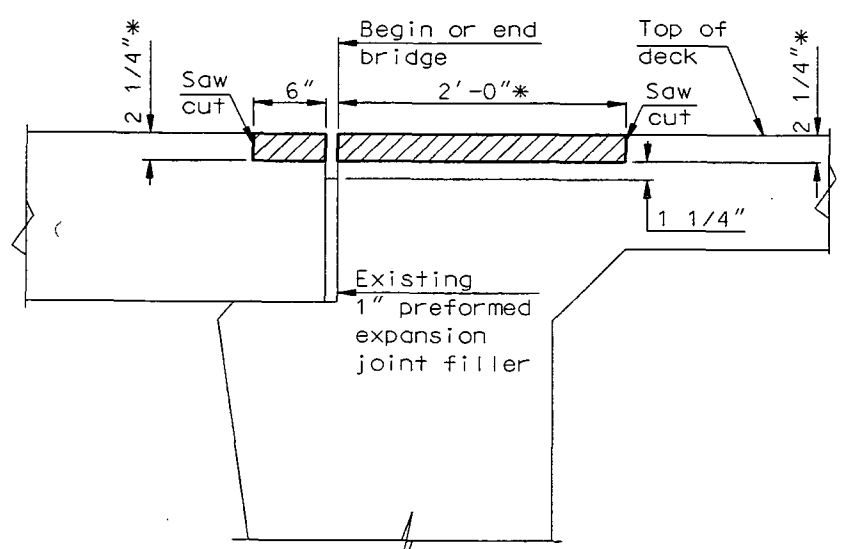
NOTE:
 The 6 x 6 x 3/4 angle shall be used in lieu of the MC8x20 shown on Standard D-764-3.



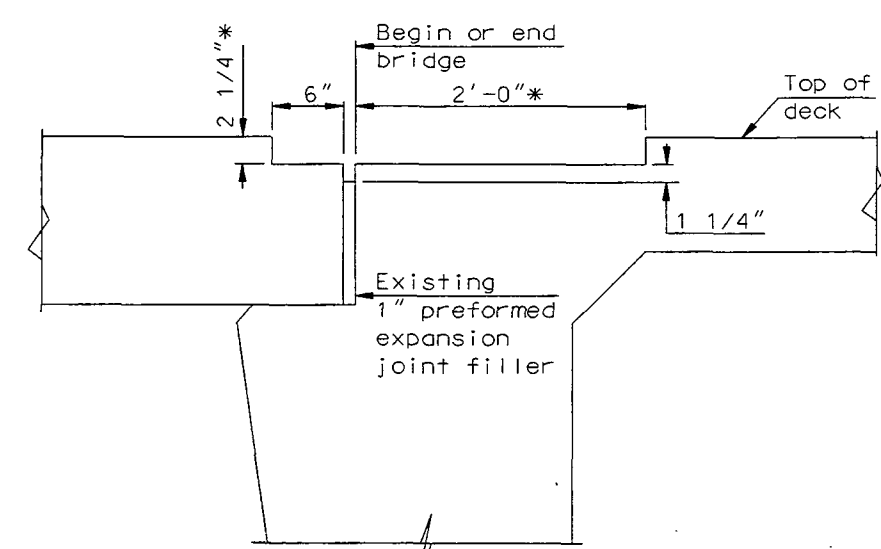
QUANTITIES		
BRIDGE END POST MODIFICATION	2	EA

GOOSE RIVER
 RAIL SLEEVES &
 END POST DETAILS

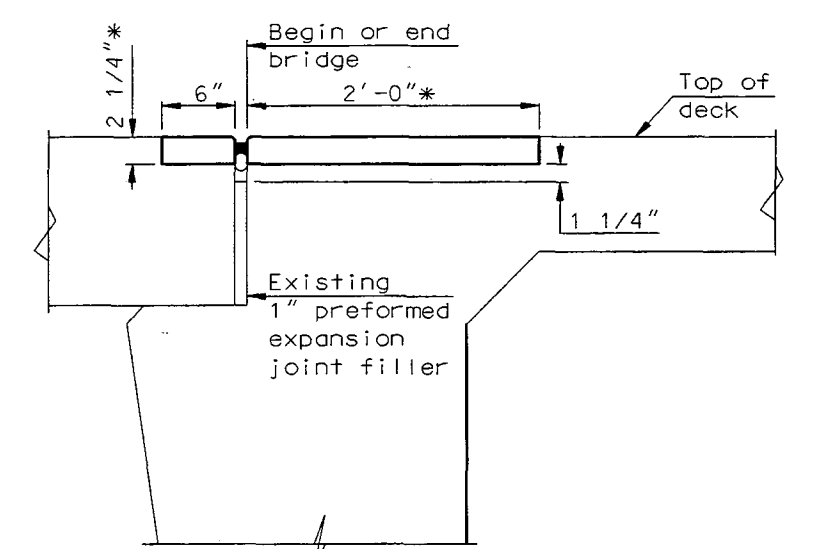
INDICATES CONCRETE REMOVAL



STAGE 1



STAGE 2

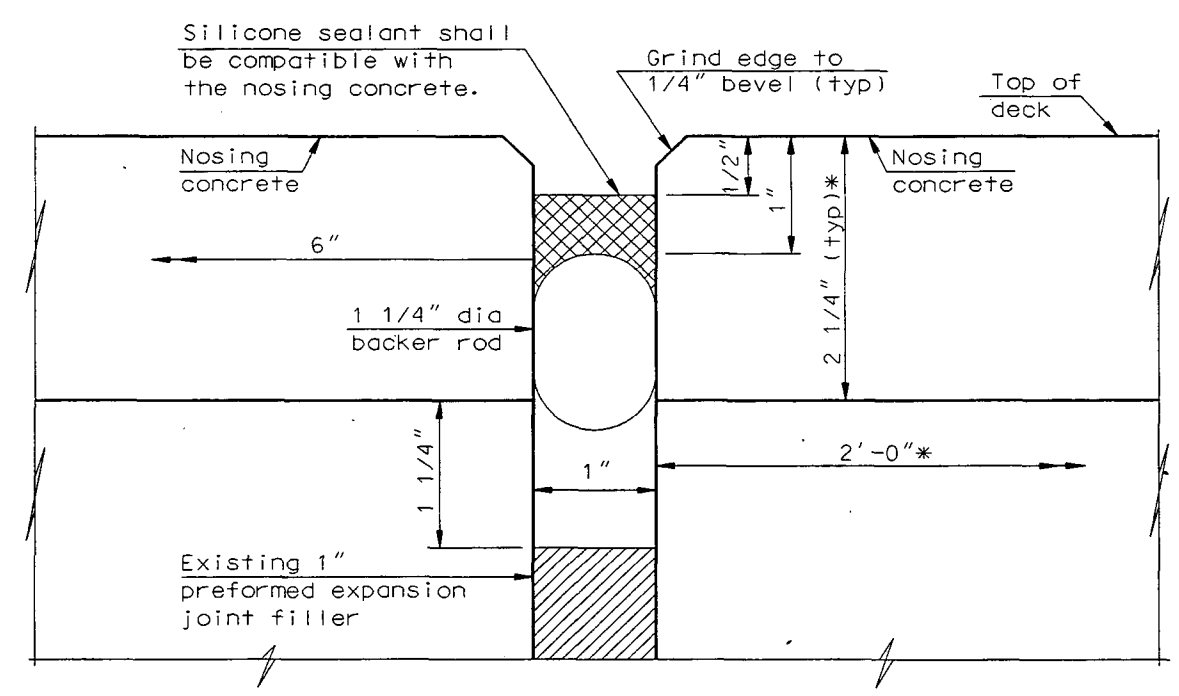


STAGE 3

* The width and thickness of the nosing concrete may be adjusted by the field engineer to match the actual deterioration.

APPROACH SLAB - BRIDGE DECK JOINT

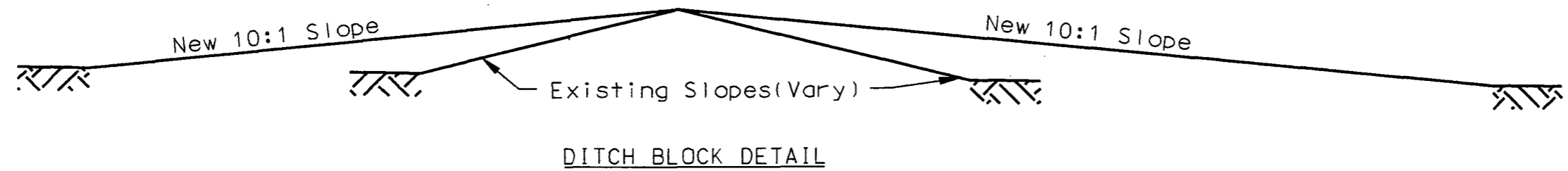
- STAGE 1:
1. Remove concrete at ends of deck and approach slabs to allow for nosing concrete.
- STAGE 2:
2. Remove the existing 1" preformed expansion joint filler to a depth of 3 1/2" below the top of the deck.
- STAGE 3:
3. Place nosing concrete in the blockout areas, both in the deck and in the approach slab.
 4. After the nosing concrete has cured, grind the 1/4" bevel edge, clean and prepare the joint, apply any necessary bonding material, install the backer rod and the silicone sealant.



JOINT DETAIL A

QUANTITIES (TWO APPROACHES)	
Nosing Concrete	34.7 CF
Silicone Sealant	74 LF

GOOSE RIVER
 APPROACH SLAB JOINT DETAILS



Note: Embankment to complete the specified work shall be included in the cost of "Flatten Ditch Block". Embankment shall be obtained from outside the R/W, and placed as specified by Section 203.02 I.

Removing, stockpiling, and replacing topsoil, and seeding shall be included in the cost of "Flatten Ditch Block". Seeding shall match the guardrail embankment seeding specifications.

203 0206 Flatten Ditch Block

Location	Quantity
RP 100.772 - Lt	1 EA
RP 101.647 - Lt	1 EA
RP 109.432 - Lt	1 EA
RP 109.714 - Lt	1 EA

TYPICAL SECTIONS
SLOPE FLATTENING

29-104.576

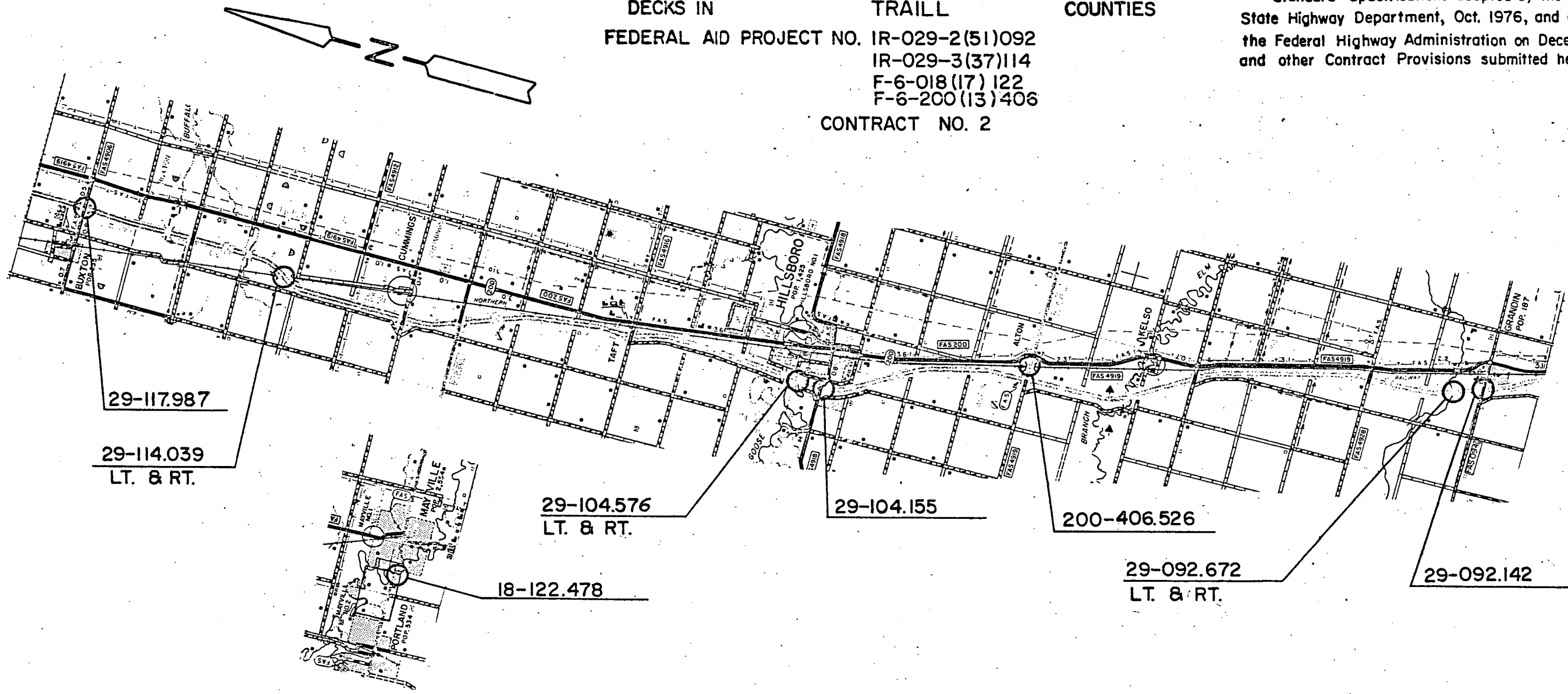
EHW REGION	STATE	PROJECT	SHEET NO.
8	N. D.	IR-029-2(51)092 IR-029-3(37)114, F-6-018(17)122 F-6-200(13)406	1

NORTH DAKOTA STATE HIGHWAY DEPARTMENT

REPAIR & OVERLAY PORTLAND CEMENT CONCRETE BRIDGE
DECKS IN TRAILL COUNTIES
FEDERAL AID PROJECT NO. IR-029-2(51)092
IR-029-3(37)114
F-6-018(17)122
F-6-200(13)406
CONTRACT NO. 2

GOVERNING SPECIFICATIONS

Standard Specifications adopted by the North Dakota State Highway Department, Oct. 1976, and approved by the Federal Highway Administration on December 17, 1976, and other Contract Provisions submitted herewith.



29-117.987

29-114.039
LT. & RT.

29-104.576
LT. & RT.

29-104.155

200-406.526

29-092.672
LT. & RT.

29-092.142

18-122.478

APPROVED DATE 9-23-82

James D. Brown
BRIDGE ENGINEER
NORTH DAKOTA
STATE HIGHWAY DEPARTMENT

APPROVED DATE 9-24-82

Roy Zink
CHIEF ENGINEER
NORTH DAKOTA
STATE HIGHWAY DEPARTMENT



U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

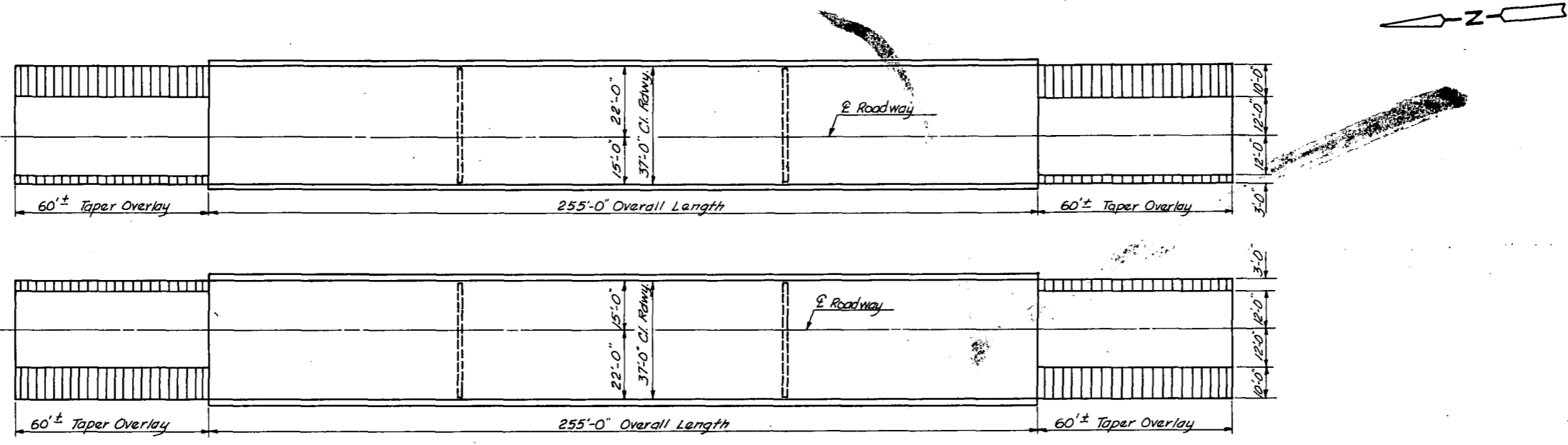
APPROVED

DIVISION ENGINEER _____ DATE _____

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	ND.	IR-029-2(51)092	2

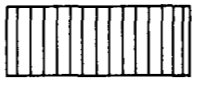
IR-029-3(37)114, F-6-018(17)122
F-6-200(13)406

<u>SHEET NO.</u>	<u>DESCRIPTION</u>	<u>SHEET NO.</u>	<u>STANDARD DRAWINGS</u>
1	Title Sheet	26	D-708-1 Valley Gutter and Curb & Gutter
2	Table of Contents	27	D-708-6 Combined Concrete Curb & Gutter for Bridge Approaches
3	Notes, Quantities and Special Provisions	28	D-722-1 Beam Guardrail General Details
4	Grandin Interchange	29	D-722-14 Box Beam Guardrail at Bridge Ends
5 - 6	South Branch of Elm River	30	D-722-15 Box Beam Guardrail at Bridge Ends
7	BNRR Separation	31	D-722-16 Box Beam Guardrail at Bridge Ends Attachment Details
8	Hillsboro Interchange	32	D-722-18 Box Beam Guardrail at Bridge Ends Attachment Details
9	Goose River at Hillsboro		
10	Goose River at Mayville		
11-16	Guardrail Details - Goose River West of Mayville		
17-18	BNRR Separation		
19	Buxton Interchange		
20	Overlay Details		
21	Sign Layout for Left Lane Closure Details		
22	Sign Layout for Right Lane Closure Details		
23	Attenuation Device Detail		
24	Sign Layout for One Lane Closure (Two Lane Roadway) Detail (Hillsboro Interchange, Goose River at Mayville, BNRR Sep. East of the Blanchard Interchange)		
25	Construction Sign Layout for Low Volume Interchanges		



PLAN

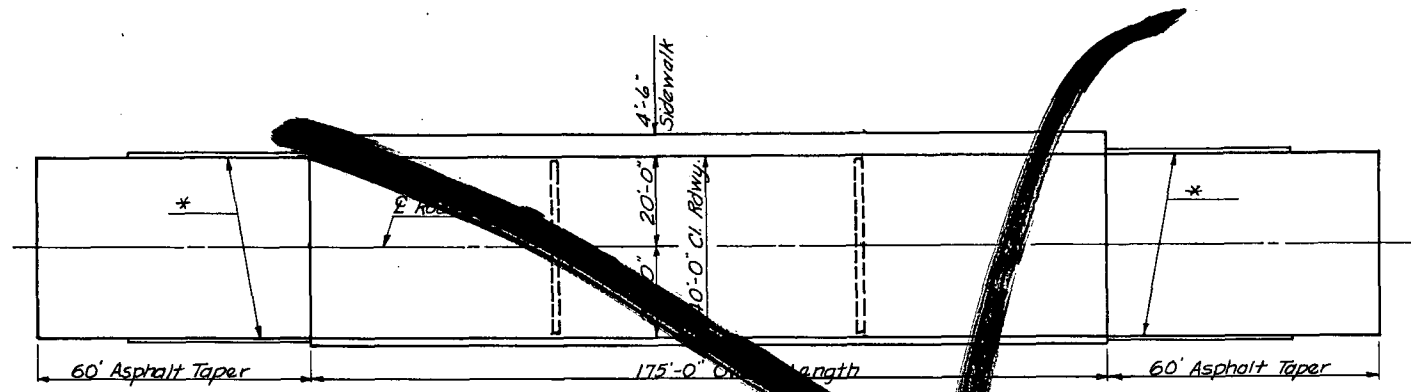
37'-0" Clear Roadway
Deck drains on both bridges



ASPHALT OVERLAY

Asphalt shoulders to be brought to final grade with an asphalt overlay and shall be incidental to "Taper Overlay".

GOOSE RIVER AT HILLSBORO

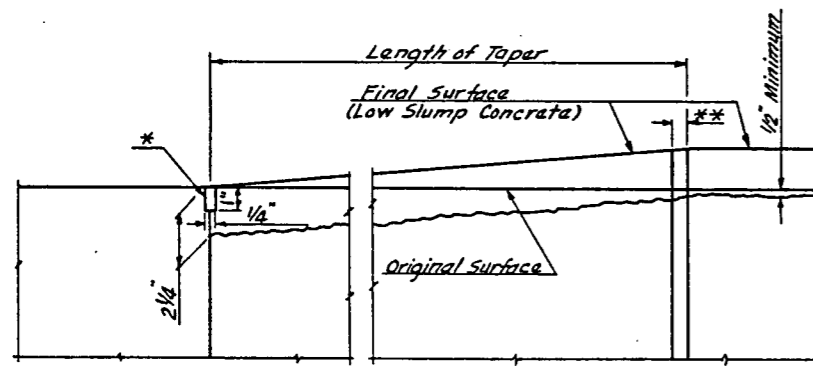
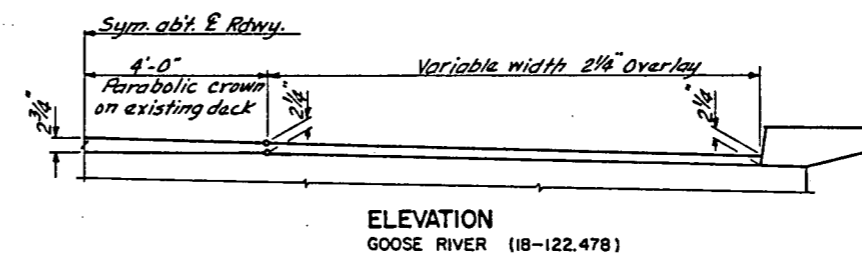
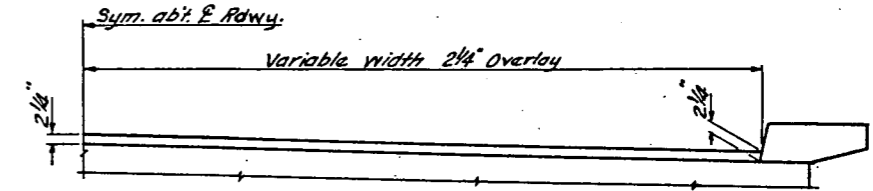
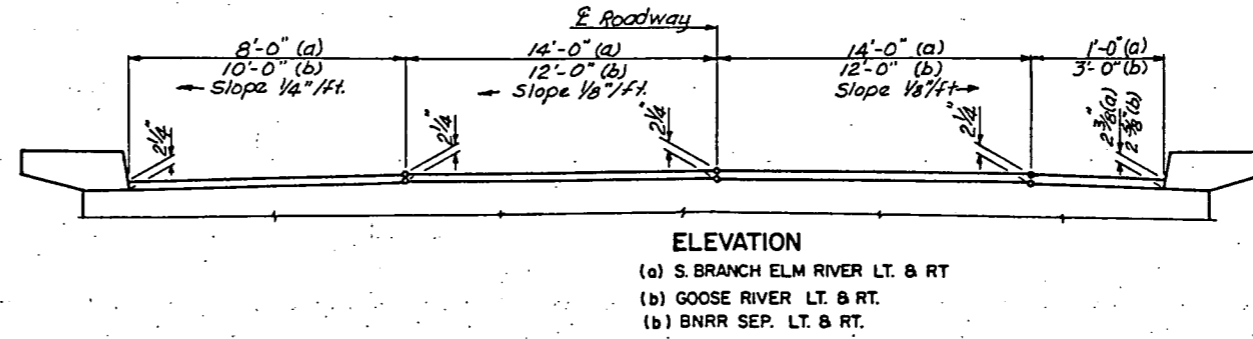
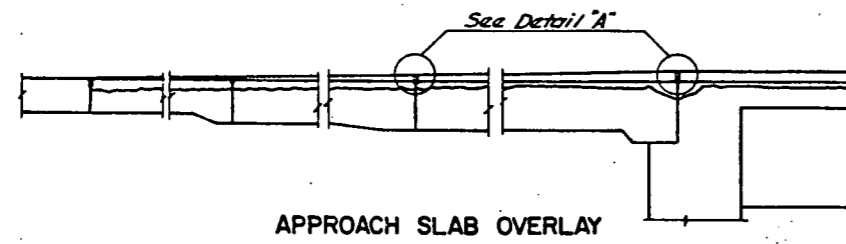


PLAN
 40'-0" Clear Roadway
 Deck drains on bridge

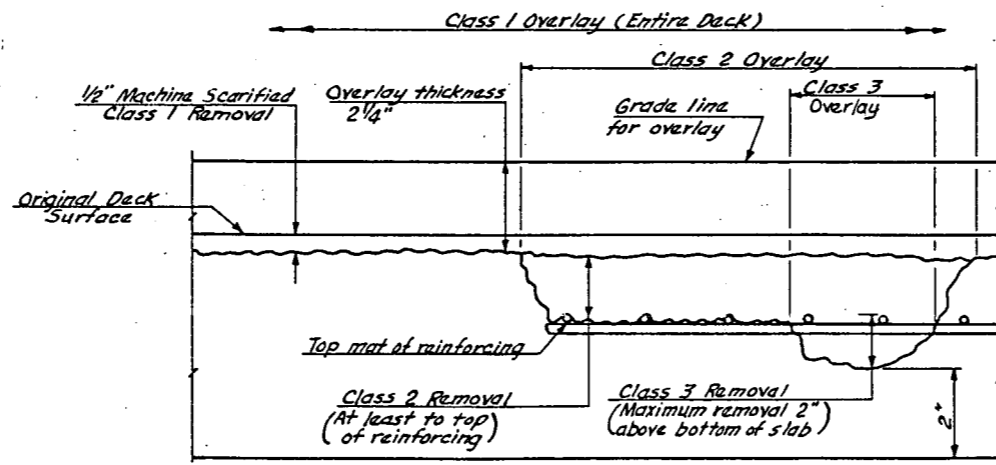
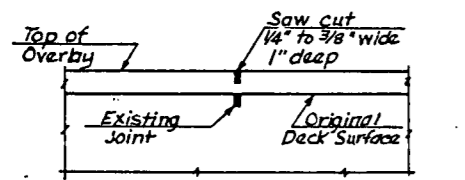
* Remove existing curb & gutter on all 4 corners of structure. Replace curb & gutter on East & West approaches, on the North side only, as shown on Standard D-708-6. Removal and replacement to be paid for under bid item "Curb and Gutter Type 1 Reinforced".

NOTE: Remove existing guardrail and replace with Box Beam Guardrail as shown on Standard's D-2-14, 15, 16 and 17. To be paid for under bid item "Box Beam Guard Rail", "Box Beam Guardrail - End Treatment" and "W-Beam Guardrail - Flared End Treatment Transition". Guardrail which is removed shall be salvaged and stockpiled at the North Dakota State Highway Department Maintenance yard at Mayville, N.D. Any embankment, if needed, shall be incidental to the payment "Box Beam Guard Rail".

GOOSE R
 AT MAYVILLE

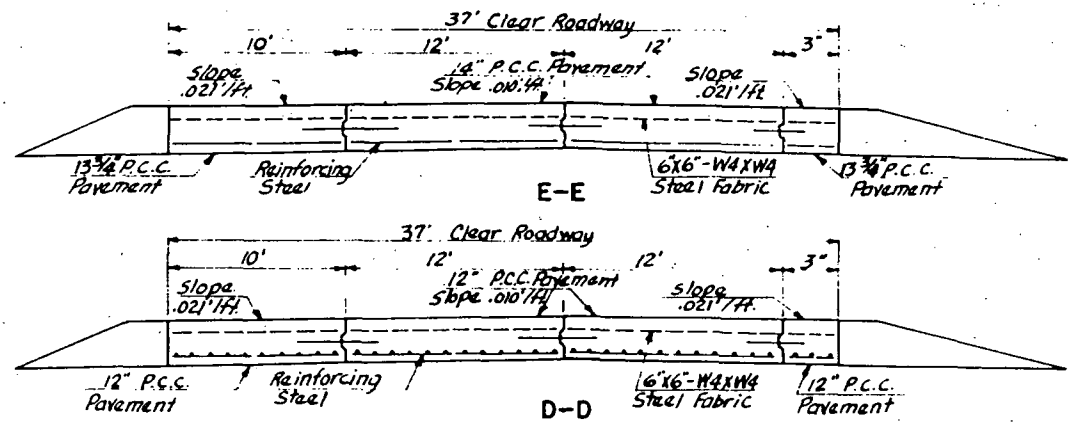
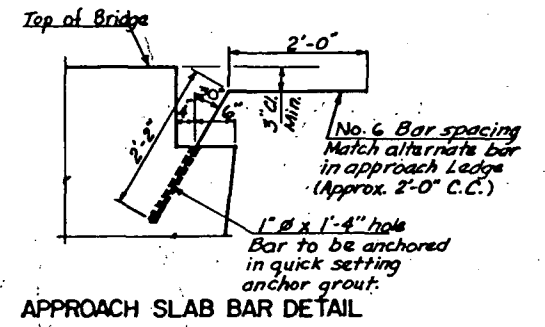
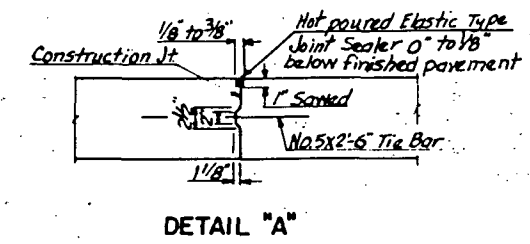
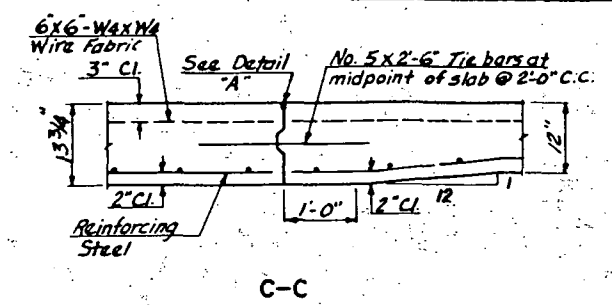
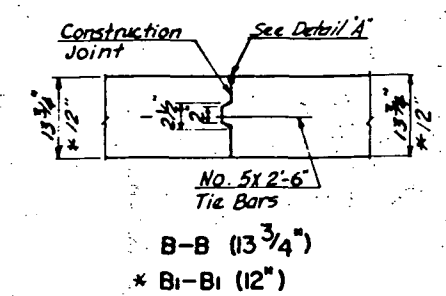
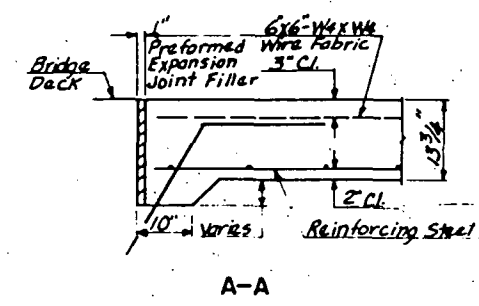
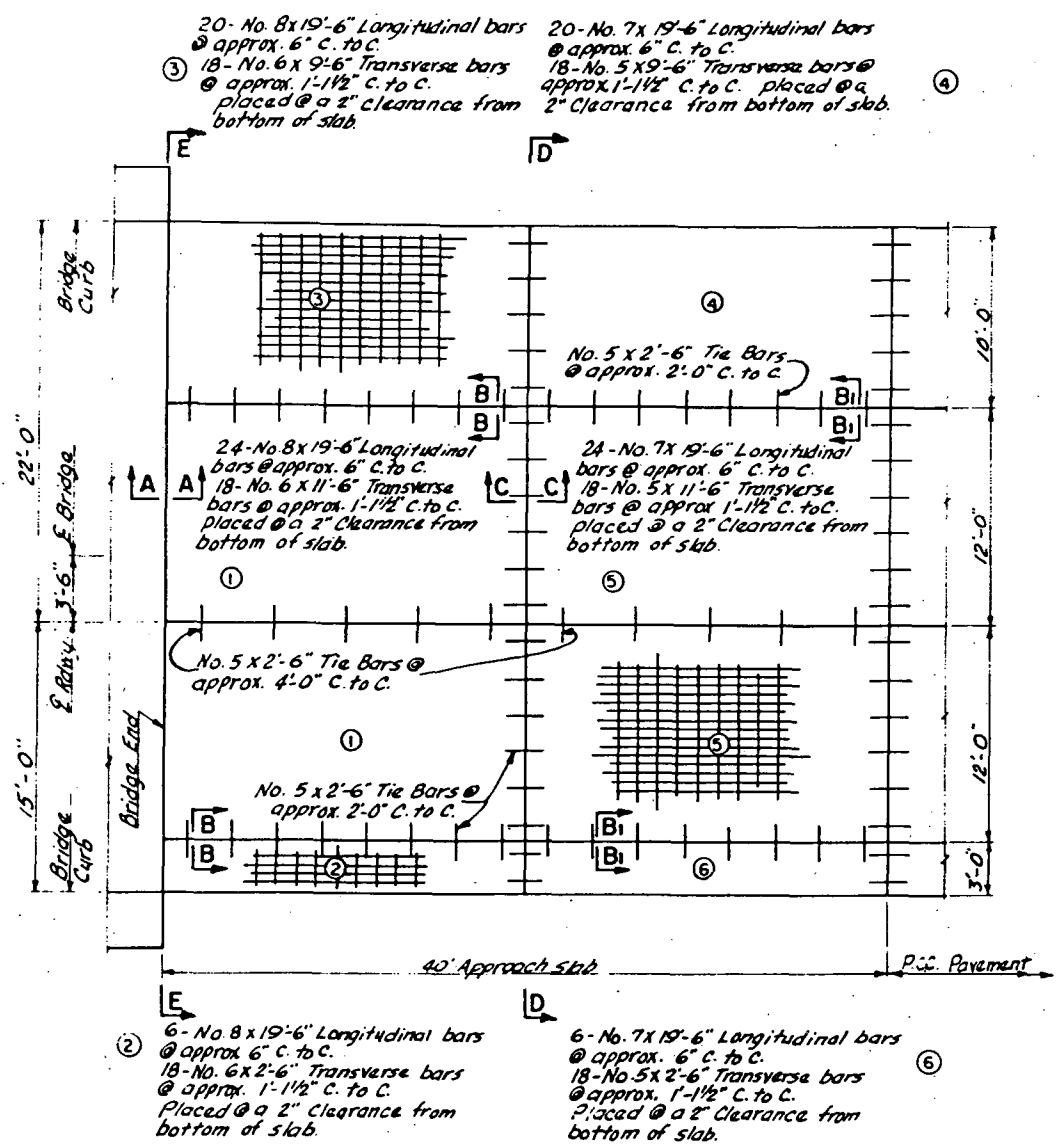


* To be filled with hot poured joint filler
 ** Remove existing pre-molded joint filler & fill hot poured joint filler. Width 1/2".



NOTE:
 Maximum limits for Class 3 Overlay will be determined in the field, but will be less than full depth of slab. (See Special Provisions.)

OVERLAY DETAILS



NOTE: 2" x 42" Recycled Hot Bituminous Pavement shall be placed beneath the approach slab. 42 tons are provided for in the mainline quantities.

NOTE: All labor and materials required in removal and replacement of approach slabs will be incidental to the pay item "Approach Slab (Remove & Replace)". Quantity for 2 Approach Slabs = 329 S.Y.

Panels No. 1, 2 & 3		Panels No. 4, 5 & 6	
21 - No. 6 x 4'-2"	131	74 - No. 7 x 19'-6"	2,950
74 - No. 8 x 19'-6"	3,853	36 - No. 5 x 11'-6"	432
36 - No. 6 x 11'-6"	622	18 - No. 5 x 9'-6"	178
18 - No. 6 x 9'-6"	257	40 - No. 5 x 2'-6"	104
18 - No. 6 x 2'-6"	68		3,664 lbs.
40 - No. 5 x 2'-6"	104		
	5,035 lbs.		

Wire Mesh 76 S.Y. Wire Mesh 76 S.Y.
Bar list for information purposes only.

GOOSE RIVER
BRIDGE APPROACH SLAB
37'-0" CLEAR ROADWAY

DESIGN DATA				
Traffic	Average Daily			Est. Max. Hr.
Current 2000	Pass: 3,385	Trucks 835	4,220	425
Forecast 2020	Pass: 5,420	Trucks 1,420	Total 6,840	685
Minimum Sight Dist. for:		Design Speed 70		
Stopping 625'		Bridges		
Limited Access Control				

JOB# 15

ERWA REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	IM-8-029(040)099	1

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION

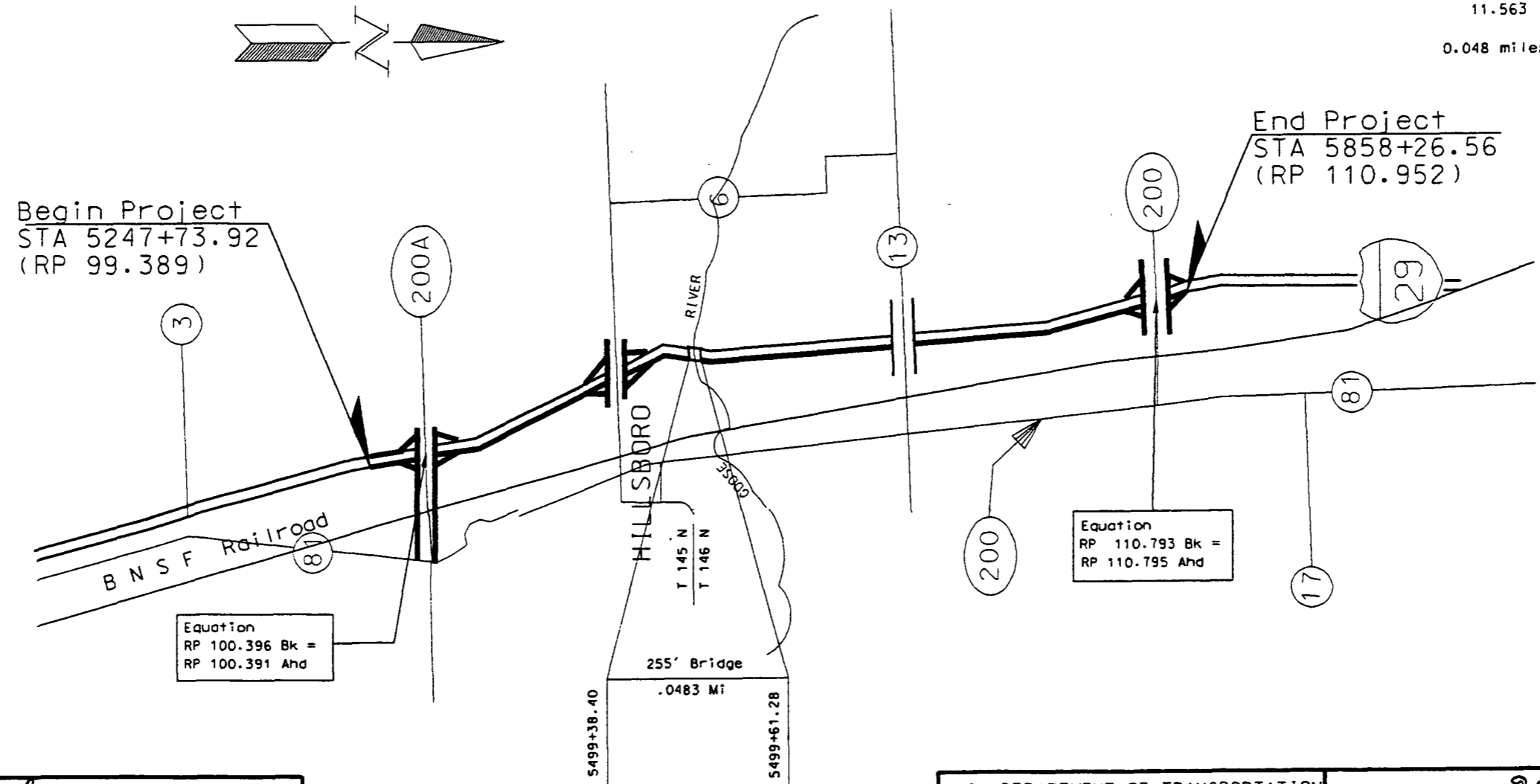
Federal Aid Project IM-8-029(040)099
 In Traill County
 Concrete Pavement Repair, Dowel Bar Retrofit,
 Grinding, Mill and Overlay
 (Northbound Roadway)
 Mill and Overlay (Southbound Roadway)

GOVERNING SPECIFICATIONS:

Standard Specifications adopted by the North Dakota Department of Transportation October 1997; Standard Drawings currently in effect; and other Contract Provisions submitted herein.

LENGTH OF PROJECT

Miles Gross	Miles Net
11.563	11.515
0.048 miles deducted for bridge	



DESIGNER *Sergiusz Marchl*
 DESIGNER _____
 DESIGNER _____
 RECOMMEND APPROVAL _____, 19____
 DESIGN ENGINEER _____

U.S. DEPARTMENT OF TRANSPORTATION
 FEDERAL HIGHWAY ADMINISTRATION

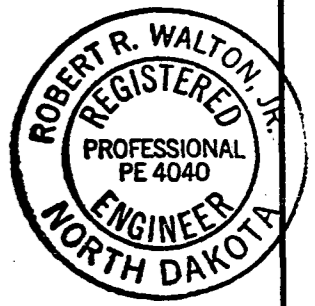
APPROVED _____
 DIVISION ADMINISTRATOR

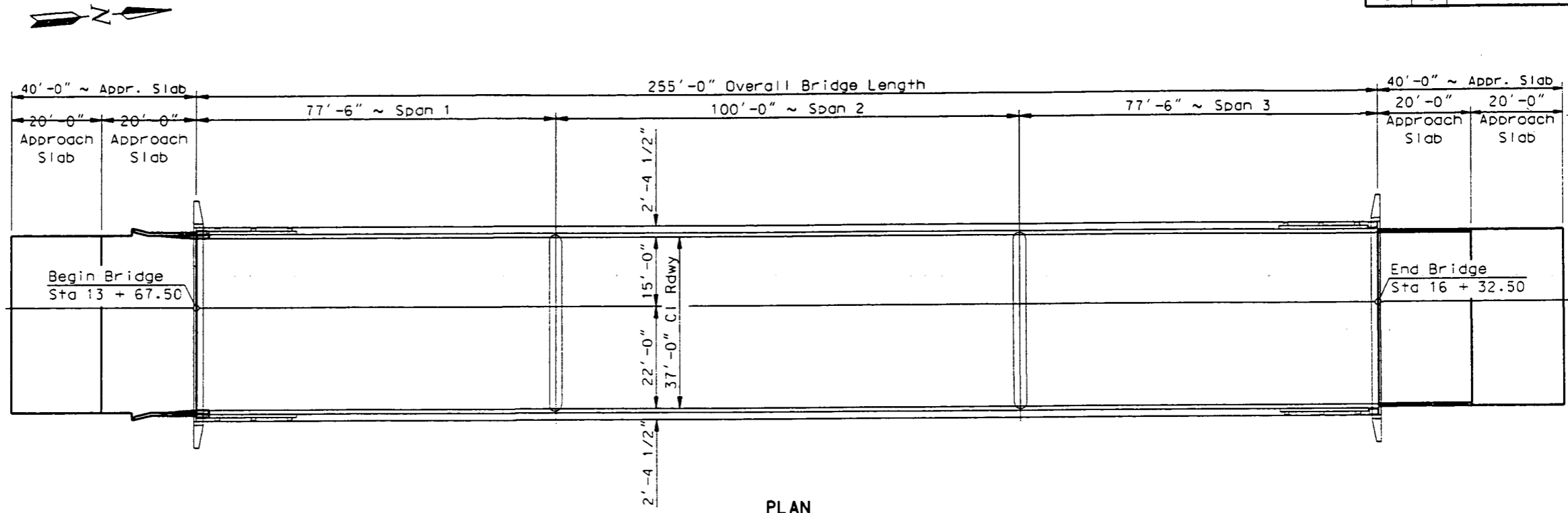
DATE _____

APPROVED DATE 9 MAR 00

Robert R. Walton, Jr.
 DISTRICT ENGINEER

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION





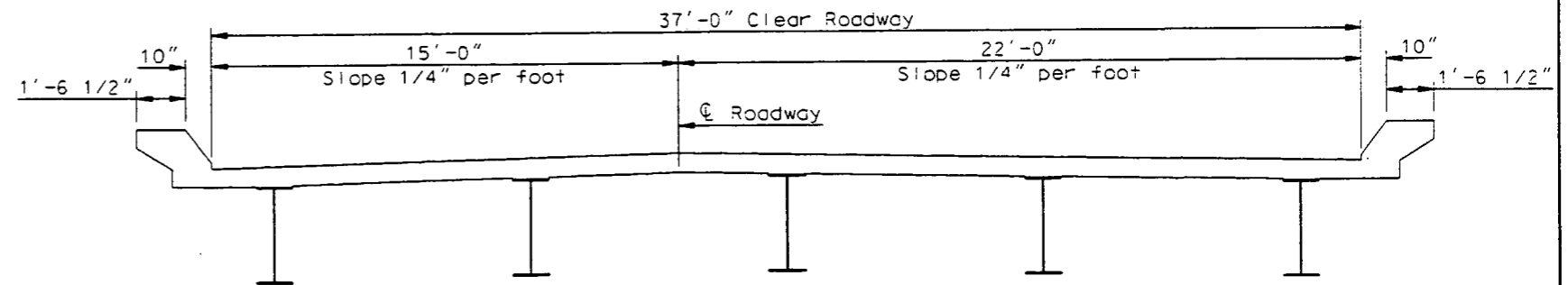
PLAN

NOTES:

930 NOSING CONCRETE: The nosing concrete material shall be an elastomeric concrete or a polymeric concrete that will provide a durable edge that can withstand live-load traffic without chipping or spalling. The nosing concrete material shall be SILSPEC 900 PNS, manufactured by Silicone Specialties Inc.; WABOCRETE II, manufactured by Watson Bowman Acme; ELASTOMERIC CONCRETE, manufactured by D.S. Brown Company or an approved equal. The nosing concrete shall be mixed and installed according to the manufacturer's recommendation. All labor and materials required to install the nosing concrete shall be included in the bid item "Nosing Concrete".

930 SILICONE: The silicone sealant shall be a rapid cure, self leveling, cold applied, two component silicone sealant that will bond to and be compatible to the nosing concrete used. The sealant shall be installed according to the manufacturer's recommendations. The silicone sealant and the nosing concrete must be supplied by the same manufacturer as a complete system. The backer rod and any necessary bonding materials shall be included in the bid item "Silicone Sealant".

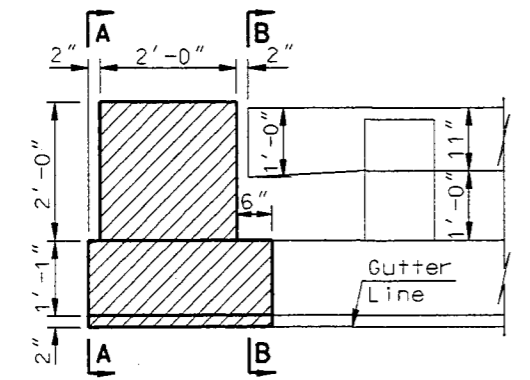
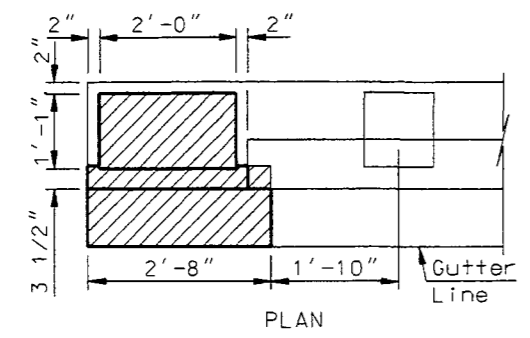
930 TECHNICAL ASSISTANCE: The Contractor shall acquire a technical assistance from the manufacturer of the nosing concrete and the silicone sealant for the surface preparation and installation of the nosing concrete and silicone sealant. A technical representative must be present for the start of surface preparation and installation for at least one day. The Contractor shall contact the manufacturer at least two weeks prior to the installation. The technical assistance shall be provided at no additional cost to the Department.



TYPICAL DECK SECTION

SPEC	CODE	ITEM DESCRIPTION	UNIT	QUANTITY
550	0217	BRIDGE APPROACH SLAB - REMOVE & REPLACE	SY	340.2
602	1210	BRIDGE END POST MODIFICATION	EA	2
930	8642	NOSING CONCRETE	CF	12.6
930	8644	SILICONE SEALANT	LF	74

NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION
GOOSE RIVER
BRIDGE LAYOUT
PROJECT: IM-8-029(040)099
STATION 328 + 88.5
TRAILL COUNTY



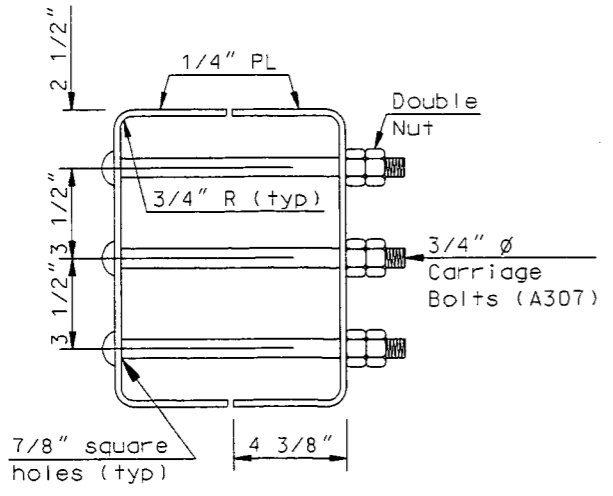
Hatched areas indicate concrete to be removed.

Care shall be taken to ensure no damage is done to the existing reinforcing steel that is to remain in place.

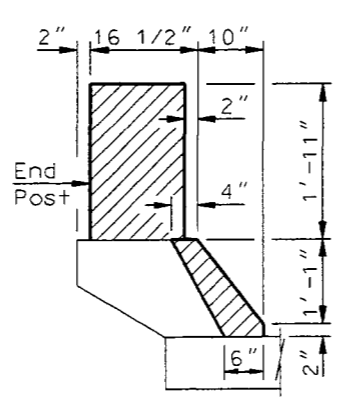
END POST REMOVAL DETAILS

NOTE:

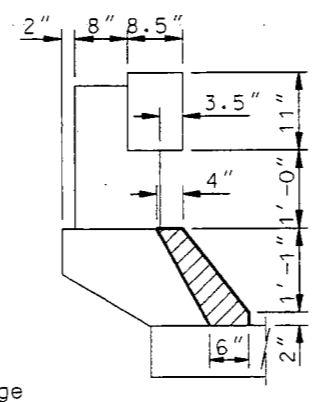
The steel plates shall be M183 steel. Plates, bolts and nuts shall be galvanized in accordance with AASHTO M111. All materials and labor required to install the rail sleeves shall be included in the pay item "Bridge End Post Modification".



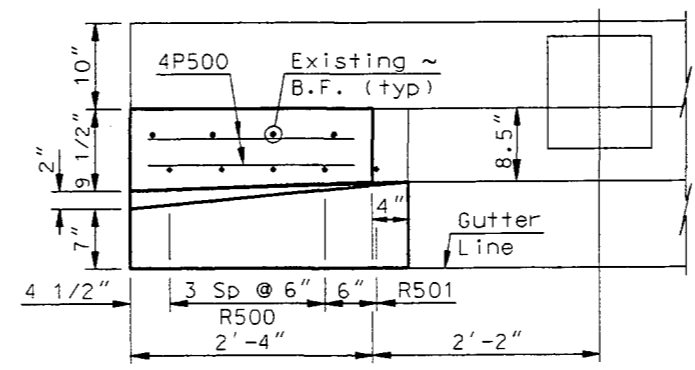
SLEEVE DETAIL



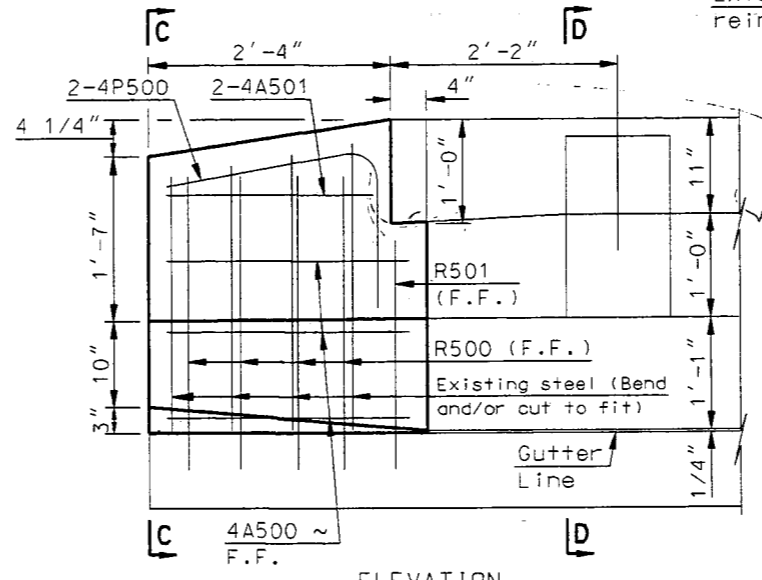
A-A



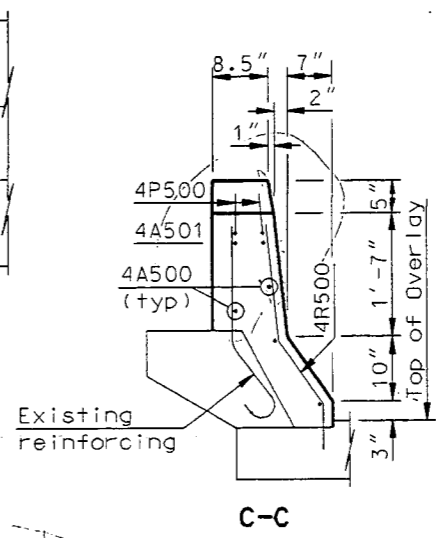
B-B



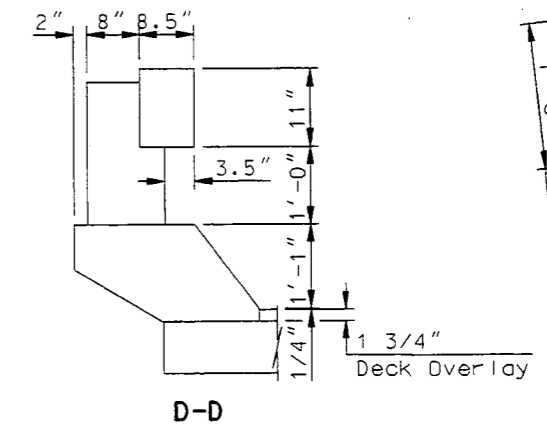
PLAN



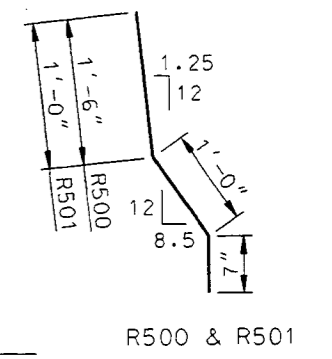
**ELEVATION
 END POST MODIFICATION DETAILS**



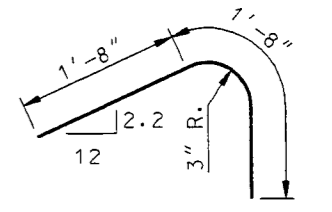
C-C



D-D



R500 & R501



P500

DIMENSIONS SHOWN ARE OUT TO OUT
BENT BAR DETAILS

QUANTITIES	
BRIDGE END POST MODIFICATION	2 EA

GOOSE RIVER	
RAIL SLEEVE & END POST MODIFICATION DETAILS	

NOTE:

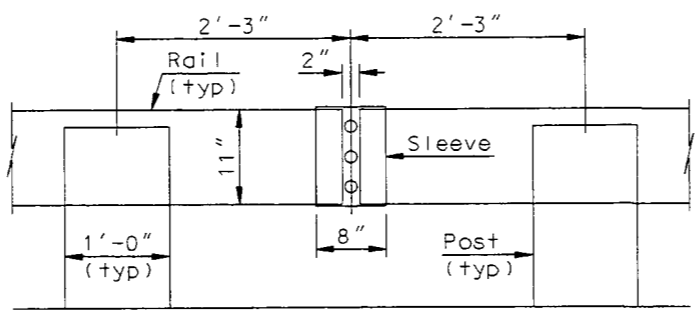
The concrete shall be class AAE-3 and the reinforcing steel shall be grade 60.

The existing end posts shall be removed and properly disposed of.

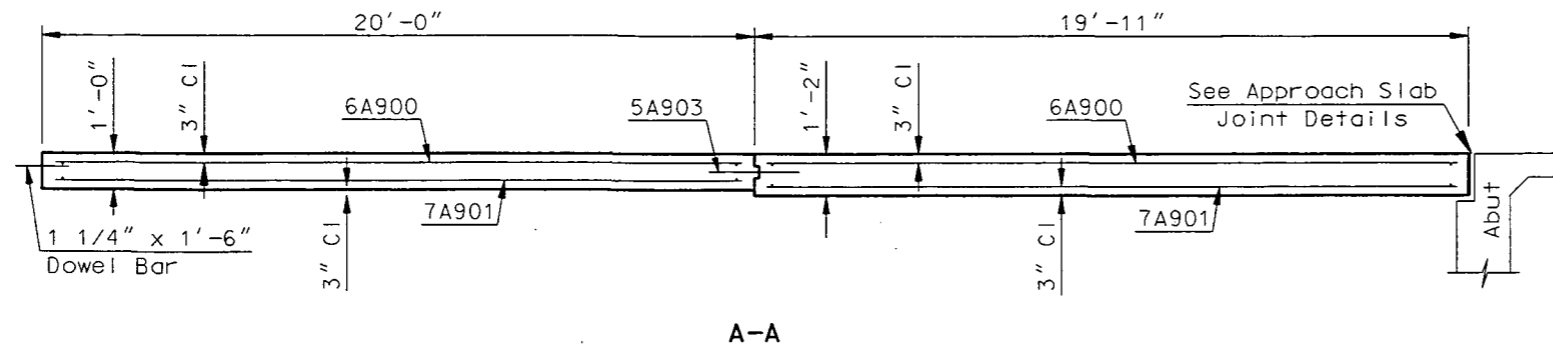
The 4R500's & 4R501's shall be installed according to the manufacturers recommendations, with a high strength adhesive specifically intended for concrete anchorage in accordance with section 806.02 of the NDDOT Standard Specifications.

The quantities shown are for informational purposes only. All materials, labor and equipment including concrete and reinforcing bars required to remove and replace the end posts shall be included in the pay item "Bridge End Post Modification".

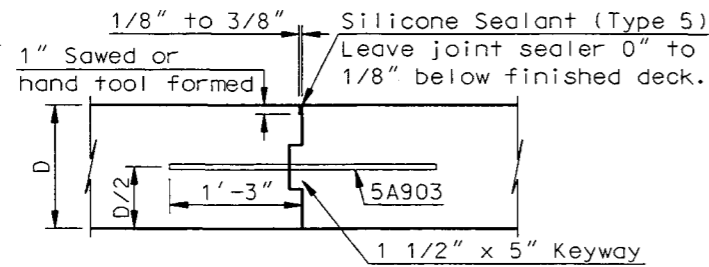
Surface Finish "D" shall be required for all surfaces of the end posts.



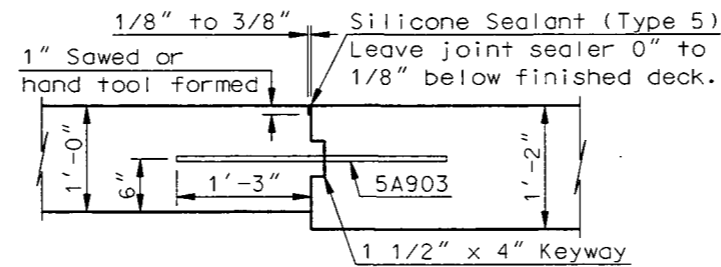
(4 Required)
**ELEVATION
 SLEEVE AT RAIL JOINT**



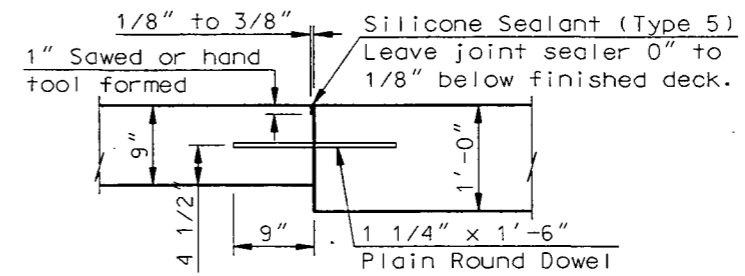
A-A



JOINT DETAIL B



JOINT DETAIL C



JOINT DETAIL D

SKEW ANGLE = 0°

BAR LIST - ONE SLAB			
SIZE	MARK	NO.	LENGTH
6	A900	136	19'-7"
7	A901	160	19'-7"
5	A902	16	6'-0"
5	A903	39	2'-6"
4	XA904	2	7'-6"
5	A905	52	15'-3"
5	A906	26	15'-11"
5	A907	52	22'-3"
5	A908	26	22'-11"
4	B900	112	4'-0"
4	D900	12	13'-3"
4	D901	4	4'-0"
4	XT900	18	3'-0"

ESTIMATED MATERIAL QUANTITIES	
REINFORCING STEEL (LBS.)	CONCRETE (C.Y.)
14,156	65.7

NOTE:

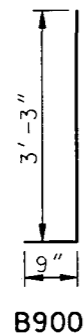
See Dwg 29-104.576R-2 for Section A-A and the location of Detail "B" and Detail "C".

See Dwg 29-104.576R-5 for Notes.

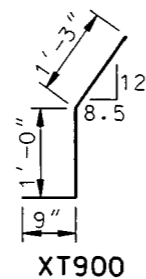
All dowel bars shall be epoxy coated and conform to AASHTO M-254 Type B.

Free ends of Type B epoxy coated dowels (minimum of one-half of dowel length plus 2 inches) shall be given a thin, uniform coating of grease. This coating shall be applied within two hours before covering with concrete.

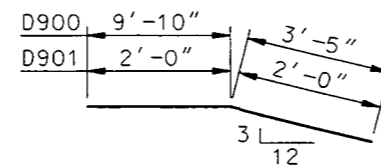
The above note applies to the transverse joint between the approach slab and the concrete pavement only. Longitudinal joints in the approach slabs shall be tied together with rebar.



B900



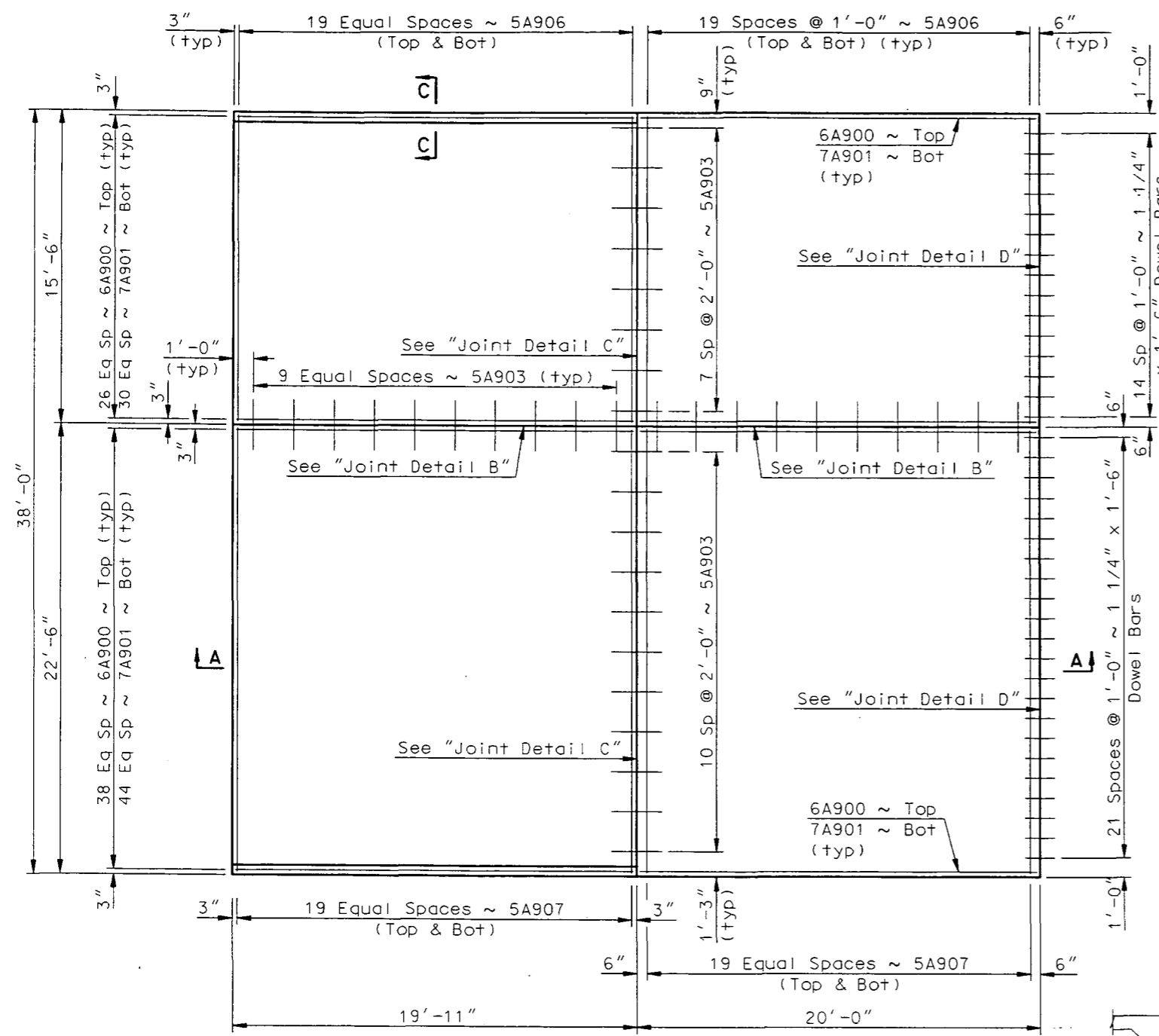
XT900



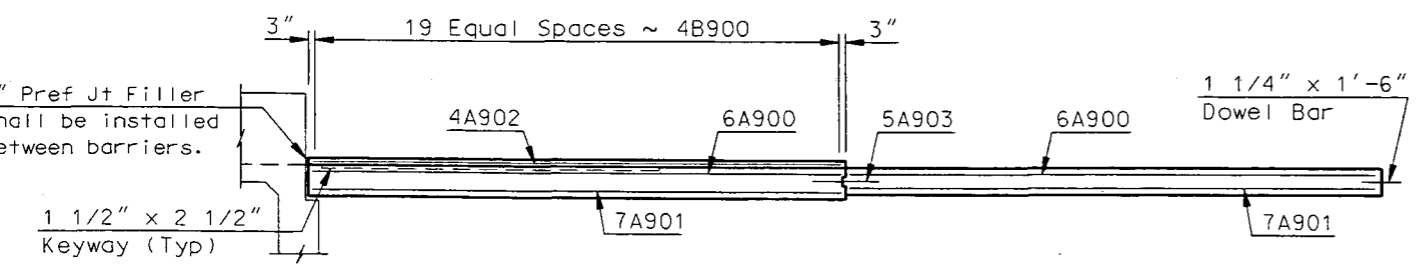
D900 & D901

QUANTITIES	(ONE SLAB)
APPROACH SLAB	171.7 SY

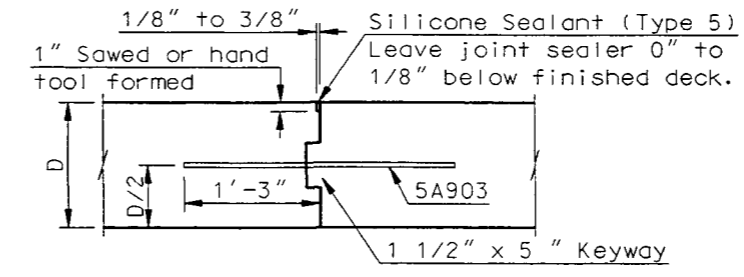
GOOSE RIVER
(AT BEGIN BRIDGE)
APPROACH SLAB DETAILS



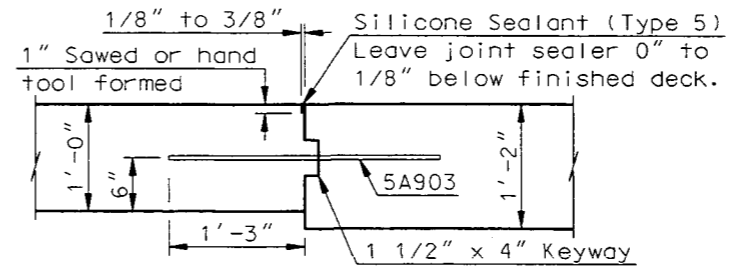
PLAN



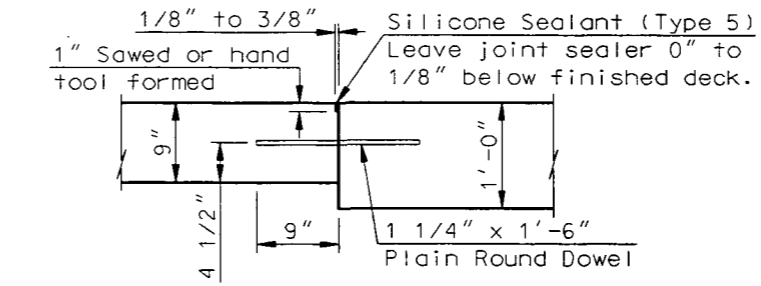
ELEVATION



JOINT DETAIL B



JOINT DETAIL C



JOINT DETAIL D

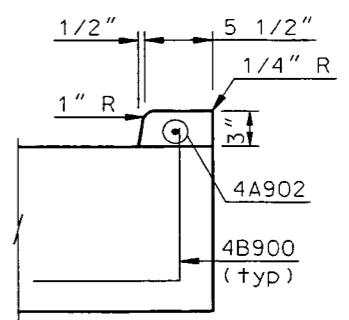
SKREW ANGLE = 0°

BAR LIST - ONE SLAB

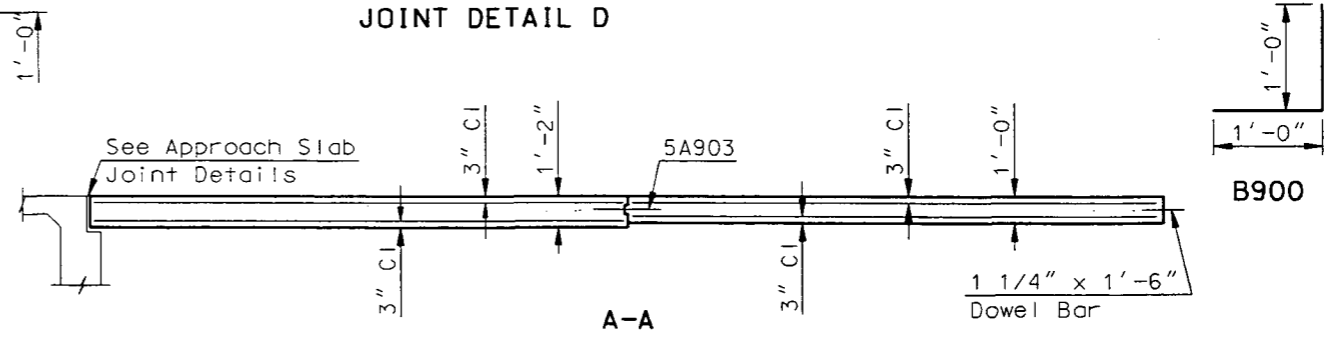
SIZE	MARK	NO.	LENGTH
6	A900	132	19'-7"
7	A901	152	19'-7"
4	A902	2	19'-7"
5	A903	39	2'-6"
5	A906	80	15'-2"
5	A907	80	22'-2"
4	B900	40	2'-0"

ESTIMATED MATERIAL QUANTITIES

REINFORCING STEEL (LBS)	CONCRETE (CY)
13,263	61.0



C-C



A-A

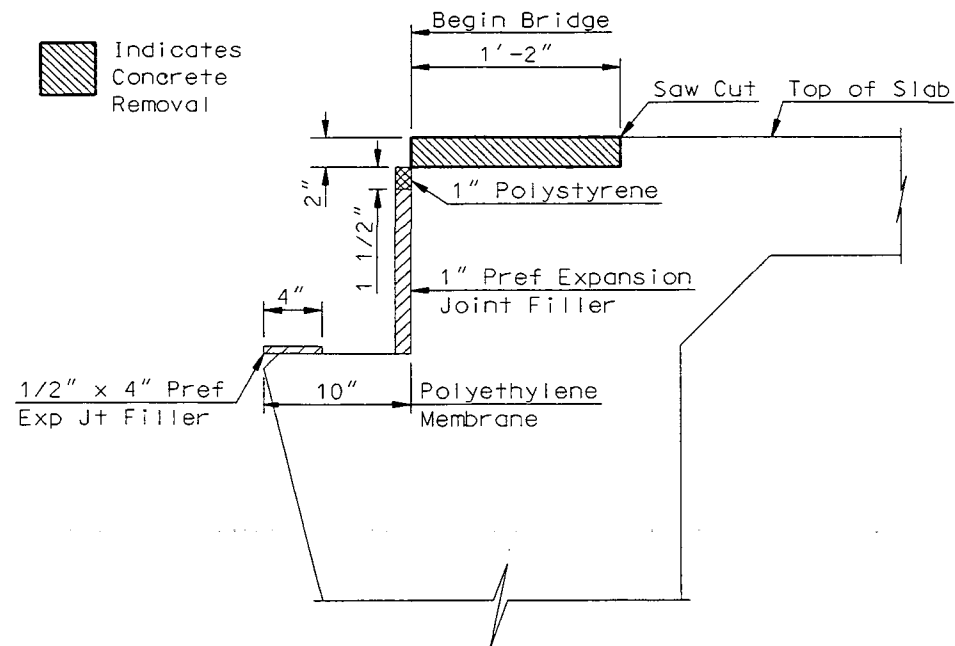
NOTE:
 See Dwg 29-104.576R-6 for Notes.

QUANTITIES (ONE SLAB)

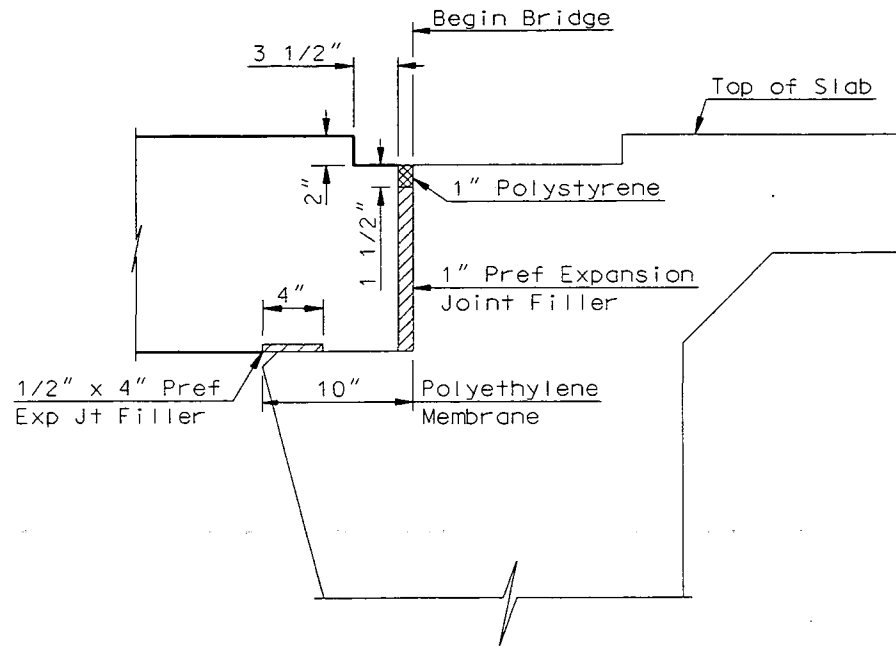
APPROACH SLAB	168.5 SY
---------------	----------

GOOSE RIVER
 (AT END BRIDGE)
 APPROACH SLAB DETAILS

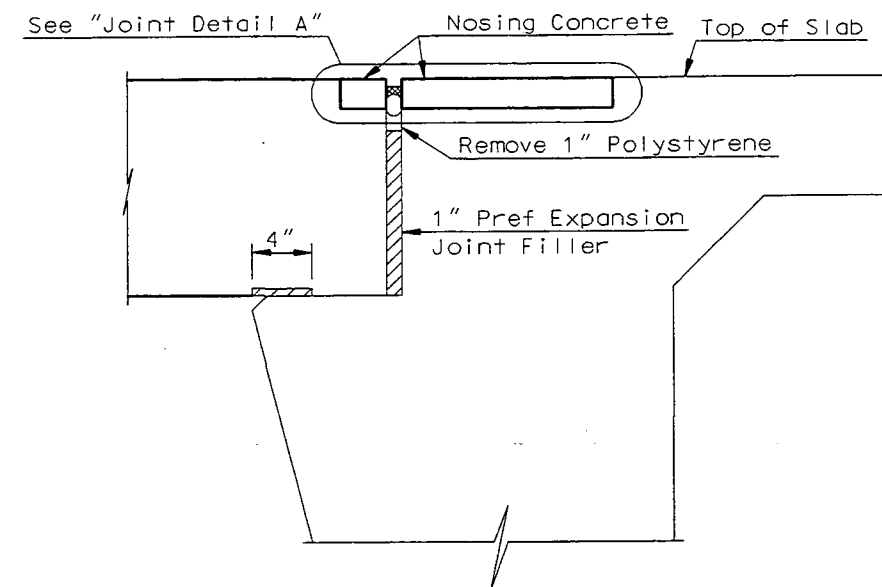
Indicates Concrete Removal



STAGE 1



STAGE 2



STAGE 3

NOTES:

STAGE 1:

1. Remove concrete at ends of deck to allow for nosing concrete.
2. Place the 1" preformed expansion joint filler, the 1/2" x 4" preformed expansion joint filler, the 1" polystyrene and the polyethylene membrane.

STAGE 2:

3. Place the new approach slab concrete. A 2" x 3 1/2" blockout shall be formed between the curbs in the approach slab as shown.

STAGE 3:

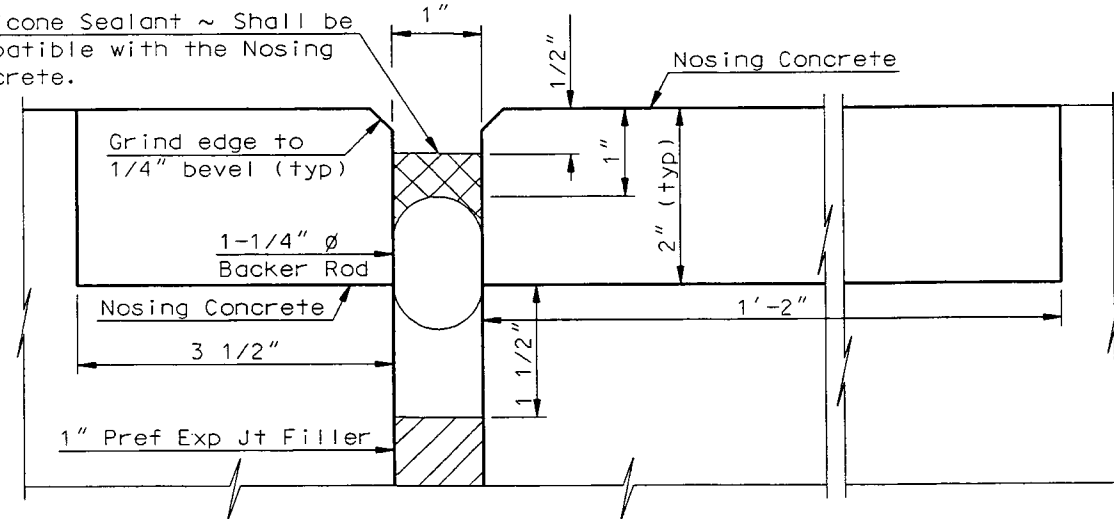
4. Remove the 1" polystyrene.
5. Place nosing concrete in the blockout areas, both in the deck and in the approach slab.
6. After the nosing concrete has cured, grind the 1/4" beveled edge. Clean and prepare the joint, apply any necessary bonding material. Install the backer rod and the silicone sealant.

All estimated material quantities shown on drawing number 29-104.576R-3 are for information purposes only. All materials including concrete, reinforcing bars, dowel bars, polyethylene membrane, preformed joint filler and labor required to build the approach slabs and approach slab barriers shall be included in the pay item, "Bridge Approach Slab - Remove & Replace".

The concrete shall be Class AE-3 and the reinforcing steel shall be Grade 60. The polyethylene membrane shall meet the requirements of AASHTO M171.

Surface Finish "D" shall be required for all surfaces of the curb transitions.

Silicone Sealant ~ Shall be compatible with the Nosing Concrete.

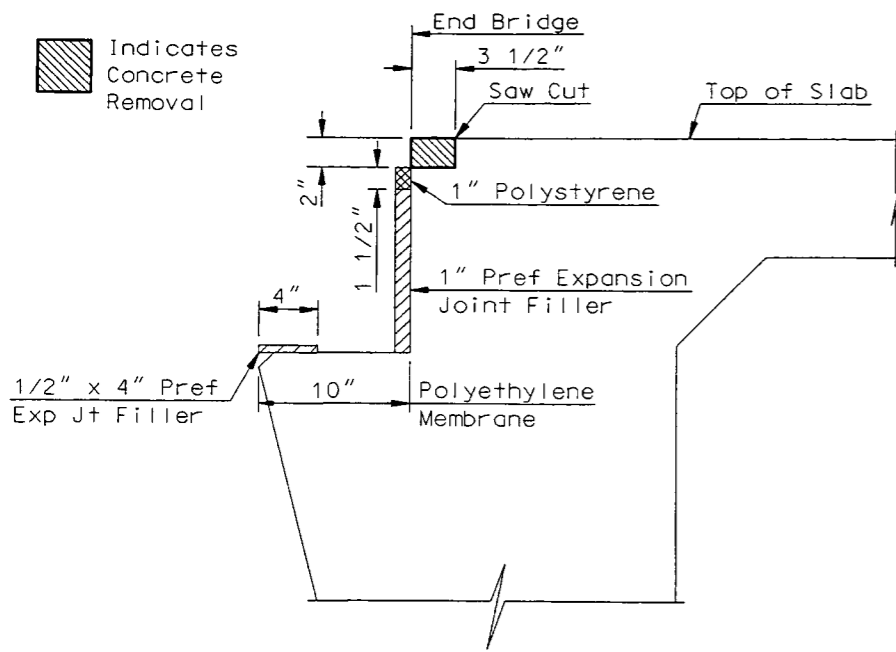


JOINT DETAIL A

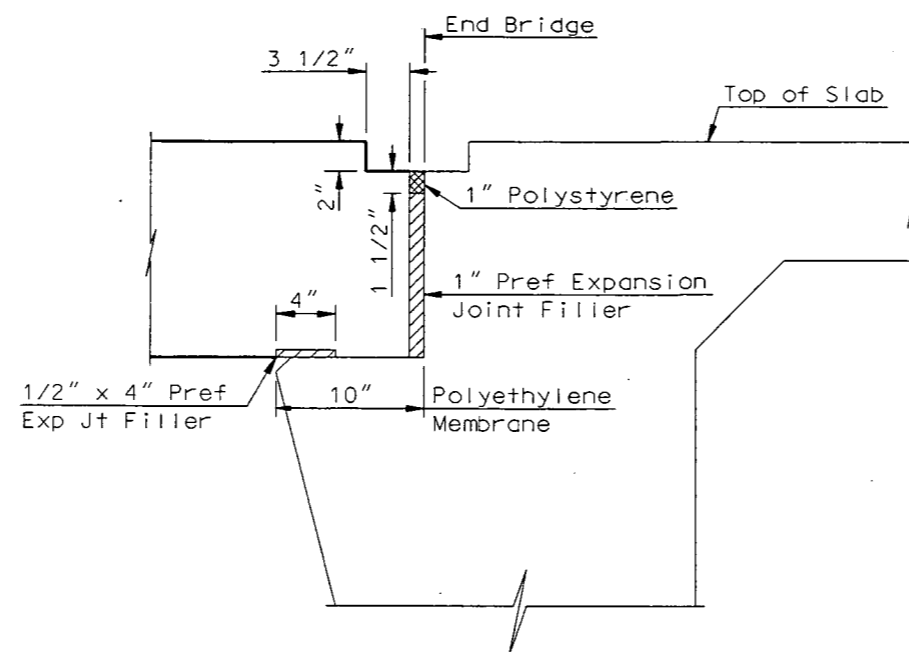
QUANTITIES (ONE APPROACH)	
NOSING CONCRETE	9.0 CF
SILICONE SEALANT	37.0 LF

GOOSE RIVER
(AT BEGIN BRIDGE)
APPROACH SLAB JOINT DETAILS
& NOTES

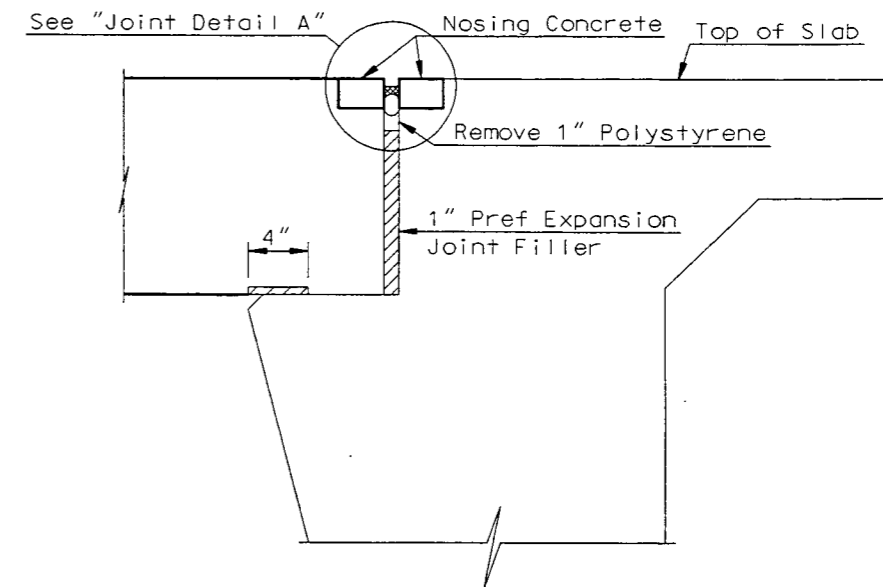
29-104.576R-5



STAGE 1



STAGE 2



STAGE 3

NOTES:

STAGE 1:

1. Remove concrete at ends of deck to allow for nosing concrete.
2. Place the 1" preformed expansion joint filler, the 1/2" x 4" preformed expansion joint filler, the 1" polystyrene and the polyethylene membrane.

STAGE 2:

3. Place the new approach slab concrete. A 2" x 3 1/2" blockout shall be formed between the curbs in the approach slab as shown.

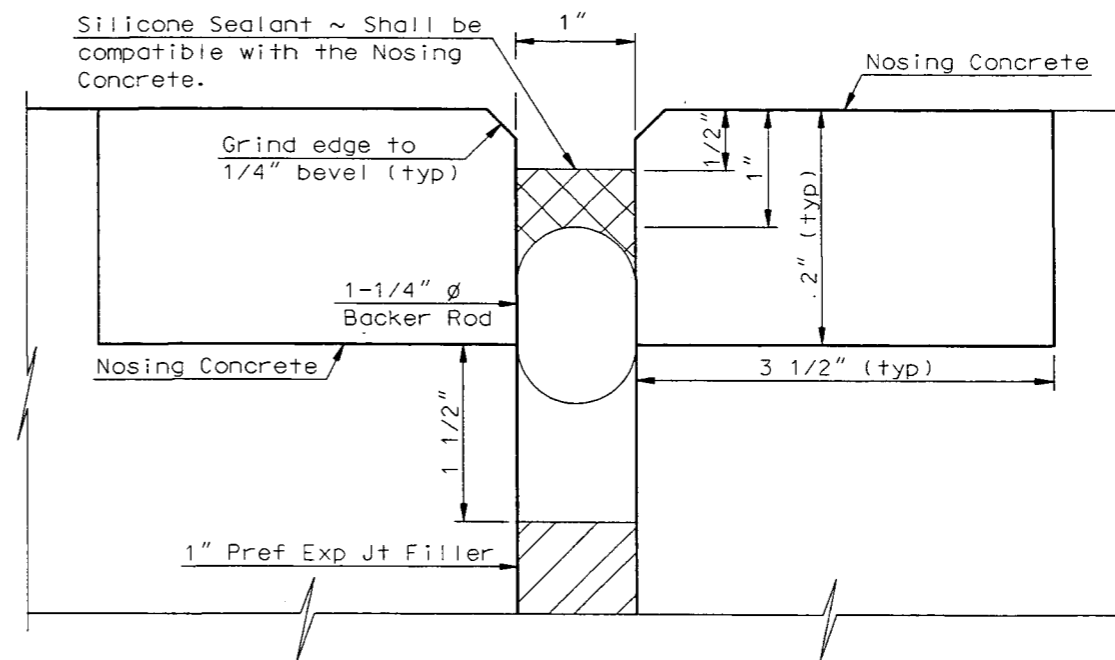
STAGE 3:

4. Remove the 1" polystyrene.
5. Place nosing concrete in the blockout areas, both in the deck and in the approach slab.
6. After the nosing concrete has cured, grind the 1/4" beveled edge. Clean and prepare the joint, apply any necessary bonding material. Install the backer rod and the silicone sealant.

All estimated material quantities shown on drawing number 29-104.576R-4 are for information purposes only. All materials including concrete, reinforcing bars, dowel bars, polyethylene membrane, preformed joint filler and labor required to build the approach slabs and approach slab barriers shall be included in the pay item, "Bridge Approach Slab - Remove & Replace".

The concrete shall be Class AE-3 and the reinforcing steel shall be Grade 60. The polyethylene membrane shall meet the requirements of AASHTO M171.

Surface Finish "D" shall be required for all surfaces of the curb transitions.

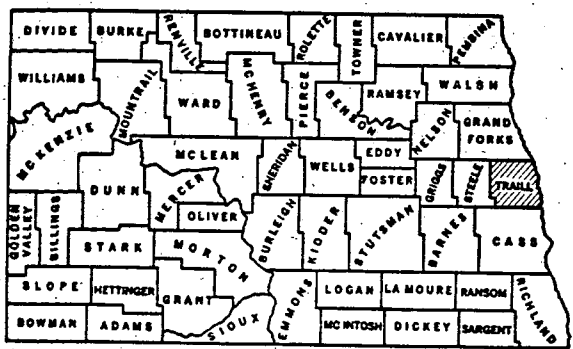


JOINT DETAIL A

QUANTITIES (ONE APPROACH)	
NOSING CONCRETE	3.6 CF
SILICONE SEALANT	37.0 LF

GOOSE RIVER
(AT END BRIDGE)
APPROACH SLAB JOINT DETAILS
& NOTES

29-104.576R-6



SKETCH-MAP OF NORTH DAKOTA SHOWING COUNTIES

NORTH DAKOTA STATE HIGHWAY DEPARTMENT

PLANS FOR THE PROPOSED IMPROVEMENT OF A STATE HIGHWAY

IN TRAILL COUNTY
FEDERAL AID PROJECT NO. I-29-2(20)99 & F-FG-426(6)
GRADING & STRUCTURAL

INDEX OF DRAWINGS

SHEET NO.	TITLE PAGE
SHEET NO. 1	TITLE PAGE
SHEET NO. 2	TYPICAL SECTIONS & SUMMARY OF QUANTITIES
SHEET NO. 3	CULVERT AND BRIDGE LIST
SHEET NO. 4	T&E INCL. PLAN AND PROFILE DRAWINGS
SHEET NO. 49	T&E INCL. INTERCHANGE LAYOUTS & DETAILS
SHEET NO. 54	T&E INCL. STRUCTURAL DRAWINGS
SHEET NO. 55	T&E INCL. FENCING PLANS
SHEET NO. 75	T&E INCL. COMPUTER TYPEOUTS
SHEET NO. 83	T&E INCL. SOIL PROFILE
SHEET NO. 108	T&E INCL. CROSS SECTIONS

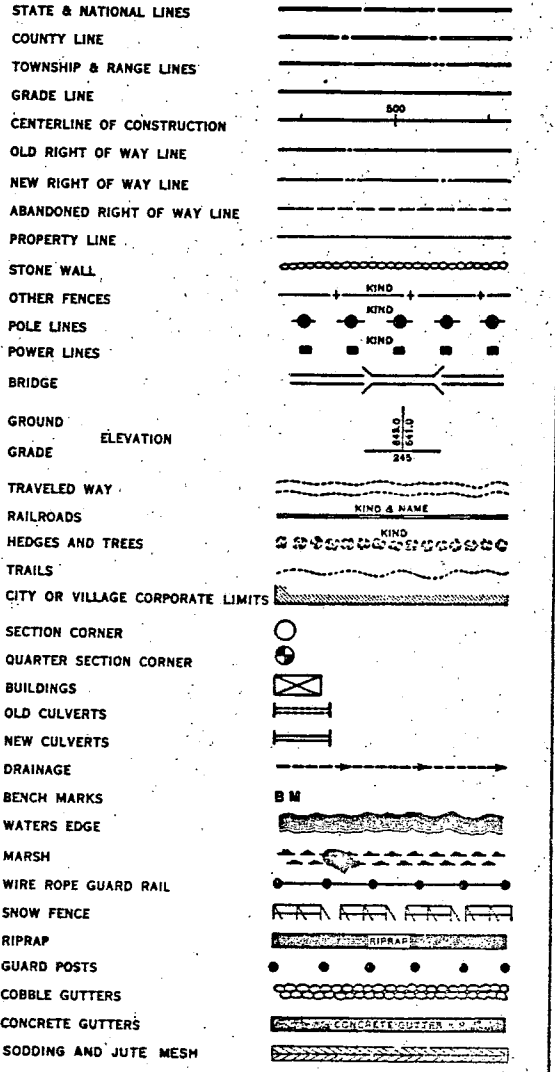
PROJ. F-FG-426(6) SHEET - 1 thru 28, 2E thru 26, 3A, 3I thru 34, 283 thru 305

LENGTH OF PROJECT	
PROJECT MILES-GROSS	MILES-NET
I-29-2(20) 11.405	11.405
F-FG-426(6) 0.198	0.198
TOTALS	11.603

FED. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
5	N. D.	I-29-2(20)99	1	319

GOVERNING SPECIFICATIONS:
Standard Specifications adopted by the North Dakota State Highway Department Jan. 1965 and approved as standard by the Bureau of Public Roads June 23, 1965. Required Contract Provisions (Form PR-1273) dated October 1966 and others submitted herewith.

KEY TO CONVENTIONAL SIGNS



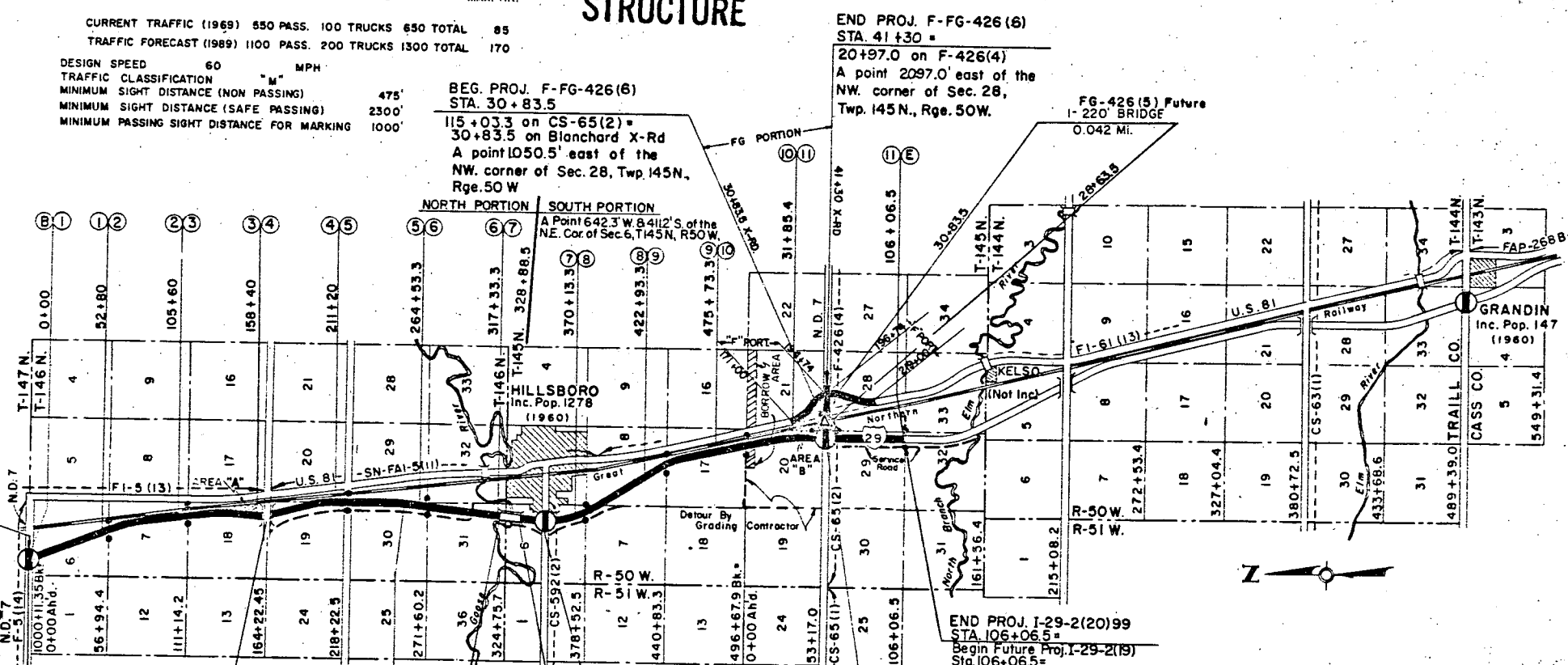
DESIGN DATA I-29-2(20)99

TRAFFIC	AVERAGE DAILY	EST. 30TH MAX. HR.
CURRENT TRAFFIC (1969)	2500 PASS. 400 TRUCKS 2700 TOTAL	340
TRAFFIC FORECAST (1989)	5050 PASS. 890 TRUCKS 5940 TOTAL	750
DESIGN SPEED	70 MPH	
TRAFFIC CLASSIFICATION	"M"	
MINIMUM SIGHT DISTANCE (NON PASSING)	600'	
FULL CONTROL OF ACCESS.		
NO POINT OF ACCESS OTHER THAN BY RAMPS AT INTERCHANGES.		

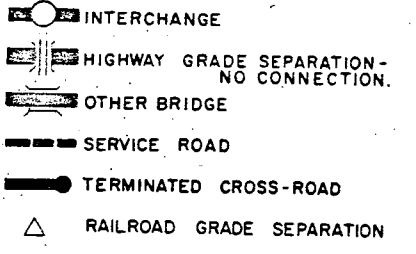
DESIGN DATA F-FG-426(6)

TRAFFIC	AVERAGE DAILY	EST. 30TH MAX. HR.
CURRENT TRAFFIC (1969)	550 PASS. 100 TRUCKS 650 TOTAL	85
TRAFFIC FORECAST (1989)	1100 PASS. 200 TRUCKS 1300 TOTAL	170
DESIGN SPEED	60 MPH	
TRAFFIC CLASSIFICATION	"M"	
MINIMUM SIGHT DISTANCE (NON PASSING)	475'	
MINIMUM SIGHT DISTANCE (SAFE PASSING)	2300'	
MINIMUM PASSING SIGHT DISTANCE FOR MARKING	1000'	

STRUCTURE



LEGEND



BEGIN PROJ. I-29-2(20)99
STA. 0+00 =
End Proj. I-29-2(16)
Sta. 1000+11.35 =
Sta. 565+00 on Proj. F-5(14) =
A point 1491.0' East of the NW
corner of Sec. 6, Twp. 146N., Rge. 50W.

EQUATION
1000+11.35 Bk. =
0+00 Ahd.

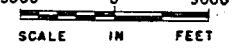
EQUATION
253+03.15 Bk. =
253+56.45 Ahd.

EQUATION
496+67.9 Bk. =
0+00 Ahd.

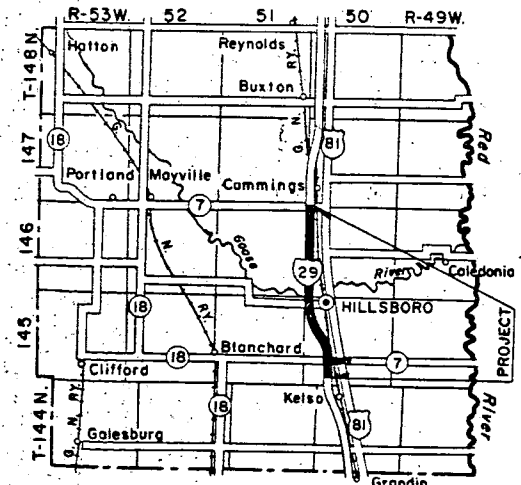
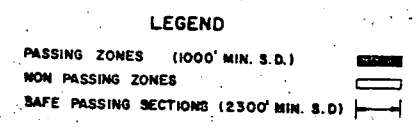
STA. CLEAR ROADWAY WIDTH DESIGN LOADING
328+88.5 37' HS 20 (1944) *

* AND ALTERNATE LOADING AS DESIGNATED IN
PPM 20-4, SECTION 4 C

LAYOUT MAP



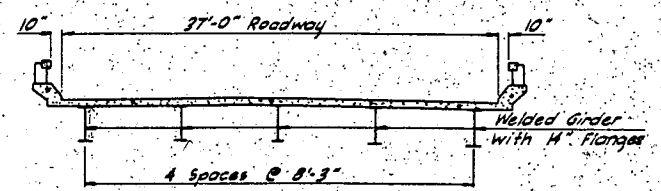
SIGHT DISTANCE DIAGRAM F-FG-426(6)



APPROVED DATE 7-10-69
R. Whidley
CHIEF ENGINEER
NORTH DAKOTA STATE
HIGHWAY DEPARTMENT

U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION
BUREAU OF PUBLIC ROADS
APPROVED
DIVISION ENGINEER DATE

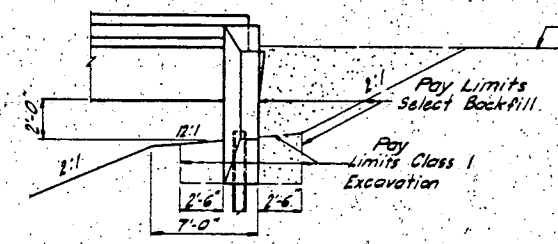
BRIDGE CODE	FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
X-071	5	N. D.	1-29-2(20)99	58	319	



DECK SECTION

1969
FEDERAL AID
PROJECT
I-29-2(20)99
NORTH DAKOTA
29-59

FEDERAL AID NAME PLATE
2 REQUIRED



DETAIL AT ABUTMENTS

The general notes for this project are shown on Drawing 29-59-1.

- ① See SP-33 for Painting.
- ② See SP-35 for Structural Steel.
- ③ See SS-21 for Piling.
- ④ See SP-27 for Backfill.
- ⑤ See SP-32 for Reinforcement.
- ⑥ See SP-34 for Quick Setting Anchor Grout.
- ⑦ See SP-29 for Optional Concrete Surface Finish.

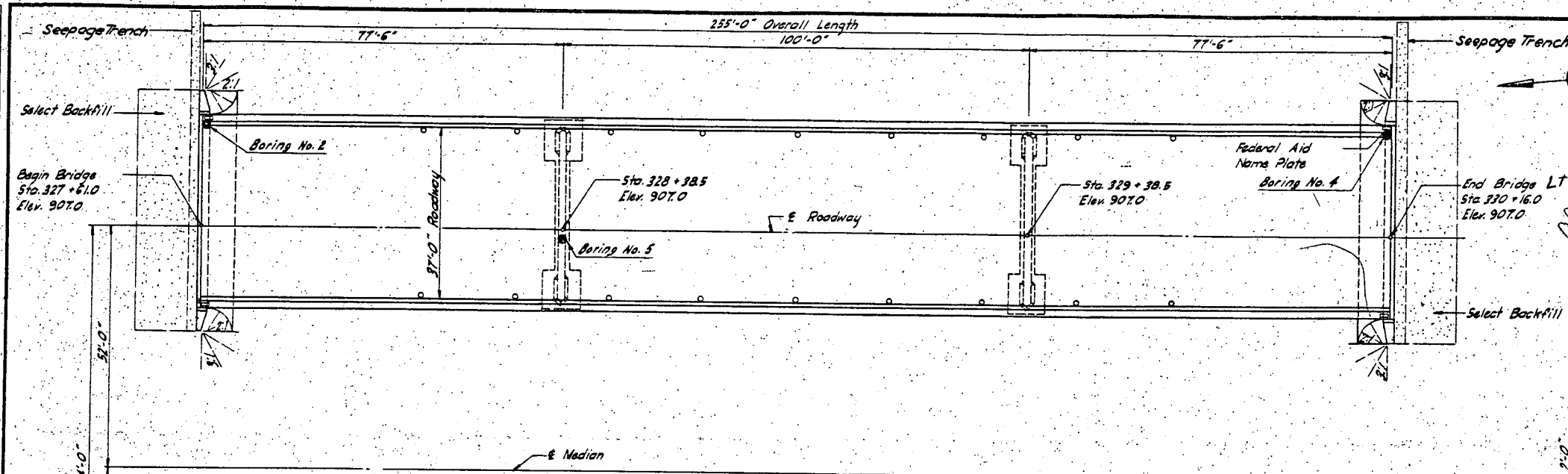
ESTIMATE OF QUANTITIES			(TWO BRIDGES)
SPEC. NO.	CODE NO.	BID ITEM	QUANTITY
203	0100	COMMON EXCAVATION	4,212.5 CU. YD.
208	0100	CLASS 1 EXCAVATION	165 CU. YD.
208	0100	CLASS 2 EXCAVATION	630 CU. YD.
228	0100	SELECT BACKFILL	360 CU. YD.
216	0100	WATER	"M" GAL.
610	1112	CLASS AE-1 CONCRETE (SUB-STRUCTURE)	381.6 CU. YD.
610	1134	CLASS AE-3 CONCRETE (I-BEAM SUPERSTR.)	603.6 CU. YD.
610	0138	CLASS AAE-3 CONCRETE (RAILING)	33.08 CU. YD.
612	0110	REINFORCING STEEL (GRADE 40)	242,064 LB.
616	4412	STRUCTURAL STEEL (A-441) (WELDED BEAM)	353,520 LB.
616	0362	STRUCTURAL STEEL (A-36) (WELDED BEAM)	66,530 LB.
622	0020	STEEL PILING (10 BP 42)	1475 LIN. FT.
622	0040	STEEL PILING (12 BP 53)	2090 LIN. FT.
622	0540	STEEL TEST PILES (10 BP 42) @ 110 FT.	2 EACH
622	1380	STEEL TEST PILES (12 BP 53) @ 85 FT.	2 EACH
750	0100	LINSEED OIL TREATMENT	97 GAL.
3000		BRIDGE BENCH MARKS	1 SET.

STRUCTURAL DRAWINGS

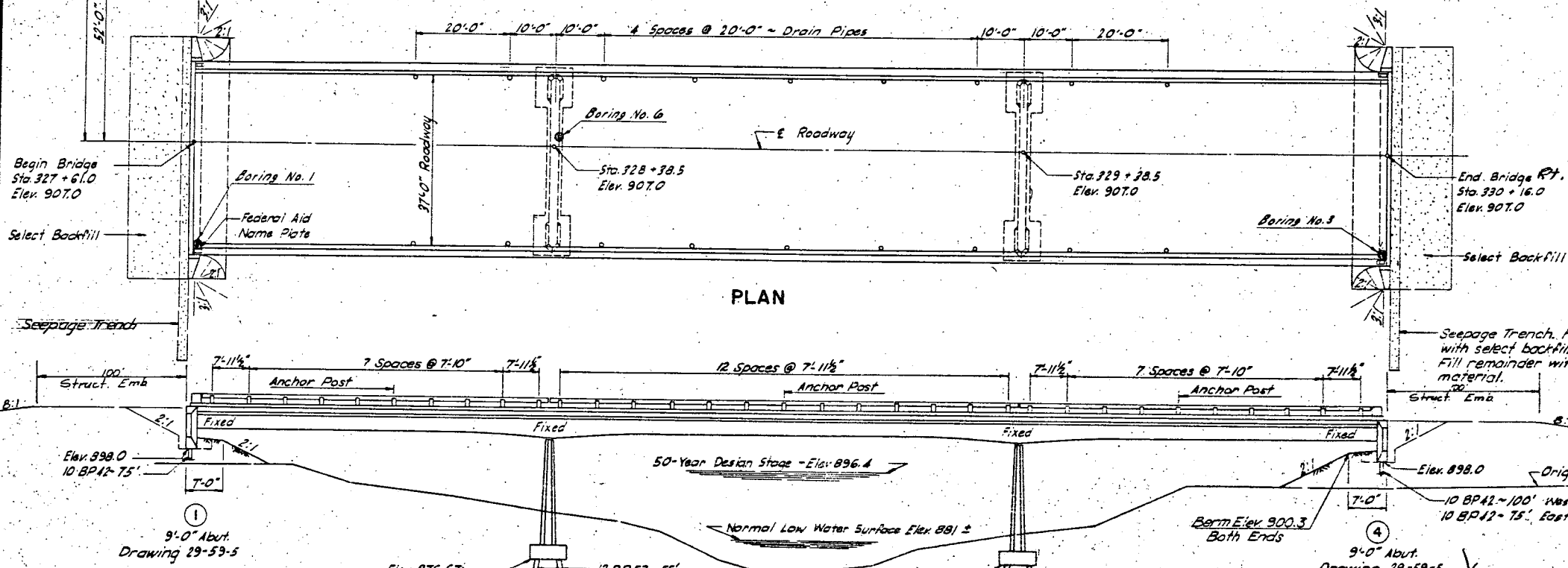
GENERAL DRAWING 29-59 THIS SHEET 29-59-1 THRU 29-59-4
SUBSTRUCTURE 29-59-5, 29-59-6, 29-59-7, H-0401
SUPERSTRUCTURE 29-59-8 THRU 29-59-12, H-0153, STD. 7.6, STD. 14.98, H-0501

DESIGN LOADING: H-20 (15.44) SCALE: 1 INCH = 15 FEET

NORTH DAKOTA
STATE HIGHWAY DEPARTMENT
GOOSE RIVER BRIDGE
BRIDGE LAYOUT
PROJECT I-29-2(20)99 STA. 328 + 88.5
TRAILL COUNTY
APPROVED: *Joseph R. Kiley*
DATE: 12-28-67
REGISTERED PROFESSIONAL ENGINEER
NORTH DAKOTA



PLAN



ELEVATION

DESIGN STRESSES:
 f_s = 20,000 psi - Reinforcing Steel
 f_c = 1,200 psi - Class AE-1 / AE-3 Concrete
 f_c = 1,700 psi - Class AAE-3 Concrete
 f_s = 20,000 psi - Structural Steel A-36
 f_s = 27,000 psi - Structural Steel A-441 (3/4" and under)
 f_s = 25,000 psi - Structural Steel A-441 (3/4" to 1 1/2")
 f_s = 23,000 psi - Structural Steel A-441 (1 1/2" to 4")

BENCH MARKS		
NO.	DESCRIPTION	ELEV.
35	Railroad Spike in Tree 328+04 - 212' Rt.	897.18
36	Railroad Spike in Tree 330+75 - 195' Rt.	895.83
37	Iron Nail by E. Pole 333+00 - 195' Rt.	906.66

PILE LOADING				
LOCATION	DEAD LOAD	LIVE LOAD	EARTH	WIND
Abutments	37.1 T.	12.3 T.		50 LB. 15 LB. 100 LB. LL.
Piers	48.0 T.	14.2 T.	5.1 T.	

ROADWAY	907.00	907.00	907.04	907.08	907.10	907.10	907.08	907.05	907.03	907.01	907.00	907.02	907.06	907.10	907.14	907.14	907.10	907.06	907.03	907.00	907.01	907.03	907.06	907.10	907.10	907.06	907.03	907.00	907.00
Begin Bridge	10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 100'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		10 Eq. Sp. = 77'-0"		
End Bridge	Brg. Abut. 1		Pier 2		Pier 3		Pier 4		Brg. Abut. 4																				

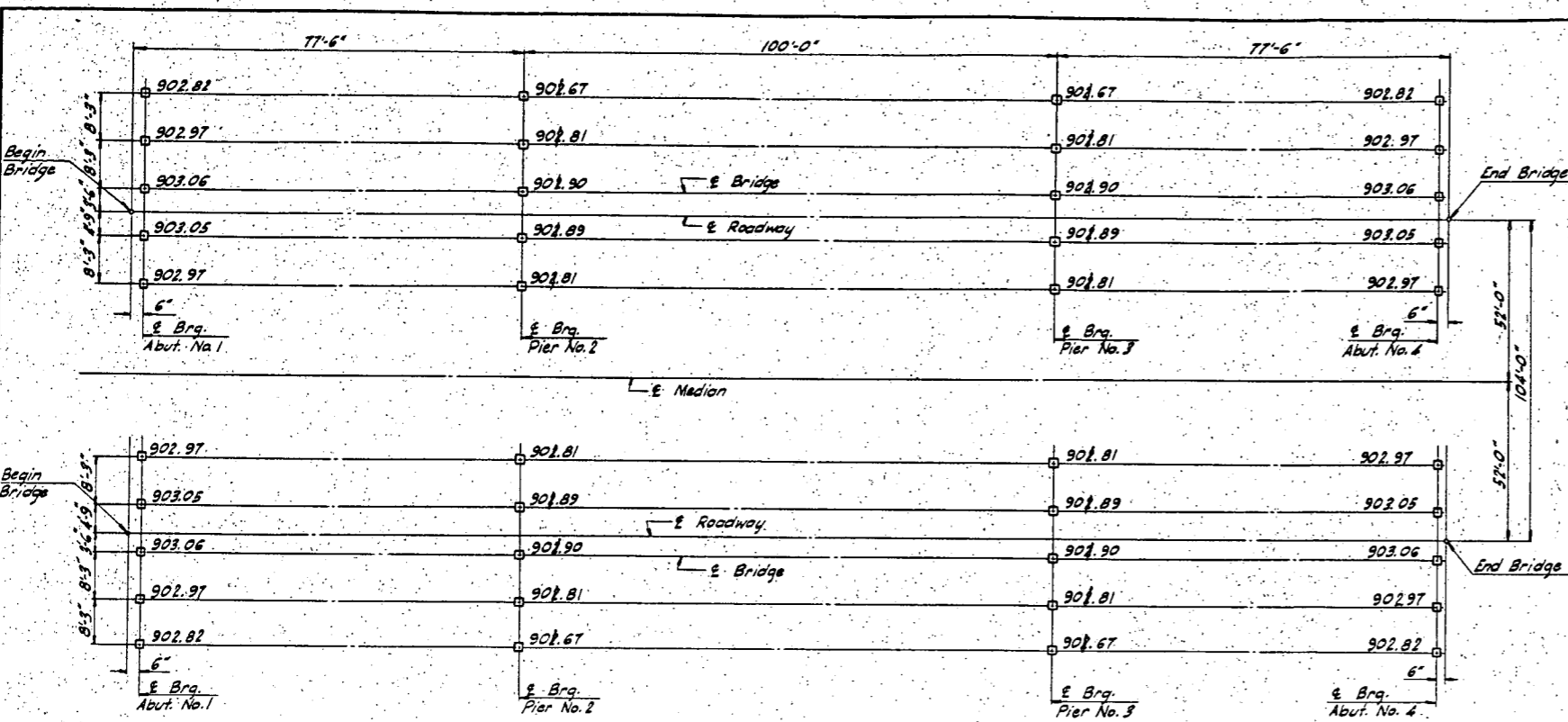
SCREED ELEVATION

Elevations are to top of finished concrete

29-59

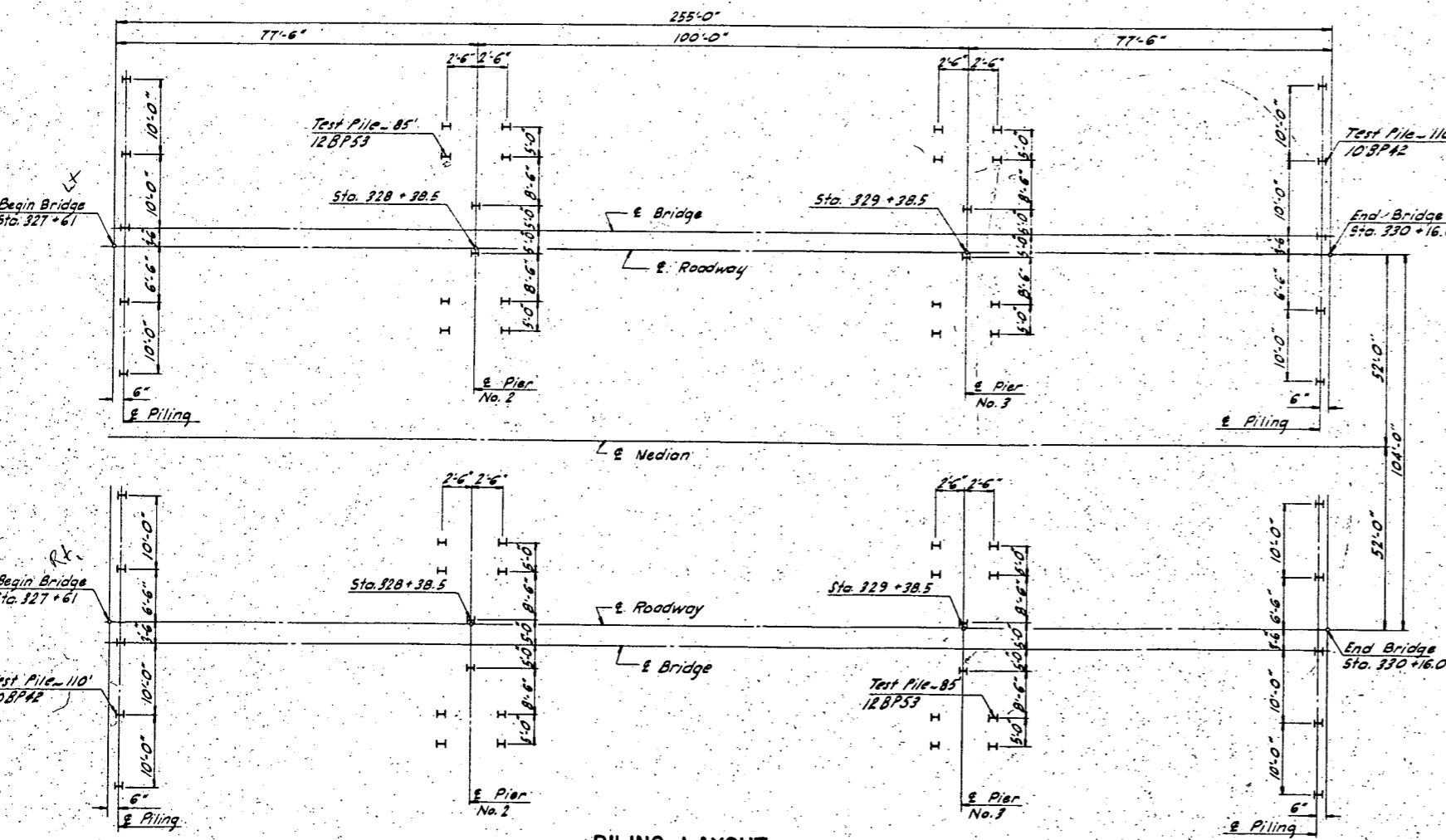
FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
5	N. D.	1-29-2(20)99		55	319

NO.	DATE	BY	REVISIONS



BEARING PLATE LAYOUT
Elevations are to top of finished concrete
(Not to Scale)

D.L.R.
9-23-69



PILING LAYOUT
(Not to Scale)

NOTES:

- GENERAL:**
- THE COST OF FURNISHING AND PLACING JOINT FILLER, ASPHALT CURB SEAL, NAME PLATES, AND OTHER MISCELLANEOUS ITEMS SHALL BE INCLUDED IN THE PRICE BID FOR CONCRETE.
- WELDING WILL NOT BE PAID FOR DIRECTLY BUT SHALL BE INCLUDED IN THE UNIT PRICE BID FOR STRUCTURAL STEEL.
- DEAD LOAD DEFLECTIONS AND VERTICAL CURVE CORRECTIONS HAVE BEEN ACCOUNTED FOR IN THE SHOP CAMBER OF THE GIRDERS.
- BEARING AREAS SHALL BE FINISHED TRUE TO PLAN AND ELEVATION BY GRINDING, IF NECESSARY, BEFORE BEARING PLATES ARE SET.
- ALL RIVETS OR HIGH STRENGTH BOLTS ARE TO BE 7/8". OPEN HOLES ARE TO BE 15/16" EXCEPT AS NOTED. SHOP CONNECTIONS SHALL BE MADE AS SHOWN. FIELD CONNECTIONS SHALL BE MADE WITH HIGH TENSILE STRENGTH BOLTS OR SHALL BE RIVETED.
- IF HIGH STRENGTH BOLTS ARE USED IN THE WEB AND FLANGE SPLICES, THE BOLTS SHALL BE PLACED WITH THE NUTS ON THE INTERIOR SIDE OF THE WEB AND ON THE UPPER SIDE OF THE FLANGE.
- BACKFILL AND EMBANKMENT:**
- ALL EMBANKMENT AND BACKFILL SHALL BE PLACED ACCORDING TO SECTION 203-2.3 OF THE STANDARD SPECIFICATIONS EXCEPT THAT MAXIMUM DRY DENSITY AS DETERMINED BY AASHTO DESIGNATION T-99 SHALL BE 95%. THE MOISTURE CONTENT IN THE SOIL AT THE TIME OF COMPACTION SHALL NOT BE LESS THAN FOUR PERCENTAGE POINTS BELOW OPTIMUM NOR MORE THAN THAT WHICH WILL PERMIT COMPACTION TO THE REQUIRED DENSITY. SELECT BACKFILL SHALL NOT BE PLACED ABOVE THE BERM ELEVATIONS UNTIL THE ENTIRE SUPERSTRUCTURE SLAB HAS CURED.
- THE EMBANKMENT AT THE ABUTMENTS SHALL BE IN PLACE BEFORE ABUTMENT PILING ARE DRIVEN.
- THE CONTRACTOR WILL BE REQUIRED TO REDRILL TO ELEV. 885.0 AT THE ABUTMENTS BEFORE DRIVING PILING. ALL PILOT HOLES, NOT COMPLETELY FILLED BY THE PILES, SHALL BE BACKFILLED WITH SAND OR FINE GRAVEL BEFORE THE SUB-STRUCTURE IS PLACED.
- EXCAVATION:**
- EXCAVATION CLASS 1 AT THE ABUTMENTS SHALL EXTEND FROM THE BOTTOM OF THE FOOTING TO THE UPPER LIMITS AS SHOWN ON THE BRIDGE LAYOUT DRAWING. EXCAVATION CLASS 2 AT THE PIERS SHALL EXTEND FROM THE BOTTOM OF THE FOOTING TO THE BOTTOM OF THE CHANNEL.
- REINFORCING STEEL:**
- DIMENSIONS FOR BENT BARS ARE GIVEN CENTER TO CENTER AND TO TANGENT INTERSECTIONS UNLESS OTHERWISE NOTED. BENT BARS SHALL BE BENT AROUND CRS1 STANDARD SIZE PINS. THE BAR FABRICATOR SHALL ADD A PREFIX TO ALL BAR DESIGNATIONS TO DIFFERENTIATE BETWEEN THE SEVERAL PARTS OF THE STRUCTURE OR STRUCTURES.
- CONCRETE:**
- ALL EXPOSED EDGES OF CONCRETE SHALL BE BEVELED WITH 3/4" TRIANGULAR MOLDING UNLESS OTHERWISE NOTED. THE DECK SLAB CONCRETE SHALL BE STRUCK OFF AND COMPACTED BY AN APPROVED DECK FINISHING MACHINE. ALL CONCRETE ABOVE THE TOP OF THE CURBS SHALL BE CLASS AAE-3. THE REMAINING SUPERSTRUCTURE CONCRETE SHALL BE CLASS AE-3 AND THE SUB-STRUCTURE CONCRETE AE-1.
- ALL CONCRETE SHALL BE COMPACTED BY VIBRATION. THE "RUBBED SURFACE FINISH" WILL BE REQUIRED FOR THE ROADWAY AND OUTSIDE VERTICAL FACES OF CURBS, EDGES OF SLAB, ALL FACES OF THE RAILS, RAIL POSTS AND END POSTS, ALL EXPOSED FACES OF BENTS, AND ALL EXPOSED FACES OF ABUTMENT WINGS. ALL OTHER SURFACES SHALL BE GIVEN THE "ORDINARY SURFACE FINISH". (SEE OPTION BELOW)
- IF THE EXPOSED FACES OF THE ABUTMENTS AND BENTS HAVE A SURFACE FINISH ACCEPTABLE TO THE ENGINEER WITHOUT RUBBING, THE REQUIREMENT FOR "RUBBED SURFACE FINISH" MAY BE WAIVED AT THE OPTION OF THE ENGINEER, AND THE "ORDINARY SURFACE FINISH" WILL APPLY.
- ALL "ORDINARY SURFACE FINISH" SHALL BE COMPLETED WITHIN 24 HOURS AFTER REMOVAL OF FORMS.
- WORK SHALL CONFORM TO ALL APPLICABLE PARAGRAPHS OF THE NORTH DAKOTA STATE HIGHWAY DEPARTMENT SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.
- THE CONTRACTOR HAS THE OPTION OF USING THE SPECIAL SURFACE FINISH AS PROVIDED IN SPECIAL PROVISION NO. 29, CONCRETE STRUCTURES, IN LIEU OF THE RUBBED SURFACE FINISH (602-3.10.3) CALLED FOR ABOVE.
- STRUCTURAL STEEL:**
- THE NORTH DAKOTA STANDARD SPECIFICATIONS 616-3.3.12, FLAME CUTTING, SHALL GOVERN THE FLAME CUTTING PREPARATION OF A36 AND A441 STEELS. IN ADDITION, PREHEATING OF THE A36 AND A441 STEEL SHALL BE REQUIRED. JUST PRIOR TO FLAME CUTTING, THE STEEL SHALL BE PREHEATED TO A TEMPERATURE CONFORMING TO TABLE 2, PAGE 3 OF SPECIAL PROVISION SP 35 GOVERNING THE PREHEATING OF STEEL THICKNESSES WHEN LOW HYDROGEN ELECTRODES ARE USED IN WELDING.
- THE BEND TEST, UNDER 616-3.3.12.2, SHALL BE REQUIRED FOR THE THICKEST A36 AND A441 STEEL USED IN THE PROJECT. AS STATED IN

PARAGRAPHS 616-3.3.12.3, "IN LIEU OF PREHEATING, THE FLAME CUT EDGES SHALL BE REMOVED TO A DEPTH OF AT LEAST 1/8" BY MILLING OR GRINDING OR IN THE CASE OF MACHINE FLAME CUTTINGS, THE EDGES MAY BE FLAME SOFTENED AFTER CUTTING". IN ACCORDANCE WITH SP 35, PARAGRAPH 406(G) THE TEST PLATE SIZE SHALL BE 3/4"x14".

THE GIRDERS SHALL BE CAMBERED IN THE SHOP AS DETAILED ON DRAWING 29-59-3. SUFFICIENT PARTS OF THE STRUCTURE SHALL BE SHOP ASSEMBLED AND ADJUSTED TO LINE, GRADE AND CAMBER OR OTHER PROPER FIT UP. IN ORDER TO ESTABLISH THE ACCURACY OF THE WORKMANSHIP AND SHOP DRAWINGS, AFTER SUB-PUNCHING OR SUBDRILLING, EACH FULL LENGTH GIRDER LINE WITH ITS FIELD SPLICE PLATES IN PLACE SHALL BE SHOP ASSEMBLED TO THE CORRECT LINE AND GRADE BEFORE REAMING. TEMPLATES SHALL NOT BE USED IN LIEU OF COMPLETE ASSEMBLY.

ALTERNATE STUD SHEAR DEVICES SHALL BE MANUFACTURED OF C-1015 OR C-1020 COLD ROLLED STEEL WHICH CONFORMS TO ASTM A 108-61T SPECIFICATIONS, SHALL CONFORM TO THE DIAMETER AND OTHER DIMENSIONS AS SHOWN. THE CONTRACTOR IS REQUIRED TO OBTAIN A PRODUCT CERTIFICATION OF THE STUD SHEAR DEVICES USED ON THIS BRIDGE IN ACCORDANCE WITH SECTION 802 OF THE 1965 NORTH DAKOTA "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION". ALTERNATE SHEAR DEVICES WILL BE PAID FOR AS CHANNEL SHEAR DEVICES. AASHTO REQUIREMENTS FOR WELDABILITY OF STUD CONNECTORS SHALL BE MET AND CERTIFIED.

TEST PILES:

THE PILES FOR THIS STRUCTURE SHALL BE DRIVEN BY A STEAM (OR AIR) OR DIESEL HAMMER HAVING A RATED ENERGY AND RAM WEIGHT NOT LESS THAN 25,400 FOOT-POUND-TONS AS COMPUTED BY THE FORMULA $W(E-8,820)+0.65$ WHERE W IS THE WEIGHT OF THE RAM IN TONS AND E IS THE RATED HAMMER ENERGY AS ALLOWED IN THE SPECIAL PROVISIONS/SUPPLEMENTAL SPECIFICATIONS. IN NO CASE SHALL THE RAM WEIGHT BE LESS THAN 4,000 POUNDS.

THE TEST PILES FOR THIS STRUCTURE SHALL BE DRIVEN TO A BEARING OF NOT LESS THAN 125% DESIGN LOAD AS DETERMINED BY THE DYNAMIC FORMULA.

MATERIALS: (STRUCTURAL STEEL)

GIRDER FLANGES, WEBS, FLANGE AND WEB SPLICE PLATES: A-441.

STIFFENERS, DIAPHRAGMS, SHEAR CONNECTORS, BEARING PLATES AND ANCHOR BOLTS: A-36, EXCEPT ALTERNATE STUD SHEAR DEVICES.

PAINT:

PAINT AND PAINTING SHALL CONFORM TO THE NORTH DAKOTA STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION; SECTIONS 710, AND 670-1.2, AND THE SPECIAL PROVISION ON PAINTING.

ALL EXPOSED STEEL SURFACES SHALL BE GIVEN ONE SHOP COAT OF RED LEAD PAINT (INCLUDING TOP OF UPPER GIRDER FLANGES BUT NOT SHEAR CONNECTORS), ONE SPOT COAT OF RED LEAD PAINT AFTER ERECTION AND CONCRETE WORK IS COMPLETED, AND TWO FIELD COATS OF ENAMEL. THE FIRST FIELD COAT SHALL CONFORM TO GREY-GREEN COLOR NO. 24260 AND THE SECOND COAT SHALL CONFORM TO GREY-GREEN COLOR NO. 24300. BOTH COATS SHALL MEET THE FEDERAL STANDARD NO. 595 FOR COLOR. COLOR CHIPS ARE ON FILE IN THE BRIDGE DIVISION OF THE NORTH DAKOTA STATE HIGHWAY DEPARTMENT, BISMARCK.

COFFERDAMS:

SHEET PILE COFFERDAMS SHALL BE DRIVEN TO FACILITATE THE CONSTRUCTION OF EACH PIER. AFTER COMPLETION OF THE PIER AND BEFORE THE SHEET PILES ARE PULLED, WATER SHALL BE PUMPED OUT AND THE BACKFILL PLACED AND COMPACTED IN THE DRY OR THE ELEVATION OF THE ORIGINAL GROUND. THE COSTS SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CLASS 2 EXCAVATION.

THE CONTRACTOR'S ATTENTION IS DIRECTED TO THE BORING LOGS WHICH SHOW A FAIRLY HARD STRATA ABOVE ELEVATION 820 AND BELOW ELEVATION 805 WITH A SOFTER STRATA BETWEEN, EXCEPT AT THE SOUTHWEST ABUTMENT. THE LENGTHS OF THE TEST PILES HAVE BEEN SELECTED SO AS TO DRIVE THROUGH THE UPPER STRATA AND SEAT IN THE LOWER HARD STRATA IF POSSIBLE. THE LENGTHS OF THE REGULAR PILING SHOWN ON THE PLANS HAVE BEEN SELECTED TO SEAT IN THE UPPER HARD STRATA. THE TEST PILE RESULTS WILL DETERMINE IF THE REGULAR PILING WILL BE SEATED IN THE UPPER OR LOWER STRATA.

LINED OIL TREATMENT:

LINED OIL TREATMENT SHALL NOT BE DONE UNTIL ALL CONCRETE WORK IS COMPLETED AND ASPHALT CURB SEAL HAS BEEN INSTALLED.

BLAST CLEANING:

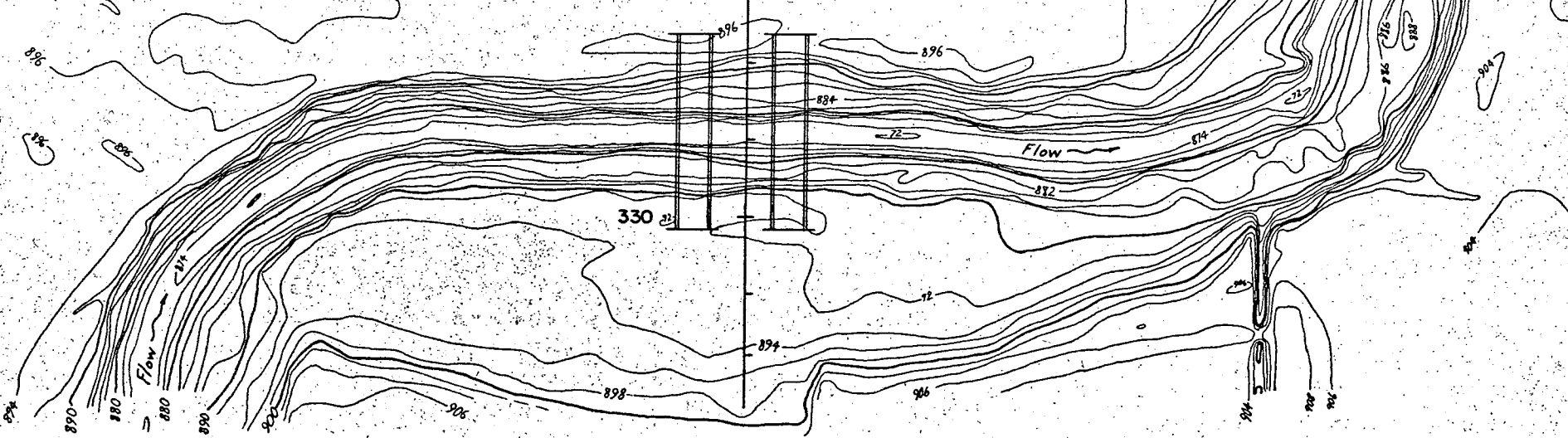
COMMERCIAL BLAST CLEANING OF ALL EXPOSED MAIN AND SECONDARY STEEL MEMBERS WILL BE REQUIRED PRIOR TO PAINTING. SEE S.P. 33. (INCLUDE IN UNIT BID PRICE FOR STRUCTURAL STEEL).

**GOOSE RIVER BRIDGE
BEARING PLATE LAYOUT
PILING LAYOUT
GENERAL NOTES**



HYDRAULIC DESIGN DATA:

DRAINAGE AREA	1090 SQ. MI.
DESIGN FREQUENCY	50 YR.
DESIGN DISCHARGE	6030 CFS
STREAM GRADIENT	0.00028 FT./FT.
DEPTH OF FLOW	23.9 FT.
WATERWAY PROVIDED BELOW DESIGN STAGE	2530 SQ.FT.
WATERWAY PROVIDED BELOW MINIMUM CLEARANCE	3920 SQ.FT.
VELOCITY OF FLOW UNDER BRIDGE	2.38 FPS
FREEBOARD PROVIDED	6.6 FT.
MAXIMUM RECORDED DISCHARGE / STAGE (1950)	9420 CFS / 903.5
MINIMUM WATER ELEVATION	881.0



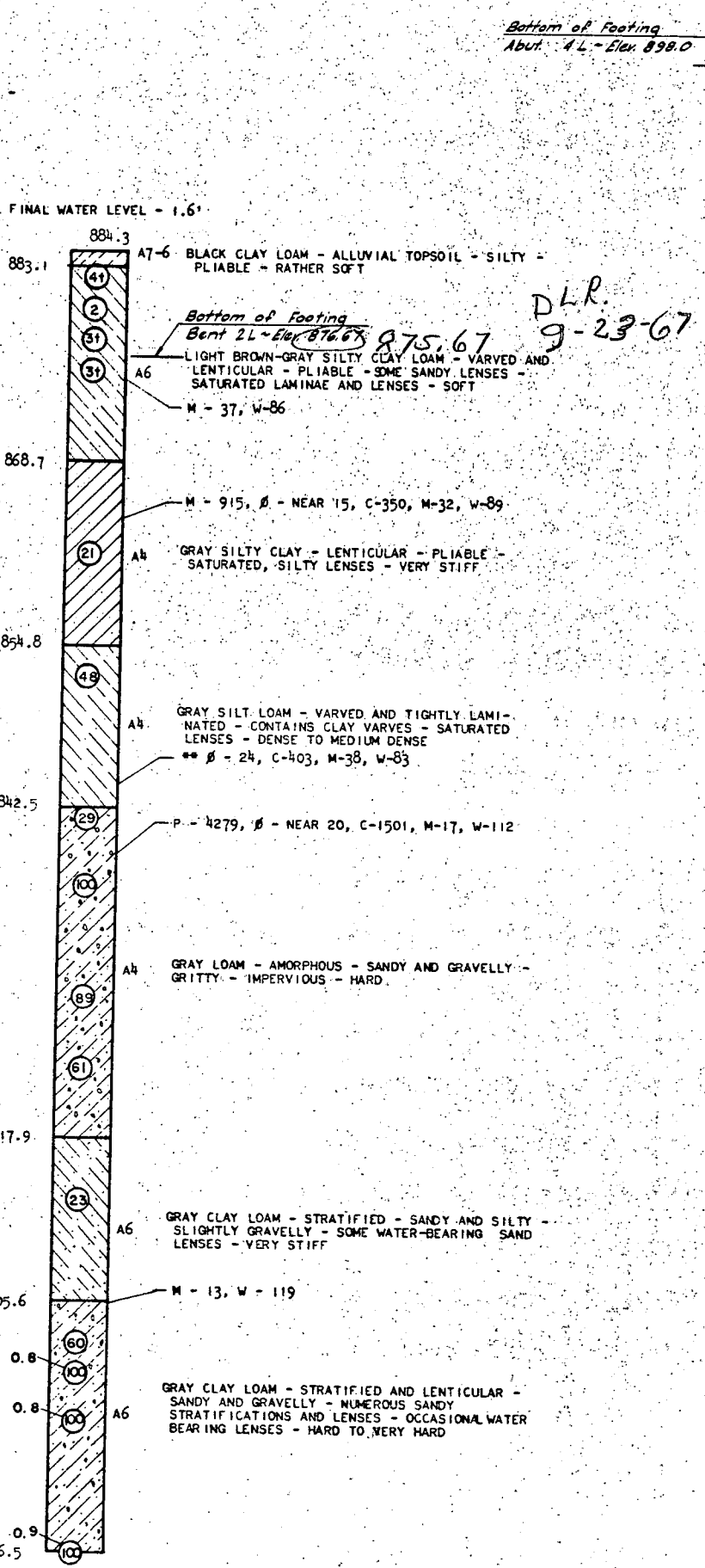
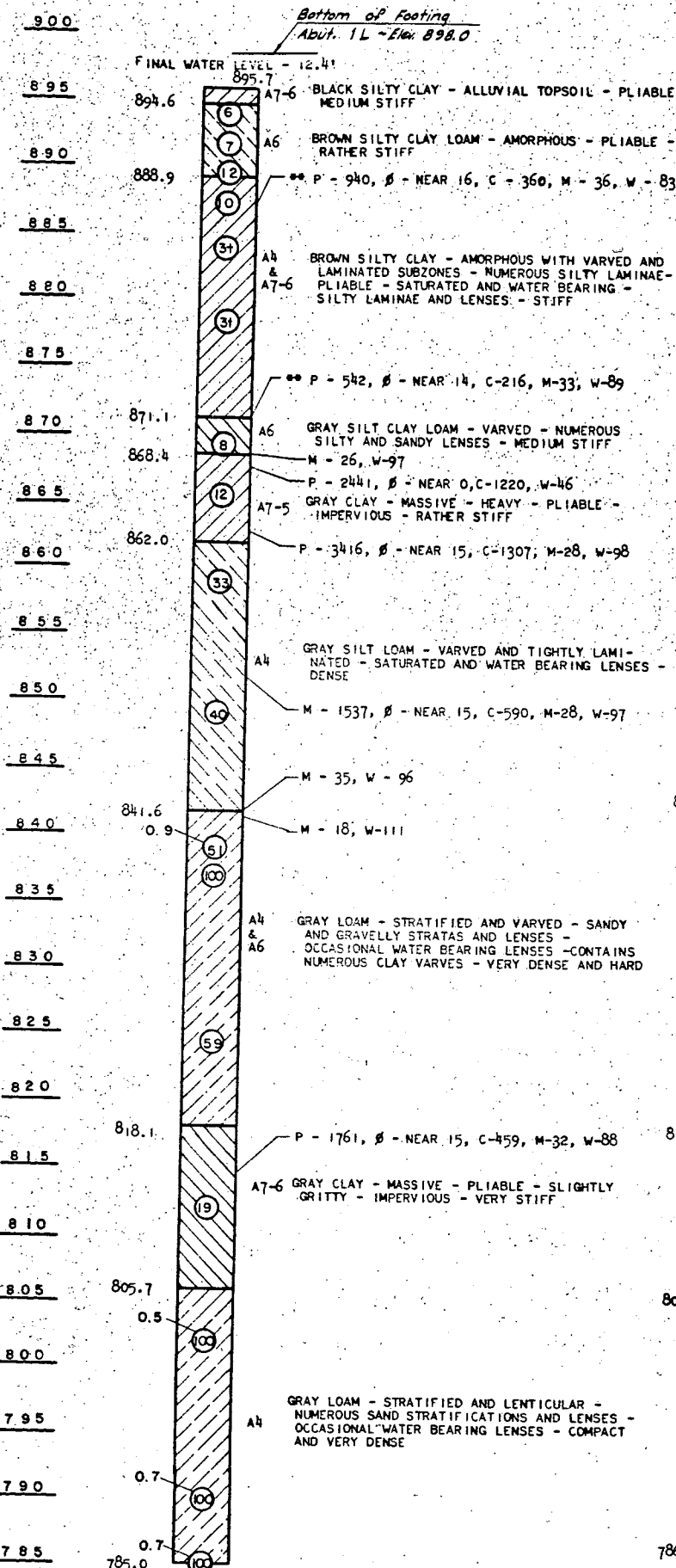
BRIDGE NO 29-59

GOOSE RIVER
TOPOGRAPHIC LAYOUT
PROJ. 1-29-2(20)99 STA. 328 + 88.5
TRAIL COUNTY

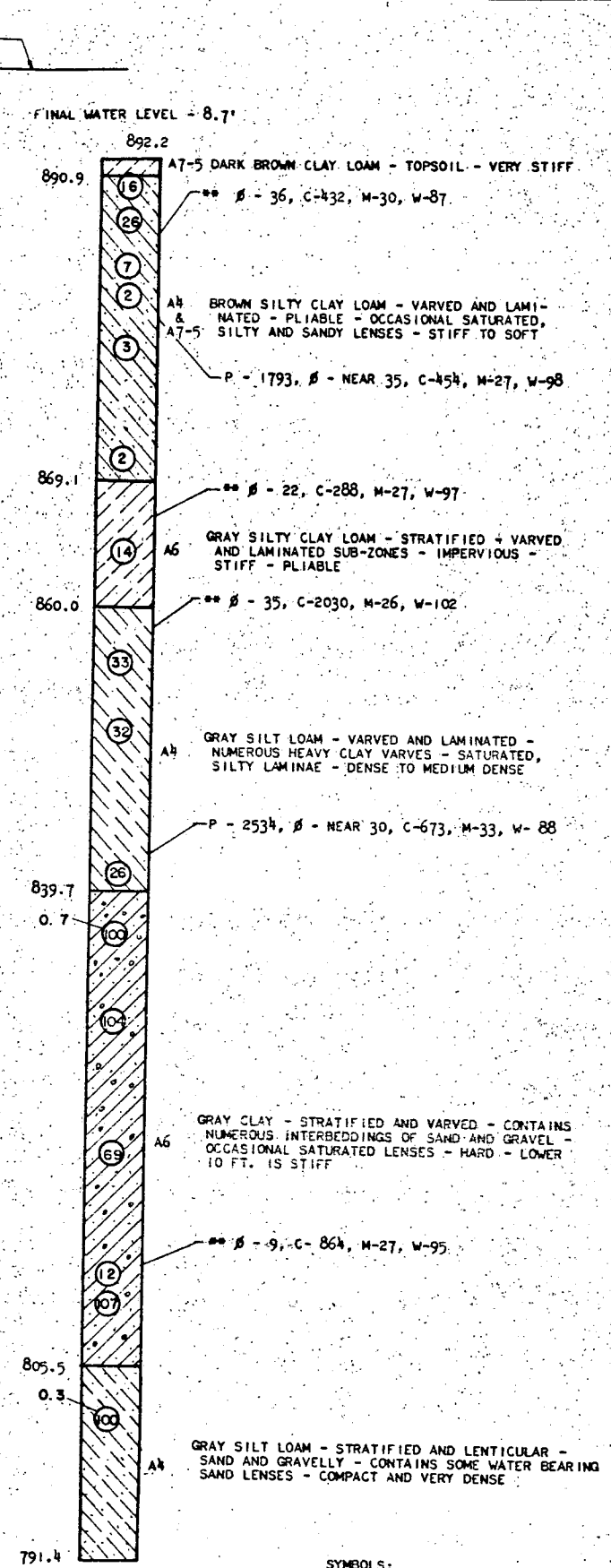
29-59-2

29-59-2

Plotted by Miller - 8/21/67



DLR. 9-23-67



SYMBOLS:

- P - MAXIMUM LOAD (LBS/SQ.FT.)
- β - SHEAR ANGLE (DEGREES)
- C - COHESION (LBS/SQ.FT.)
- M - MOISTURE (PERCENT)
- W - DRY WEIGHT (LBS/CU.FT.)
- ** - TRIAXIAL

NOTES:

ENCIRCLED NUMBERS INDICATE THE NUMBER OF BLOWS DELIVERED BY A 140 LB. HAMMER FROM A HEIGHT OF 30" TO DRIVE CORE TUBE 1.0'.

THE BORING LOG DATA SHOWN IS FOR DESIGN PURPOSE ONLY. THE STATE ASSUMES NO RESPONSIBILITY IF SOIL CONDITIONS ENCOUNTERED DURING CONSTRUCTION DIFFER FROM THOSE SHOWN.

BRIDGE NO. 29-59L

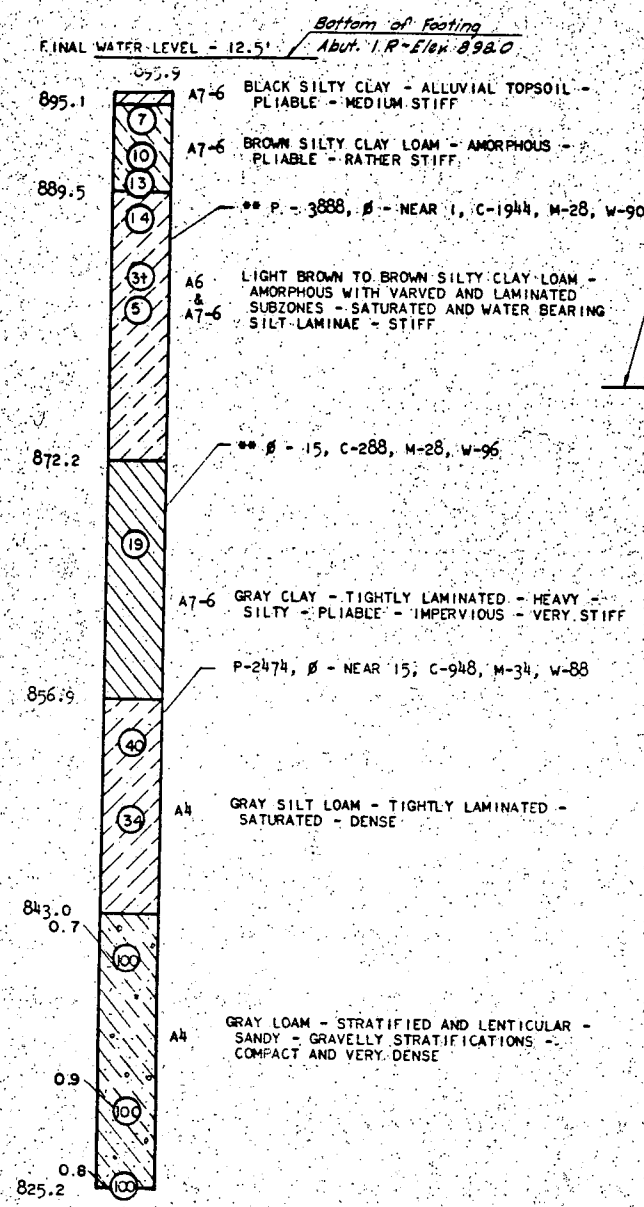
BORING LOG

GOOSE RIVER

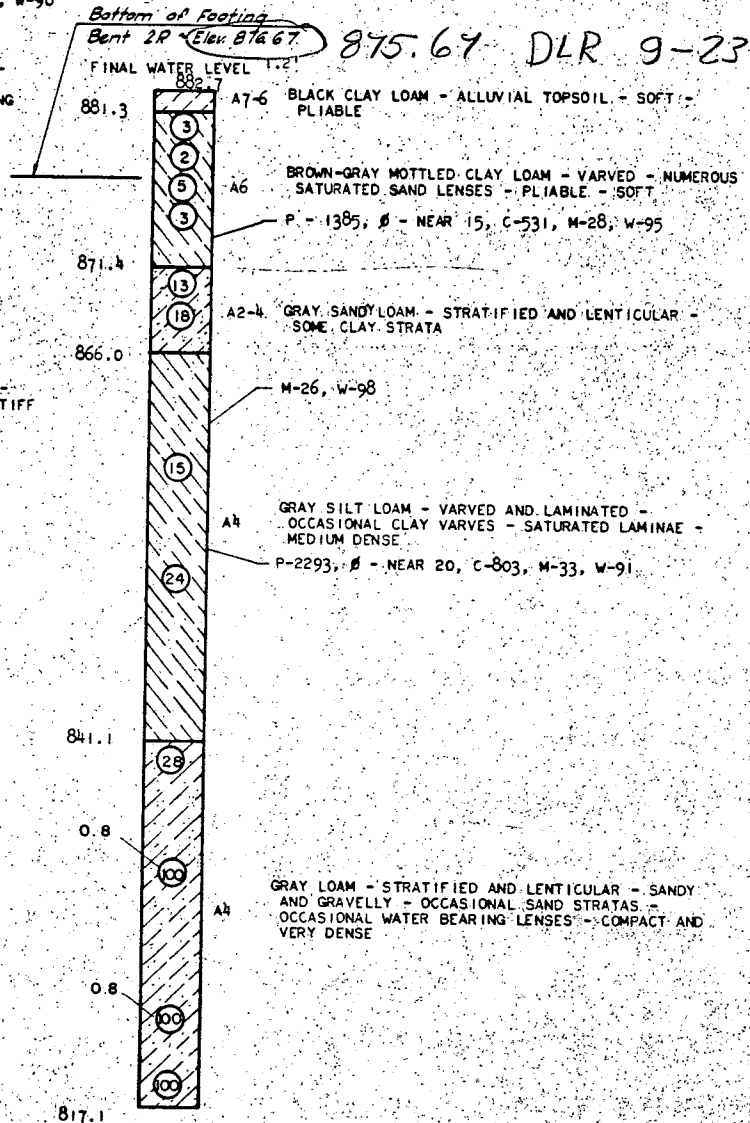
TRAILL COUNTY

Plotted by JLS - Aug. 1967

900
895
890
885
880
875
870
865
860
855
850
845
840
835
830
825
820
815
810
805
800
795
790
785



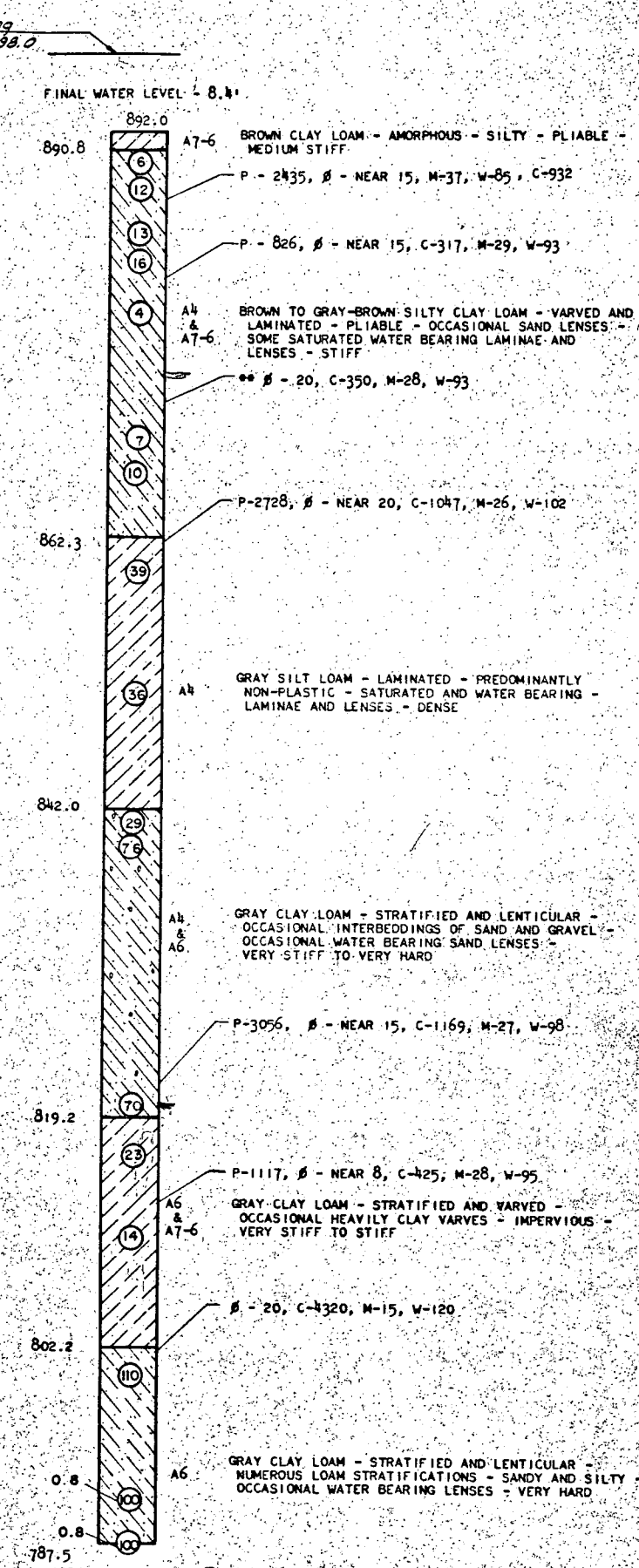
BORING NO. 1
STA. 327+62 - 75' RT. of
Surv. L



BORING NO. 6
STA. 329+38.5 - 50' RT. of
Surv. E

SYMBOLS:
P - MAXIMUM LOAD (LBS/50 FT.)
beta - SHEAR ANGLE (DEGREES)
C - COHESION (LBS/50 FT.)
M - MOISTURE (PERCENT)
W - DRY WEIGHT (LBS/CU.FT.)
** - TRIAXIAL

NOTES:
ENCIRCLED NUMBERS INDICATE THE NUMBER OF BLOWS DELIVERED BY A 140 LB. HAMMER FROM A HEIGHT OF 30" TO DRIVE CORE TUBE 1.0'. THE BORING LOG DATA SHOWN IS FOR DESIGN PURPOSES ONLY. THE STATE ASSUMES NO RESPONSIBILITY IF SOIL CONDITIONS ENCOUNTERED DURING CONSTRUCTION DIFFER FROM THOSE SHOWN.



BORING NO. 3
STA. 330+15 - 75' RT. of
Surv. L

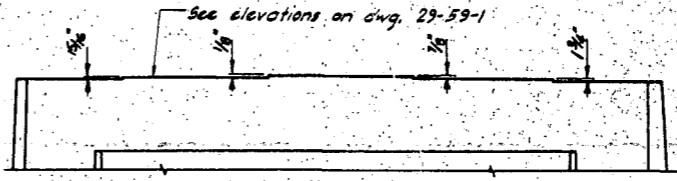
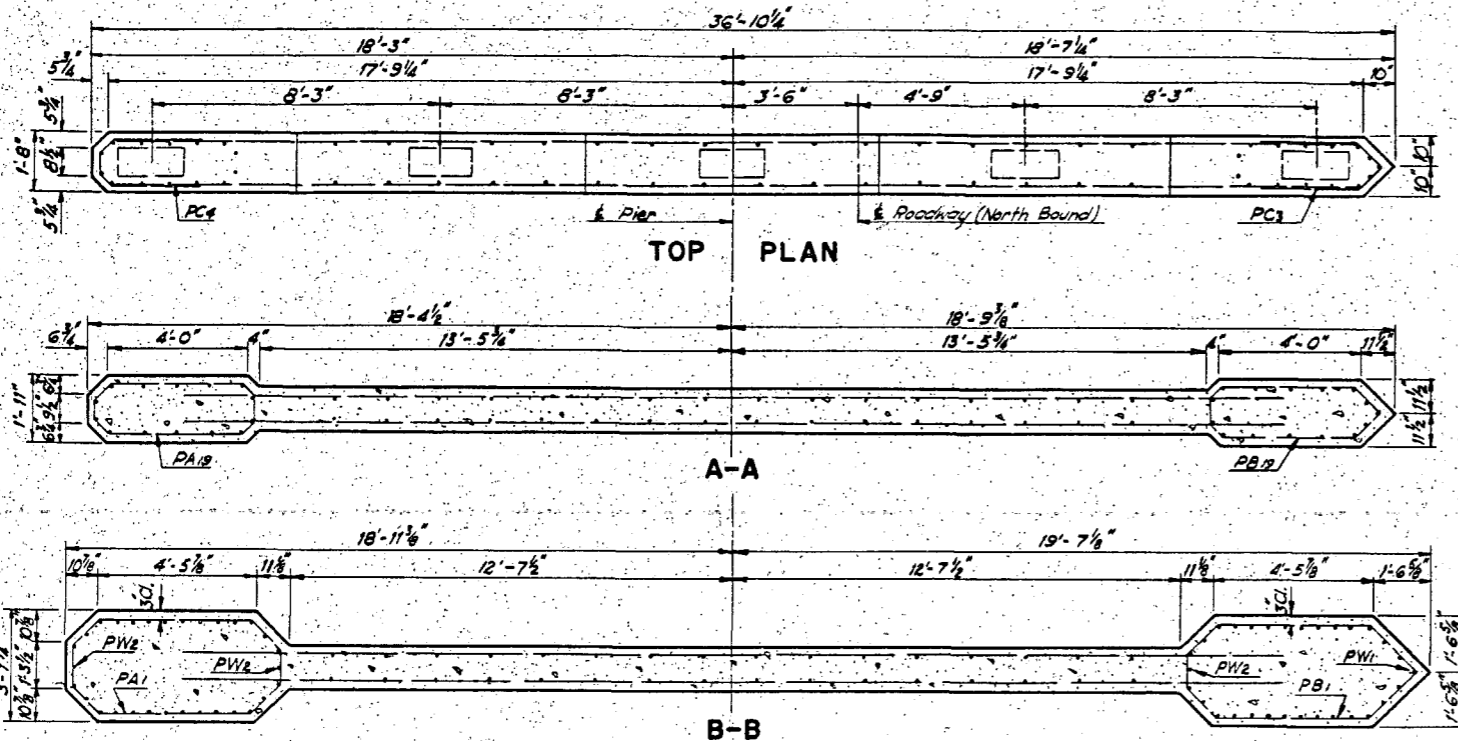
875.6
3.5
8100

BRIDGE NO. 29-59R

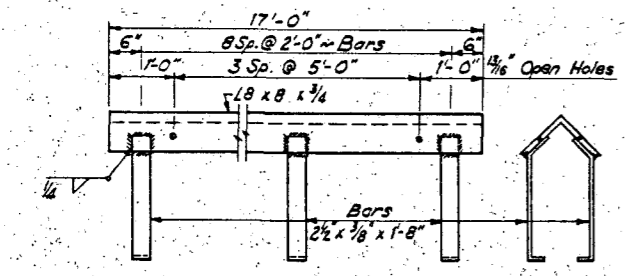
BORING LOG
GOOSE RIVER

TRAILL COUNTY

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
5	N. D.	1-29-2(20)99	61	319	

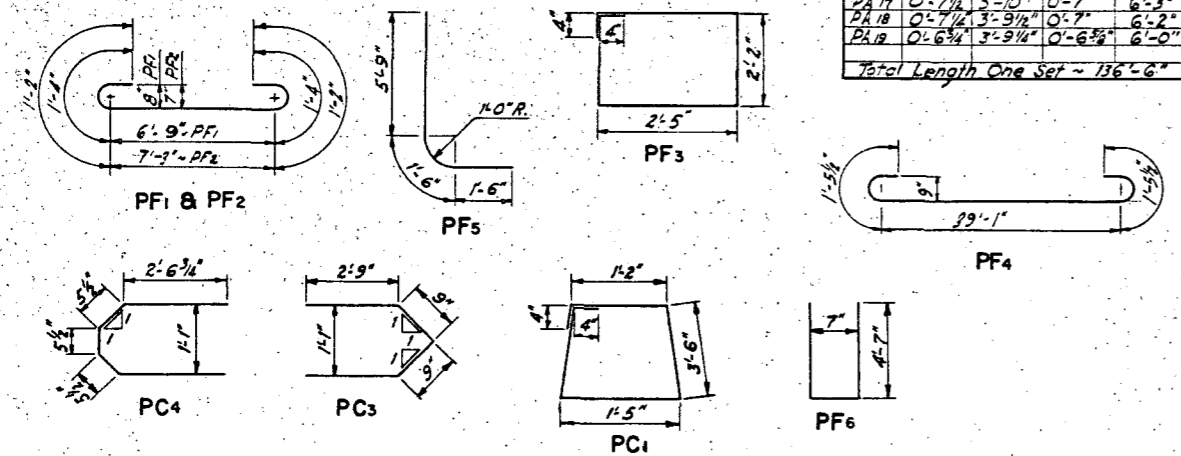


CAP ELEVATION
LOOKING SOUTH ON SOUTHBOUND BRIDGE



ICE NOSE ANGLE

PA BARS				PB BARS				LENGTH		
A	B	C	LENGTH	W	X	Y	Z			
PA1	1'-0 1/2"	4'-3"	0'-11 1/2"	8'-3"	PB1	1'-9 1/2"	4'-3"	1'-0 1/4"	0'-11"	8'-6"
PA2	1'-0 1/4"	4'-2 3/4"	0'-11 1/8"	8'-3"	PB2	1'-9"	4'-2 3/4"	1'-0 1/4"	0'-9"	8'-3"
PA3	1'-0"	4'-2 1/2"	0'-10 3/4"	8'-0"	PB3	1'-8 1/2"	4'-2 1/2"	1'-0"	0'-10"	8'-3"
PA4	0'-11 1/4"	4'-2 1/4"	0'-11 1/4"	8'-0"	PB4	1'-8"	4'-2 1/4"	0'-11 1/4"	0'-8 1/4"	8'-0"
PA5	0'-11 1/2"	4'-1 3/4"	0'-10 3/4"	7'-9"	PB5	1'-7 1/2"	4'-1 3/4"	0'-11 1/4"	0'-9 1/4"	8'-0"
PA6	0'-11"	4'-1 1/2"	0'-10 1/4"	7'-9"	PB6	1'-7"	4'-1 1/2"	0'-11"	0'-7 1/4"	7'-9"
PA7	0'-10 3/4"	4'-1"	0'-10 3/4"	7'-8"	PB7	1'-6 1/2"	4'-1"	0'-10 1/4"	0'-7 3/4"	7'-8"
PA8	0'-10 1/2"	4'-0 3/4"	0'-10 3/8"	7'-6"	PB8	1'-6"	4'-0 3/4"	0'-10 1/4"	0'-7"	7'-6"
PA9	0'-10"	4'-0 1/2"	0'-9 3/4"	7'-3"	PB9	1'-5 1/2"	4'-0 1/2"	0'-10"	0'-8 1/4"	7'-6"
PA10	0'-9 3/4"	4'-0 1/2"	0'-9 3/8"	7'-3"	PB10	1'-4 3/4"	4'-0 1/2"	0'-9 3/4"	0'-7 1/4"	7'-4"
PA11	0'-9 1/2"	3'-11 1/2"	0'-8 7/8"	7'-0"	PA11	1'-4 1/2"	3'-11 1/2"	0'-9 1/4"	0'-7 3/4"	7'-4"
PA12	0'-9"	3'-11"	0'-8 3/4"	6'-11"	PB12	1'-3 3/4"	3'-11"	0'-9"	0'-8"	7'-2"
PA13	0'-8 3/4"	3'-11"	0'-8 1/2"	6'-9"	PB13	1'-3"	3'-11"	0'-8 3/4"	0'-7 1/4"	7'-0"
PA14	0'-8 1/2"	3'-10 3/4"	0'-8 1/4"	6'-9"	PB14	1'-2 3/4"	3'-10 3/4"	0'-8 1/2"	0'-8 1/4"	7'-0"
PA15	0'-8 1/4"	3'-10 1/2"	0'-7 3/4"	6'-6"	PB15	1'-2"	3'-10 1/2"	0'-8 1/4"	0'-8 1/4"	6'-9"
PA16	0'-8"	3'-10 1/4"	0'-7 3/8"	6'-6"	PB16	1'-1 3/4"	3'-10 1/4"	0'-8"	0'-7 1/4"	6'-9"
PA17	0'-7 3/4"	3'-10"	0'-7"	6'-3"	PB17	1'-1 1/2"	3'-10"	0'-7 3/4"	0'-6 3/4"	6'-7"
PA18	0'-7 1/2"	3'-9 3/4"	0'-6 3/4"	6'-2"	PB18	1'-1 1/4"	3'-9 3/4"	0'-7 1/4"	0'-6 1/4"	6'-6"
PA19	0'-6 3/4"	3'-9 1/4"	0'-6 3/8"	6'-0"	PB19	1'-1 1/4"	3'-9 1/4"	0'-6 3/4"	0'-6"	6'-4"
Total Length One Set ~ 136'-6"				Total Length One Set ~ 140'-1"						



BENT BAR DETAILS

Dimensions shown are center to center

PIER BAR LIST (ONE PIER)

MARK	NUMBER	SIZE	LENGTH	SHAPE	UNIT WT.	
PA1	2					
PA19	Set	4	136'-6"	Bent	9113	
PB1	2					
PB19	Set	4	140'-1"	Bent	9352	
PC1		24	5	10'-3"	Bent	10.69
PC2		12	9	34'-4"	Str.	116.73
PC3		6	6	7'-0"	Bent	10.57
PC4		6	6	6'-6"	"	9.76
PF1	16	8	9'-0"	Bent	2403	
PF2	8	7	8'-11"	"	1823	
PF3	16	6	9'-10"	"	1477	
PF4	4	9	42'-0"	Str.	14280	
PF5	60	10	8'-9"	Bent	3765	
PF6	16	5	9'-9"	"	1017	
PW1	29	10	22'-9"	Str.	9790	
PW2	31	10	13'-0"	"	5594	
DW3	32	5	21'-0"	"	2197	
PW4	38	2	5	31'-0"	"	3234
*SP1	1	4	3'-8"	Str.		
*SP2	1	5	4'-0"	"		
*SP3	1	6	4'-6"	"		
*SP4	1	7	5'-0"	"		
*SP5	1	8	5'-4"	"		

*Sample replacement bar to be spliced to bar from which 2'-0" sample has been cut. Furnish only one set for the entire bridge. This is not a pay item and shall be included in the unit price bid for reinforcing steel.

See Dwg. 29-59-6 For Elevation, View, End View, Footing Plan, and Section.

QUANTITIES (ONE PIER)

Concrete Class A2-1	74.7	Cu. Yd.
Reinforcing Steel	12,435	Lbs.
Structural Steel (A-36)	736	Lbs.
Piling (See Layout)		
Excavation (See Layout)		

GOOSE RIVER BRIDGE
26'-0" PIER
37'-0" ROADWAY
HS20 LOADING

REVISIONS

NO.	DATE	BY	DESCRIPTION
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			

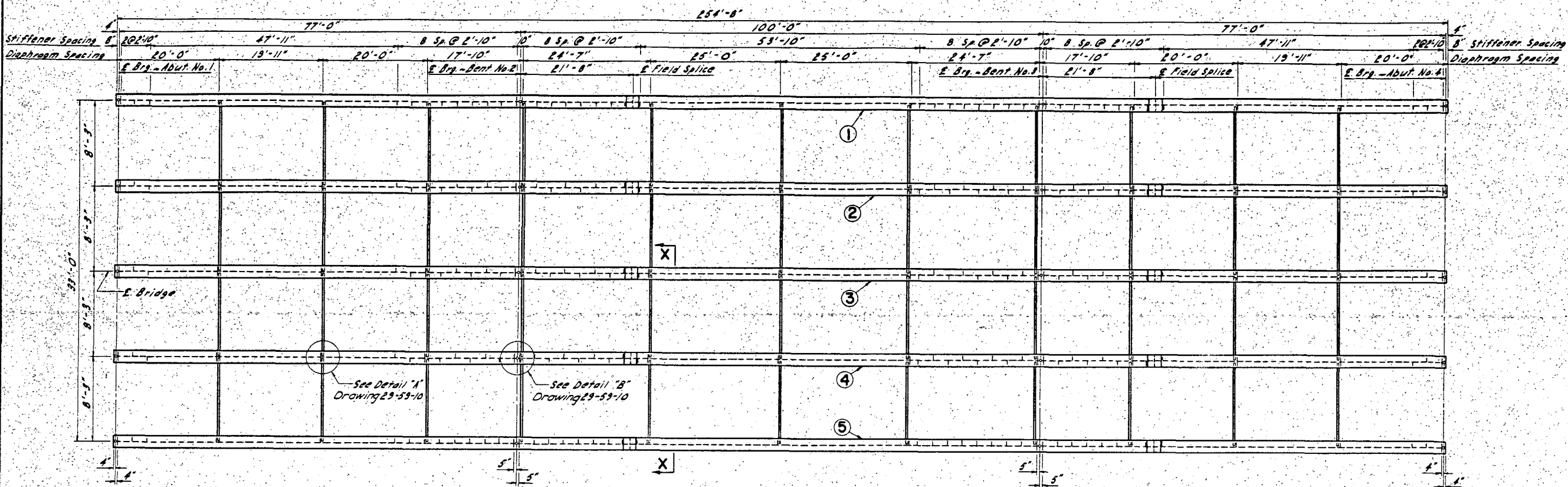
DESIGN MADE BY: MEW
CHECKED BY: LPH
DETAILS MADE BY: GAL
TRACING CHECKED BY: LFG
QUANTITIES MADE BY: G.L.L.
CHECKED BY: D.L.R.

29-59-7

29-59-7

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
5	N. D.	1-29-2(20)99	62	319	

DESIGN	TRACING	QUANTITIES
MADE BY: L.F.H. CHECKED BY: L.P.H.	MADE BY: L.L.G. CHECKED BY: G.L.L.	MADE BY: L.L.G. CHECKED BY: G.L.L.
DETAILS	REVISIONS	DATE
MADE BY: L.L.G. CHECKED BY: G.L.L.		
TRACING		
MADE BY: L.L.G. CHECKED BY: G.L.L.		
QUANTITIES		
MADE BY: L.L.G. CHECKED BY: G.L.L.		



STEEL LAYOUT

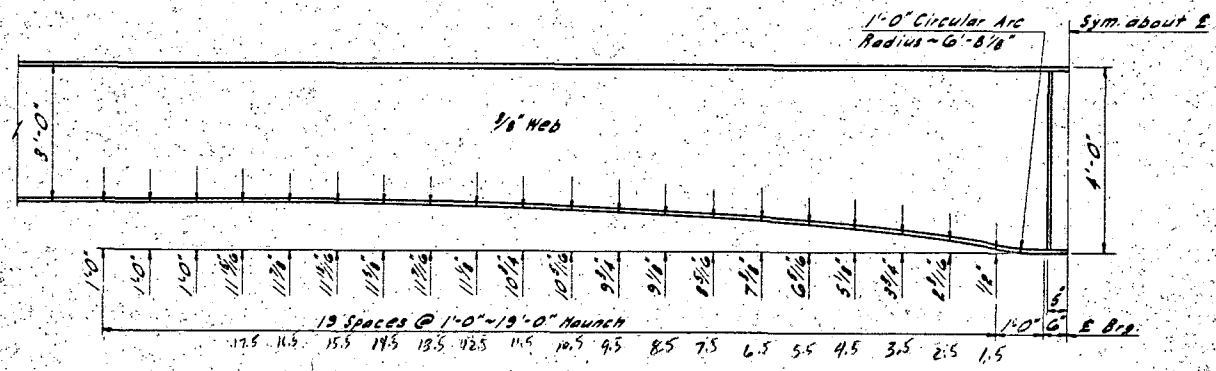
FIELD SPLICES:
TWO FIELD SPLICES ARE PROVIDED ON DRAWING 29-59-9. FALSEWORK WITH PROVISIONS FOR JACKING MUST BE PROVIDED DURING ERECTION TO HOLD THE GIRDERS IN ALIGNMENT WHILE SPLICES ARE BEING MADE.

OPTIONAL FIELD SPLICES:
IN ADDITION TO THE TWO FIELD SPLICES ON DRAWING 29-59-9, TWO OPTIONAL FIELD SPLICES ARE SHOWN. THESE TWO ADDITIONAL FIELD SPLICES MAY BE USED BY THE CONTRACTOR UNDER THE FOLLOWING CONDITIONS:
1. THEY ARE MADE AT NO EXPENSE TO THE STATE.
2. FLANGE AND WEB SPLICE PLATES AND BOLTS WILL BE AS SHOWN IN DETAILS ON DRAWING 29-59-10
3. FALSEWORK AT THESE SPLICE POINTS WITH PROVISIONS FOR JACKING MUST BE PROVIDED DURING ERECTION TO HOLD THE GIRDERS IN ALIGNMENT WHILE SPLICES ARE BEING MADE.

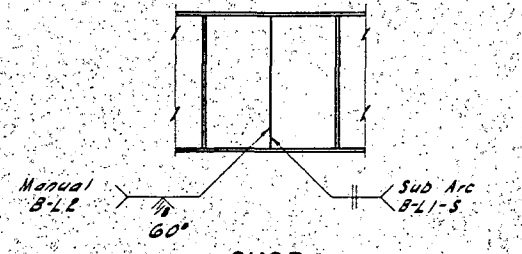
THE SHOP FABRICATION AND ERECTION DRAWING MUST INDICATE FOUR PICKUP POINTS FOR EACH MEMBER OVER 100 FEET LONG TO BE USED DURING SHIPPING AND ERECTION.

THE DETAILS SHOWN ARE FOR A THREE SPAN CONTINUOUS WELDED GIRDER AND REPRESENT GIRDER NO. 1 IN A FIVE (5) GIRDER BRIDGE. GIRDERS NO. 2, NO. 3, NO. 4 AND NO. 5 ARE SIMILAR TO GIRDER NO. 1 AND SHALL BE FABRICATED IN ACCORDANCE WITH THESE DETAILS AND DRAWINGS 29-59-9 THRU 29-59-10.

ALL SHOP BUTT WELDS IN THE FLANGE PLATES SHALL BE MADE BEFORE FINAL FITTING AND WELDING INTO THE GIRDER.

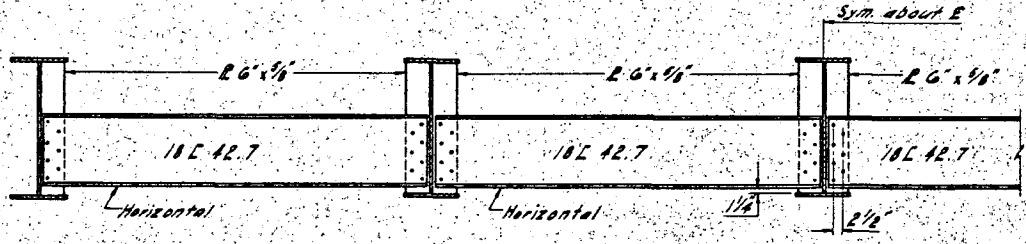


GIRDER HAUNCH DETAILS

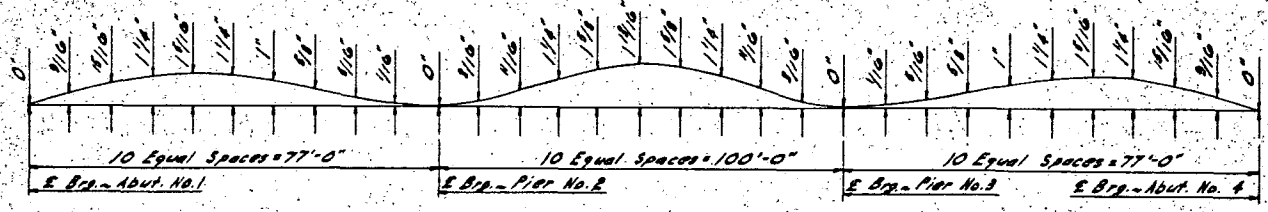


SHOP WEB SPLICE

If optional field splices are used in spans 1 and 2, shop web splices will not be used in those spans.



X-X
Typical for all diaphragms



SHOP CAMBER DIAGRAM

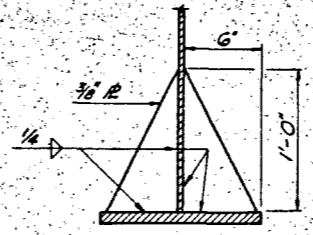
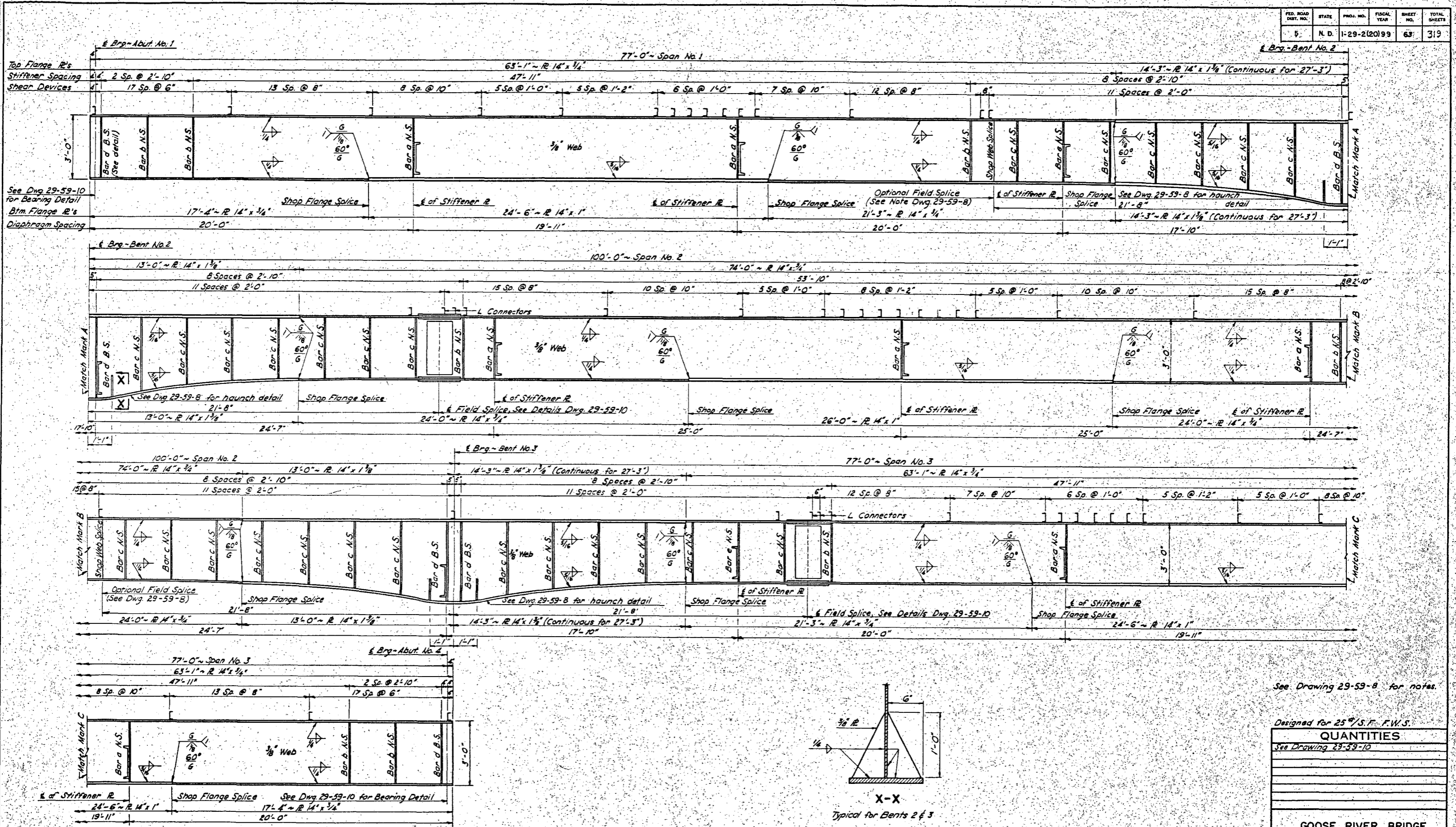
The shop camber diagram above represents the total rise in inches above a chord between the E of abutment bearings that shall be cut into the web plate of the girders to compensate for the dead load deflection of the superstructure, plus the vertical curve of the roadway. The camber shown shall be in addition to the rise required for the girder haunch. The depth of the web plate will also vary according to the thickness of the flange plates.

Designed for 25 #5 F.F.W.S.

QUANTITIES	
SEE DRAWING 29-59-10	

GOOSE RIVER BRIDGE
SUPERSTRUCTURE DETAILS
37'-0" ROADWAY
HS20 LOADING

DESIGN	MADE BY: NEW
CHECKED BY: LPH	CHECKED BY: LPH
DETAILS	MADE BY: LPH
CHECKED BY: GAG	CHECKED BY: GAG
TRACING	MADE BY: DLP
CHECKED BY: GAG	CHECKED BY: GAG
QUANTITIES	MADE BY: Z.P.
	CHECKED BY: GAG



X-X
Typical for Bents 2 & 3

See Drawing 29-59-8 for notes.

Designed for 25% S.F. F.W.S.
QUANTITIES
See Drawing 29-59-10

**GOOSE RIVER BRIDGE
WELDED GIRDER DETAIL**

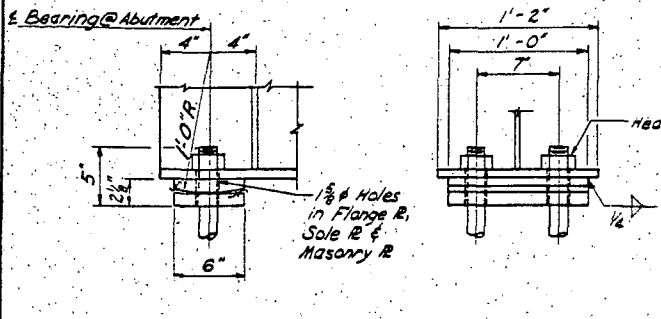
37'-0" ROADWAY
HS20 LOADING

Nomenclature:
B. S. = Both Sides
N. S. = Near Side

ELEVATION

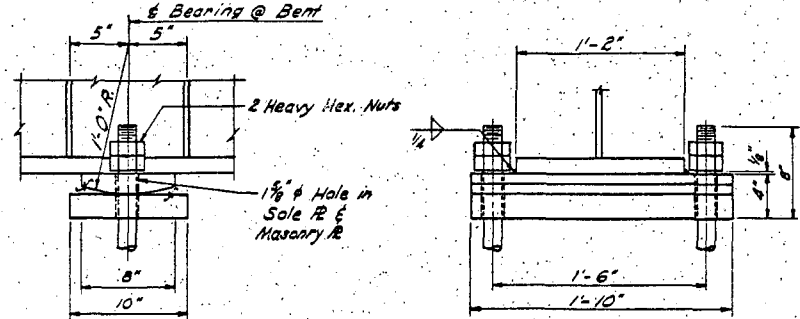
FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
5	N. D.	1-29-2(20)99	64	313	

DESIGN	MADE BY	NEW
DETAILS	CHECKED BY	LPH
TRACING	MADE BY	GAL
QUANTITIES	CHECKED BY	DLR
	MADE BY	G.F.S.
	CHECKED BY	G.A.L.



Sole R ~ 6" x 1 1/8" x 1'-0"
 Masonry R ~ 6" x 1" x 1'-0"
 * Swedge Bolts ~ 1/2" x 1'-9"

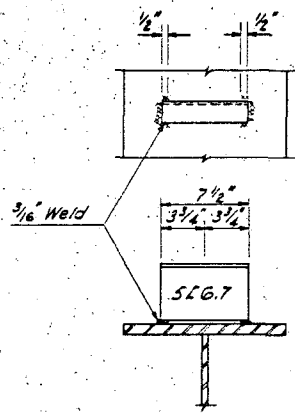
* Note: Swedge bolts to be drilled in and anchored in quick setting anchor grout. See Special Provisions.



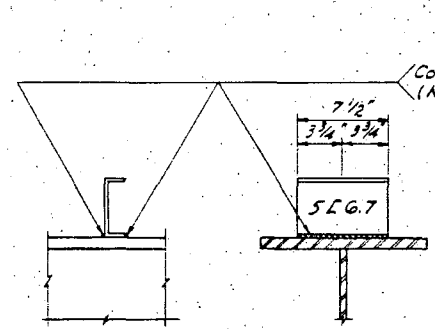
Sole R ~ 8" x 1 1/8" x 1'-10"
 Masonry R ~ 10" x 2 1/8" x 1'-10"
 Swedge Bolts ~ 1/2" x 2'-0"

ABUTMENT BEARING DETAILS

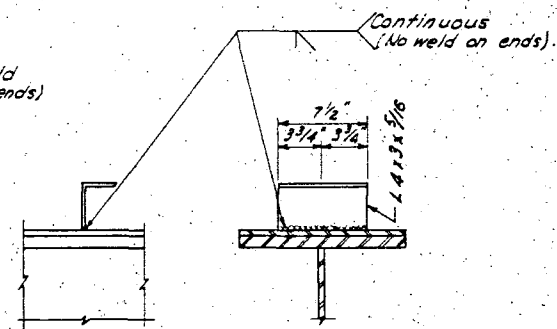
PIER BEARING DETAILS



Use in Span 1 & 3 and in negative moment regions where spacing is 2'-0".

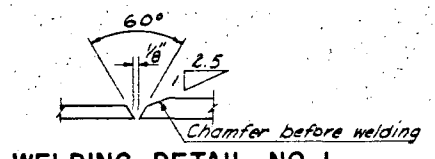


Use in positive moment region except over splice R's.



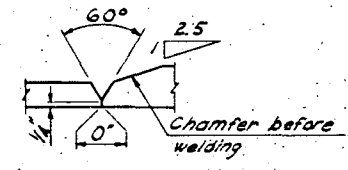
Use over splice R's.

SHEAR DEVICE DETAILS



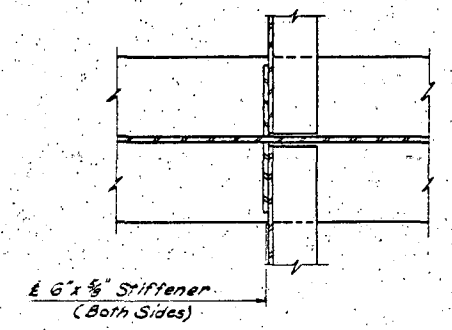
WELDING DETAIL NO. 1

SHOP FLANGE SPLICE
 Bottom Flange Shown
 For manual shielded arc welding use joint B-L2 (Shown above).

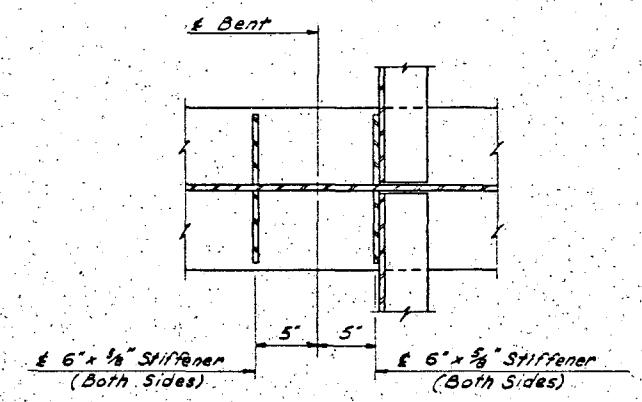


ALTERNATE DETAIL NO. 1

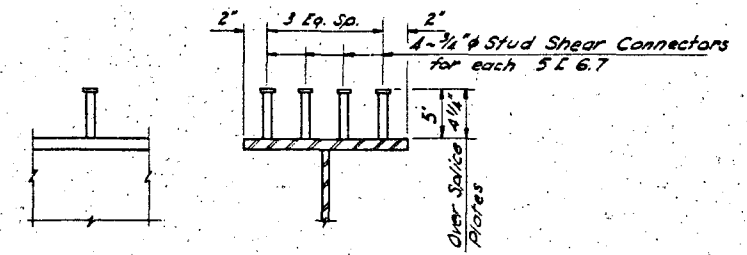
SHOP FLANGE SPLICE
 Bottom Flange Shown
 Submerged Arc AWS B-L2b-S
 If submerged arc welding is used AWS B-L2b-S is an acceptable replacement for B-L2 (Manual).



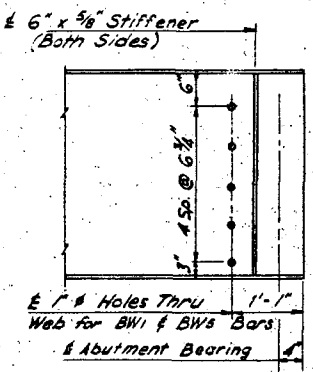
DETAIL "A"



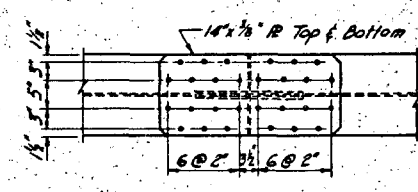
DETAIL "B"



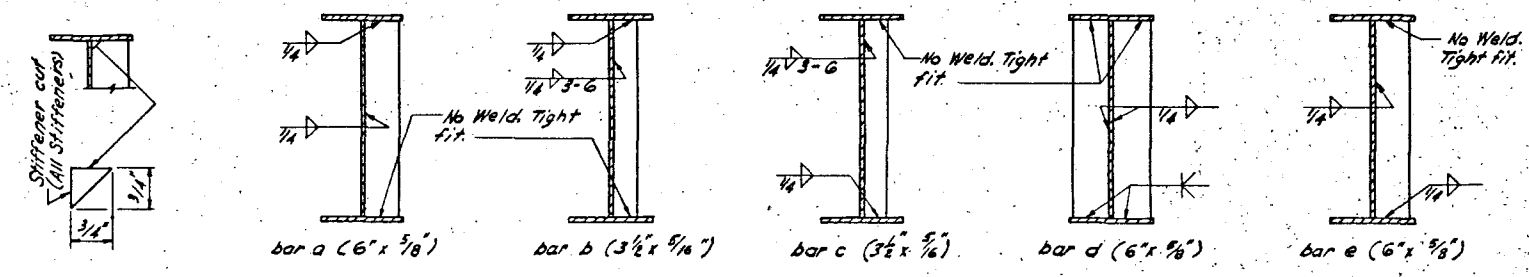
ALTERNATE SHEAR DEVICE



BEAM DETAILS



FIELD SPLICE DETAIL



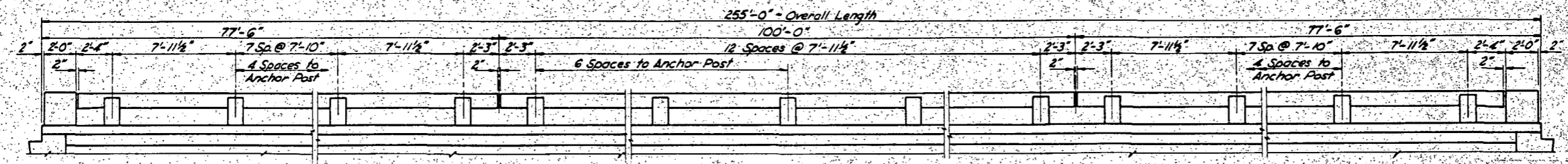
WEB STIFFENER DETAILS

29-59-10

Designed for 25 #/S.F. F.W.S.

QUANTITIES (ONE BRIDGE)	
Structural Steel (A-44)	176,760 Lbs.
Structural Steel (A-36)	31,568 Lbs.

GOOSE RIVER BRIDGE
 SUPERSTRUCTURE DETAILS
 37'-0" ROADWAY
 HS20 LOADING



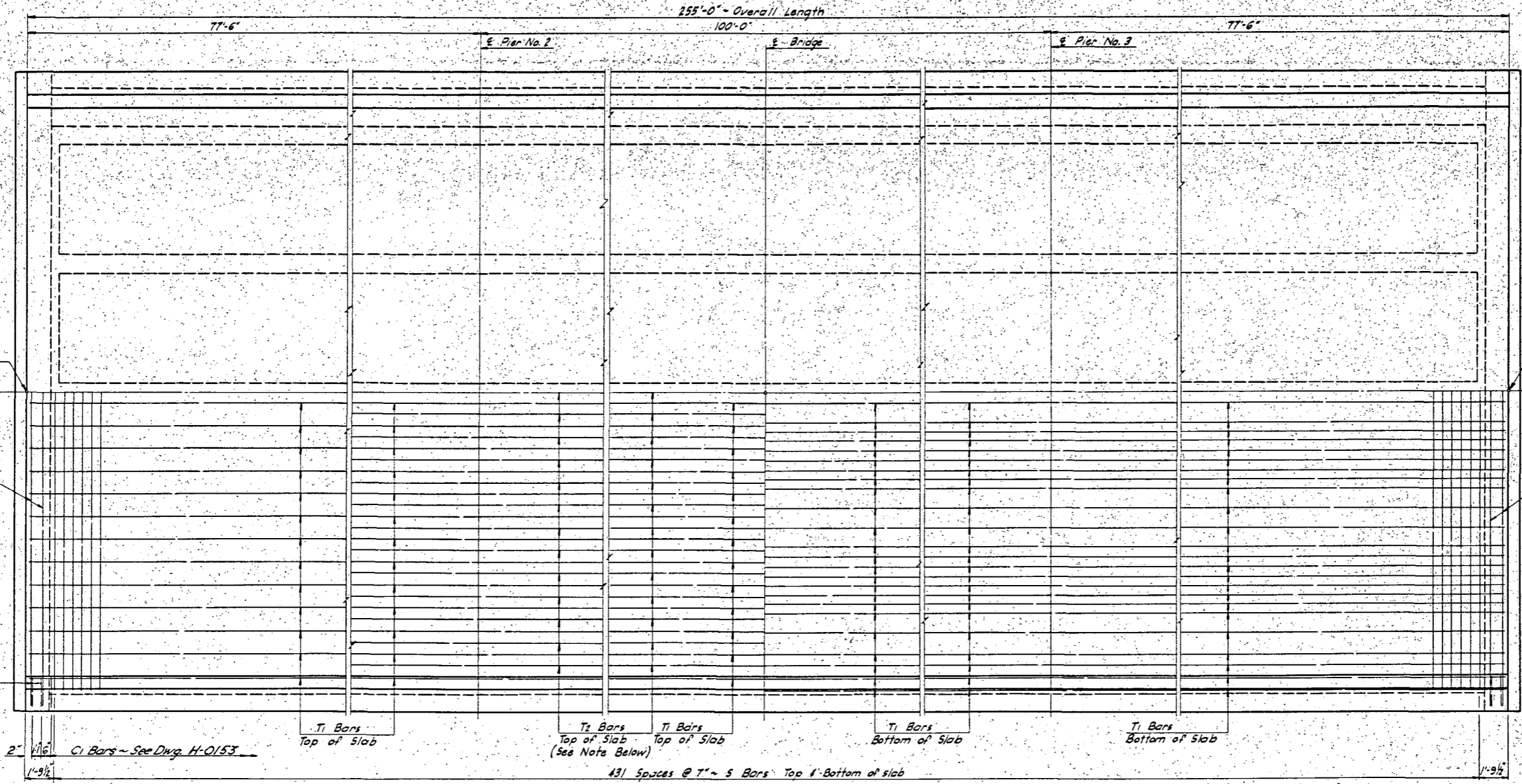
PART ELEVATION
See H-0153 for Railing Details



CURB SLEEVE DETAILS

NOTES:

It is assumed that the Contractor can place the slab concrete in one continuous operation in accordance with Sections 602-36.1 thru 602-36.2.1.4 of the Standard Specifications. If the Contractor cannot pour at the specified rate, he shall submit drawings to the Bridge Engineer, for approval, showing proposed construction joints and pouring sequence.
Each curb shall be poured in one continuous operation.
See Std. Drawing H-0153 for railing details.
See Drawing 29-59-8 thru 29-59-10 for Structural Steel details.
The end beams shall be poured at the same time that the deck slab is poured. Special care shall be taken to completely fill the space under the girders in the end beams with concrete.



PART PLAN

Half Showing Slab Reinforcing
Typical Both Sides - by Rotation

SPLICING DETAILS

T1, E1, & E2

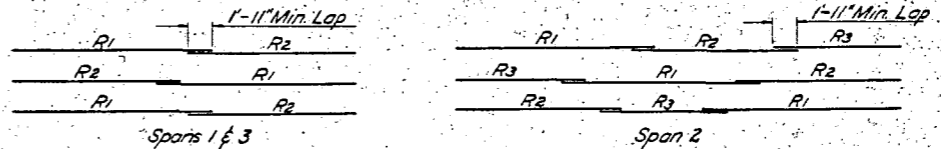
Designed for 25' S.F. F.W.S.

QUANTITIES

See Drawing 29-59-12 for Quantities

**GOOSE RIVER BRIDGE
SLAB & RAILING DETAILS**

37'-0" ROADWAY
HS20 LOADING



R BAR SPLICING DETAIL

NOTE:

T2 Bar placement - 20 feet each side of pier.

DESIGN	MADE BY	CHECKED BY	DATE
DETAILS	W.P.H.	L.P.H.	
TRACING	G.A.L.	G.A.L.	
QUANTITIES	G.A.L.	G.A.L.	

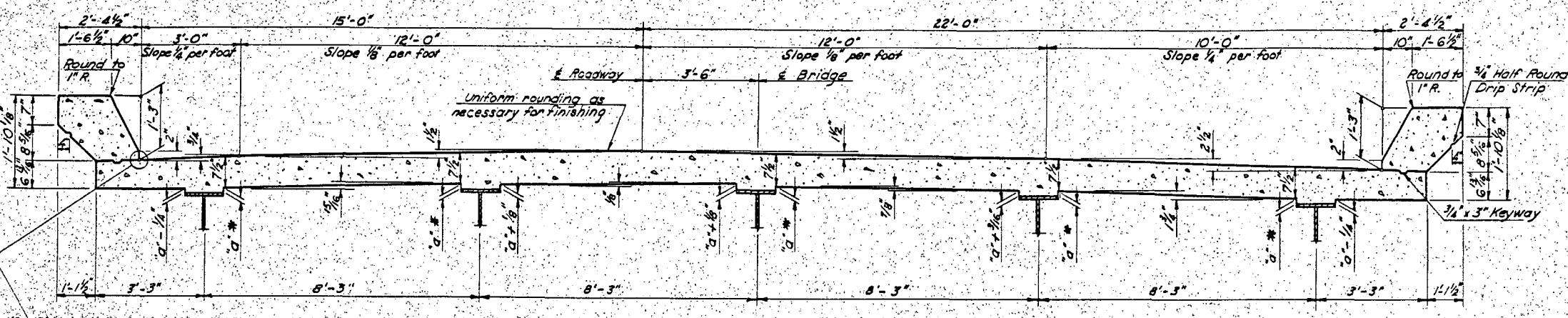
29-59-11

SUPERSTRUCTURE (ONE BRIDGE)

MARK	NUMBER	SIZE	LENGTH	SHAPE	UNIT WT.
P4	180	6	3'-8"	Bent	5.51
P5	18	6	4'-5"	"	6.63
P6	132	5	4'-11"	"	5.13
P7	132	3	3'-10"	"	1.44
P8	120	3	2'-10"	"	1.06
P9	12	3	4'-4"	"	1.63
RC	456	3	2'-11"	Bent	1.10
R1	36	6	40'-0"	Str.	60.08
R2	36	6	36'-8"	"	55.08
R3	12	6	26'-8"	"	40.06

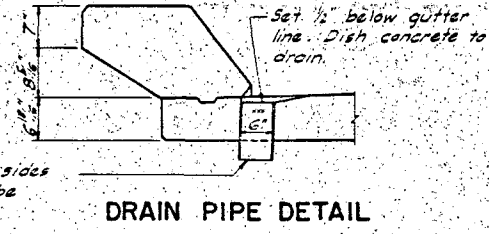
SUPERSTRUCTURE (ONE BRIDGE)

MARK	NUMBER	SIZE	LENGTH	SHAPE	UNIT WT.
BW1	32	6	2'-8"	Str.	32.55
BW2	68	6	6'-4"	Bent	9.51
BW3	76	6	3'-0"	"	4.51
BW4	8	6	9'-4"	"	14.02
BW5	20	5	5'-0"	"	5.22
BW6	68	5	6'-9"	"	7.04
BW7	84	5	4'-8"	"	4.87
C1	580	5	5'-9"	Bent	6.00
C2	8	5	7'-1"	"	7.39
E1	14	6	38'-1"	Str.	57.20
E2	24	5	37'-9"	"	39.38
S	864	6	39'-2"	Str.	58.83
T1	5/8	5	37'-9"	Str.	39.38
T2	50	6	40'-0"	"	60.08

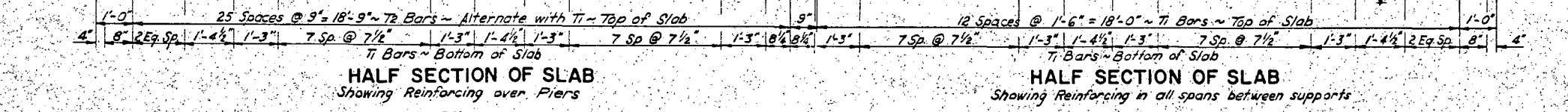
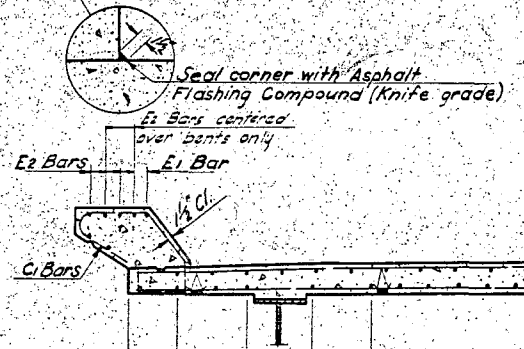


SECTION OF SLAB
Showing Dimensions

* a equals 1/8" @ bearings of piers and abutments. At intermediate points "a" equals screed elevation at roadway minus top of girder elevation minus (.9043 girder no. 1), (.7569 girder no. 2), (.6675 girder no. 3), (.6806 girder no. 4), (.7587 girder no. 5).

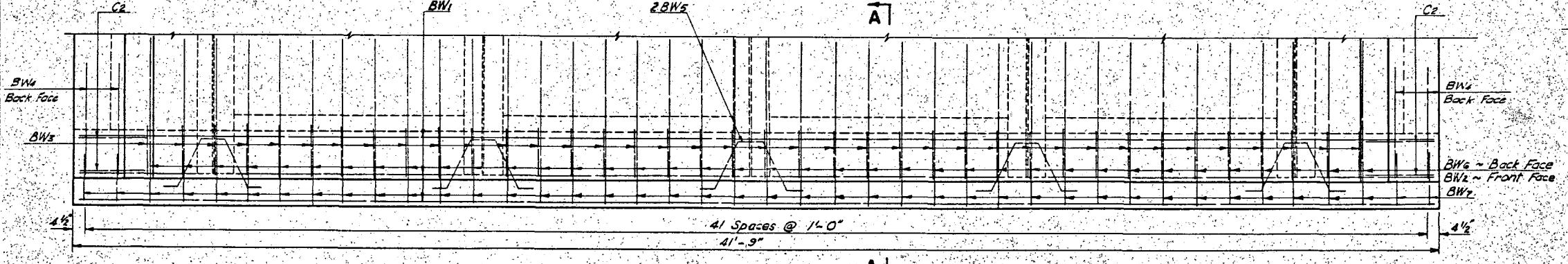


DRAIN PIPE DETAIL

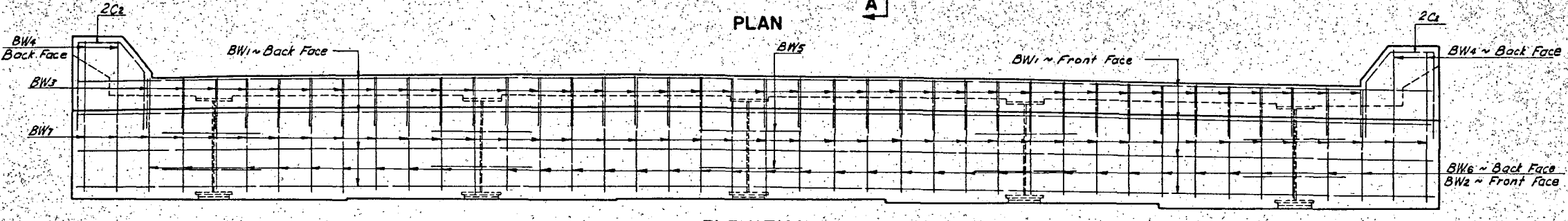


HALF SECTION OF SLAB
Showing Reinforcing over Piers

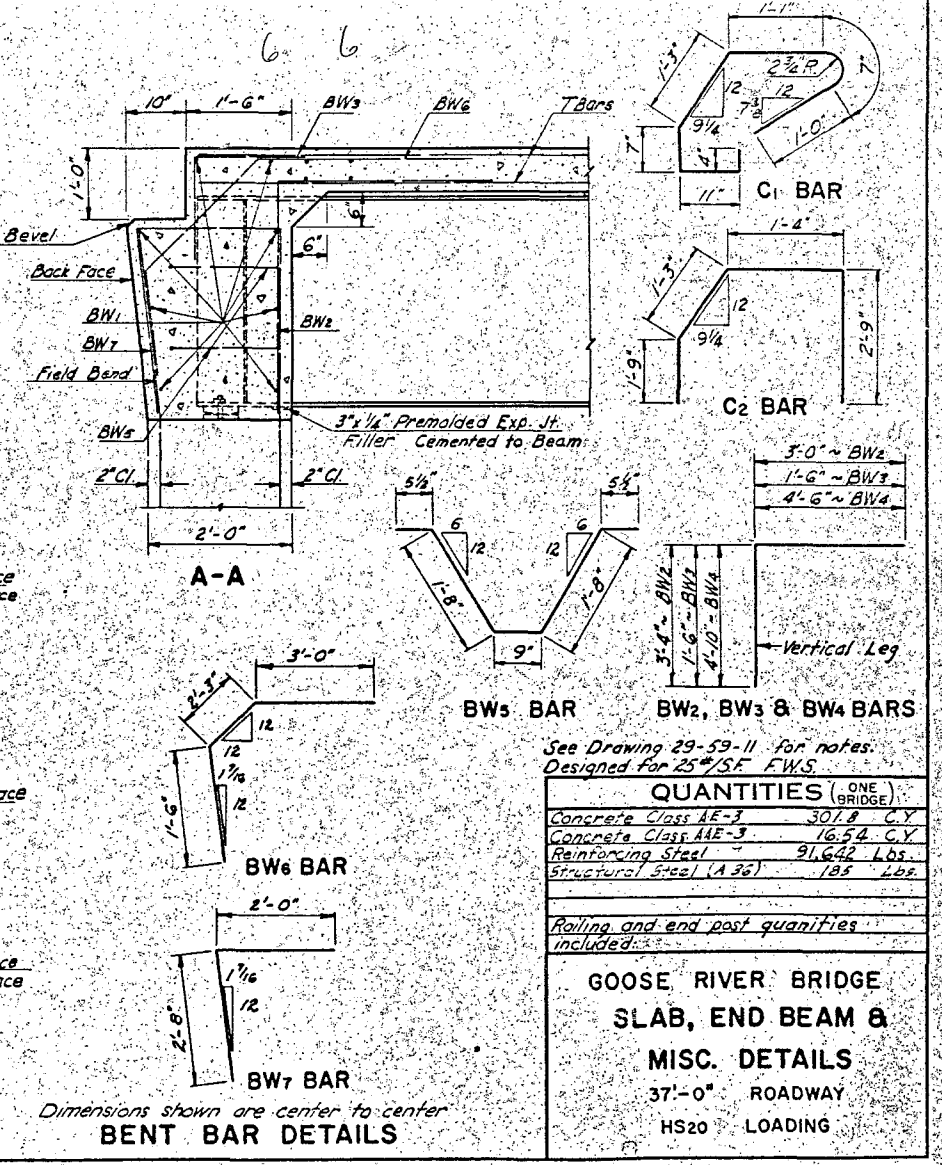
HALF SECTION OF SLAB
Showing Reinforcing in all spans between supports



PLAN



ELEVATION
END BEAM DETAILS



BENT BAR DETAILS

See Drawing 29-59-11 for notes.
Designed for 25" SF F.W.S.

QUANTITIES (ONE BRIDGE)

Concrete Class AE-3	301.8	C.Y.
Concrete Class AAE-3	16.54	C.Y.
Reinforcing Steel	91,642	Lbs.
Structural Steel (A 36)	185	Lbs.

Rolling and end post quantities included.

GOOSE RIVER BRIDGE
SLAB, END BEAM & MISC. DETAILS
37'-0" ROADWAY
HS20 LOADING

MADE BY: ALEW
CHECKED BY: LFG
DESIGN: LFG
DETAILS: LFG
TRACING: LFG
QUANTITIES: LFG

29-59-12